Log of Test Borings

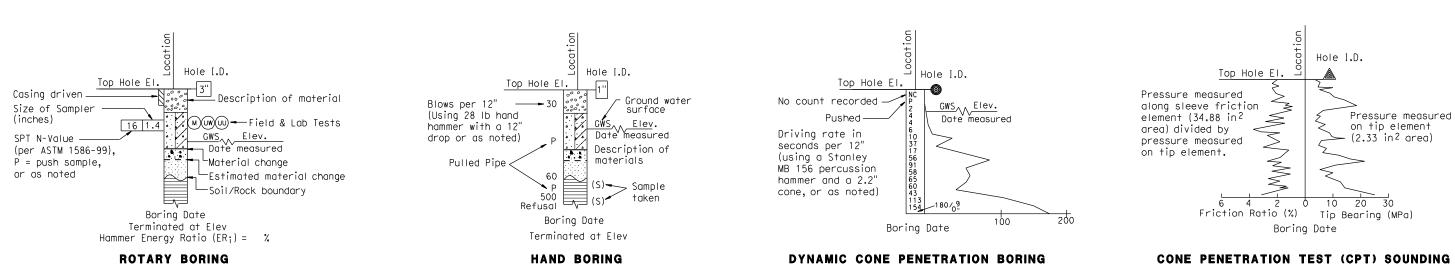
REFERENCE: CALTRANS SOIL & ROCK LOGGING, CLASSIFICATION, AND PRESENTATION MANUAL (JUNE 2007), EXCEPT AS NOTED IN APPENDIX A.1 OF THE FINAL GEOTECHNICAL SUMMARY REPORT, SR-710 TUNNEL TECHNICAL STUDY, LOS ANGELES COUNTY, CALIFORNIA, DATED APRIL, 2010.

CEMENTATION				
Description	Criteria			
Weak	Crumbles or breaks with handling or little finger pressure.			
Moderate	Crumbles or breaks with considerable finger pressure.			
Strong	Will not crumble or break with finger pressure.			

	BOREHOLE IDENTIFICATION					
Symbol	Hole Type	Description				
Size	А	Auger Boring				
Size	R P	Rotary drilled boring Rotary percussion boring (air)				
Size	R	Rotary drilled diamond core				
Size	HD HA	Hand driven (1-inch soil tube) Hand Auger				
0	D	Dynamic Cone Penetration Boring				
	СРТ	Cone Penetration Test (ASTM D 5778-95)				
_ ] [ ]	0	Other				
	Note: Size in inches.					

	CONSISTENCY OF COHESIVE SOILS					
Description	Unconfined Compressive Strength (tsf)	Pocket Penetrometer Measurement (tsf)	Torvane Measurement (tsf)	Field Approximation		
Very Soft	< 0.25	< 0.25	< 0.12	Easily penetrated several inches by fist		
Soft	0.25 to 0.50	0.25 to 0.50	0.12 to 0.25	Easily penetrated several inches by thumb		
Medium Stiff	0.50 to 1.0	0.50 to 1.0	0.25 to 0.50	Penetrated several inches by thumb with moderate effort		
Stiff	1 to 2	1 to 2	0.50 to 1.0	Readily indented by thumb but penetrated only with great effort		
Very Stiff	2 to 4	2 to 4	1.0 to 2.0	Readily indented by thumbnail		
Hard	> 4.0	> 4.0	> 2.0	Indented by thumbnail with difficulty		

	PLASTICITY OF FINE-GRAINED SOILS				
Description	Criteria				
Nonplastic	A 1/8-inch thread cannot be rolled at any water content.				
Low	The thread can barely be rolled and the lump cannot be formed when drier than the plastic limit.				
Medium	The thread is easy to roll and not much time is required to reach the plastic limit.  The thread cannot be rerolled after reaching the plastic limit. The lump crumbles when drier than the plastic limit.				
High	It takes considerable time rolling and kneading to reach the plastic limit. The thread can be rerolled several times after reaching the plastic limit. The lump can be formed without crumbling when drier than the plastic limit.				



PREPARED BY KATHLEEN REYES  CHECKED BY DAN JANKLY  CHECKED BY DAN JA	ENGINEERING SERVICES	GEOTECHNICAL SERVICES	PREPARED FOR THE STATE OF	DIVISION OF ENGINEERING SERVICES STRUCTURE DESIGN	BRIDGE NO.	S	OIL LEGEND	
CHECKED BY DAN JANKLY  DEPARTMENT OF TRANSPORTATION  DESIGN BRANCH  N/A  LOG OF LEST BORINGS 1 OF 2  ORIGINAL SCALE IN INCHES	PREPARED BY	KATHLEEN REYES		l	POST MILES			
GRIGINAL SCALE IN INCHES DISREGARD PRINTS BEARING DISREGARD PRINTS BEARING	CHECKED BY	DAN JANKLY		DESIGN BRANCH	N/A LC	OG OF TE	ST BORINGS	1 OF 2
	GS LOTB SOIL LEGEND	ORIGINAL SCALE IN INCHES FOR REDUCED PLANS	0 1 2 3	CU EA 07-187900	DISREGARD PRINTS BEARING EARLIER REVISION DATES	NG	REVISION DATES	SHEET

PLANS APPROVAL DATE

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REFERENCE: CALTRANS SOIL & ROCK LOGGING, CLASSIFICATION, AND PRESENTATION MANUAL (JUNE 2007), EXCEPT AS NOTED IN APPENDIX A.1 OF THE FINAL GEOTECHNICAL SUMMARY REPORT, SR-710 TUNNEL TECHNICAL STUDY, LOS ANGELES COUNTY, CALIFORNIA, DATED APRIL, 2010.

GROUP SYMBOLS AND NAMES							
Graphic/Sym	Graphic/Symbol Group Names Graphic/Symbol Group Names						
GW	Well-graded GRAVEL Well-graded GRAVEL with SAND		CL	Lean CLAY Lean CLAY with SAND Lean CLAY with GRAVEL SANDY lean CLAY			
GP	Poorly graded GRAVEL Poorly graded GRAVEL with SAND		02	SANDY lean CLAY with GRAVEL GRAVELLY lean CLAY GRAVELLY lean CLAY with SAND			
GW-C	Well-graded GRAVEL with SILT and SAND		CL-ML	SILTY CLAY SILTY CLAY with SAND SILTY CLAY with GRAVEL SANDY SILTY CLAY			
GW-C	Well-graded GRAVEL with CLAY (or SILTY CLAY) Well-graded GRAVEL with CLAY and SAND (or SILTY CLAY and SAND)			SANDY SILTY CLAY with GRAVEL GRAVELLY SILTY CLAY GRAVELLY SILTY CLAY with SAND			
GP-(	Poorly graded GRAVEL with SILT and SAND		ML	SILT SILT with SAND SILT with GRAVEL SANDY SILT			
GP-(	Poorly graded GRAVEL with CLAY (or SILTY CLAY) Poorly graded GRAVEL with CLAY and SAND (or SILTY CLAY and SAND)			SANDY SILT with GRAVEL GRAVELLY SILT GRAVELLY SILT with SAND			
GM	SILTY GRAVEL with SAND		OL	ORGANIC lean CLAY ORGANIC lean CLAY with SAND ORGANIC lean CLAY with GRAVEL SANDY ORGANIC lean CLAY			
GC	CLAYEY GRAVEL CLAYEY GRAVEL with SAND			SANDY ORGANIC lean CLAY with GRAVEL GRAVELLY ORGANIC lean CLAY GRAVELLY ORGANIC lean CLAY with SAND			
GC-C	SILTY, CLAYEY GRAVEL SILTY, CLAYEY GRAVEL with SAND		OL	ORGANIC SILT ORGANIC SILT with SAND ORGANIC SILT with GRAVEL SANDY ORGANIC SILT			
SW	Well-graded SAND Well-graded SAND with GRAVEL			SANDY ORGANIC SILT with GRAVEL GRAVELLY ORGANIC SILT GRAVELLY ORGANIC SILT with SAND			
SP	Poorly graded SAND Poorly graded SAND with GRAVEL		СН	Fat CLAY Fat CLAY with SAND Fat CLAY with GRAVEL SANDY fat CLAY			
SW-S	Well-graded SAND with SILT and GRAVEL		J.,	SANDY fat CLAY with GRAVEL GRAVELLY fat CLAY GRAVELLY fat CLAY with SAND			
SW-S	Well-graded SAND with CLAY (or SILTY CLAY) Well-graded SAND with CLAY and GRAVEL (or SILTY CLAY and GRAVEL)		мн	Elastic SILT Elastic SILT with SAND Elastic SILT with GRAVEL SANDY elastic SILT			
SP-9	Poorly graded SAND with SILT and GRAVEL			SANDY elastic SILT with GRAVEL GRAVELLY elastic SILT GRAVELLY elastic SILT with SAND			
SP-9	Poorly graded SAND with CLAY (or SILTY CLAY) Poorly graded SAND with CLAY and GRAVEL (or SILTY CLAY and GRAVEL)		ОН	ORGANIC fat CLAY ORGANIC fat CLAY with SAND ORGANIC fat CLAY with GRAVEL SANDY ORGANIC fat CLAY			
SM	SILTY SAND SILTY SAND with GRAVEL			SANDY ORGANIC fat CLAY with GRAVEL GRAVELLY ORGANIC fat CLAY GRAVELLY ORGANIC fat CLAY with SAND			
sc	CLAYEY SAND CLAYEY SAND with GRAVEL		ОН	ORGANIC elastic SILT ORGANIC elastic SILT with SAND ORGANIC elastic SILT with GRAVEL SANDY ORGANIC elastic SILT			
sc-s	SILTY, CLAYEY SAND SILTY, CLAYEY SAND with GRAVEL			SANDY ORGANIC elastic SILT with GRAVEL GRAVELLY ORGANIC elastic SILT GRAVELLY ORGANIC elastic SILT with SAND			
77. 77. PL	PEAT		OL/OH	ORGANIC SOIL ORGANIC SOIL with SAND ORGANIC SOIL with GRAVEL SANDY ORGANIC SOIL			
	COBBLES COBBLES and BOULDERS BOULDERS	ST ST 3 ST ST 3 ST ST 3		SANDY ORGANIC SOIL with GRAVEL GRAVELLY ORGANIC SOIL GRAVELLY ORGANIC SOIL with SAND			

FIELD	AND	LABORATORY	TESTING
11660	7110		1 - 0 1 111 4

- (C) Consolidation (ASTM D 2435)
- (CAI) Cerchar Abrasivity Index
- (CL) Collapse Potential (ASTM D 5333)
- (CP) Compaction Curve (CTM 216)
- CR) Corrosivity Testing (CTM 643, CTM 422, CTM 417)
- CSA Compressive Strength and Elastic Moduli of Rock Core (ASTM D 7012)
- CU Consolidated Undrained Triaxial (ASTM D 4767)
- (DS) Direct Shear (ASTM D 3080)
- (EI) Expansion Index (ASTM D 4829)
- Elastic Moduli of Rock Core in Uniaxial Compression (ASTM D 3148)
- M) Moisture Content (ASTM D 2216)
- OC) Organic Content-% (ASTM D 2974)
- P Permeability (CTM 220)
- (PA) Particle Size Analysis (ASTM D 422)
- Pl Plasticity Index (AASHTO T 90) Liquid Limit (AASHTO T 89)
- (PL) Point Load Index (ASTM D 5731)
- (PM) Pressure Meter
- (PP) Pocket Penetrometer
- (PTS) Petrographic Thin Section Analysis
- (R) R-Value (CTM 301)
- (SD) Slake Durability (ASTM D 4644)
- (SE) Sand Equivalent (CTM 217)
- (SG) Specific Gravity (AASHTO T 100)
- (SL) Shrinkage Limit (ASTM D 427)
- (SW) Swell Potential (ASTM D 4546)
- (TV) Pocket Torvane
- Unconfined Compression-Soil
  (ASTM D 2166)
  - Unconfined Compression-Rock (ASTM D 2938)
- Unconsolidated Undrained Triaxial (ASTM D 2850)
- (UW) Unit Weight (ASTM D 4767)
- (VS) Vane Shear (AASHTO T 223)

DIST COUNTY	ROUTE	POST MILES TOTAL PROJECT	SHEET No	TOTAL SHEETS		
7 LA	710	N/A				
GEOTECHNICAL ENGINEER  3/18/10 DATE  PLANS APPROVAL DATE  The State of California or its officers or agents shall not be responsible for the accuracy or completeness of electroaic copies of this plan sheet.						
CH2M HILL 6 HUTTON CENTRE DRIVE, SUITE 700 SANTA ANA, CA 92707						

APPARENT DENSI	TY OF COHESIONLESS SOILS
Description	SPT N <sub>60</sub> (Blows / 12 inches)
Very loose	0 - 4
Loose	5 - 10
Medium Dense	11 - 30
Dense	31 - 50
Very Dense	> 50

	MOISTURE				
Description Criteria					
Dry	Absence of moisture, dusty, dry to the touch				
Moist	Damp but no visible water				
Wet	Visible free water, usually soil is below water table				

PERCE	PERCENT OR PROPORTION OF SOILS				
Description	Criteria				
Trace	Particles are present but estimated to be less than 5%				
Few	5 to 10%				
Little	15 to 25%				
Some	30 to 45%				
Mostly	50 to 100%				

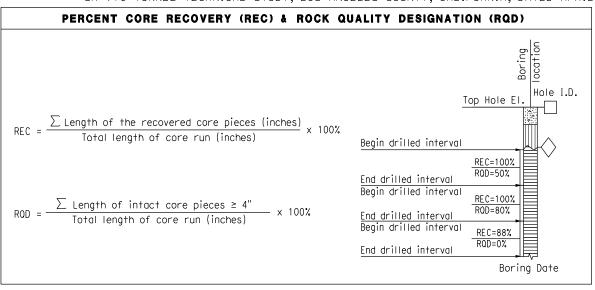
	PARTICLE SIZE				
De	scription	Size			
Boulder		> 12"			
Cobble		3" to 12"			
Gravel	Coarse	3/4" to 3"			
Graver	Fine	No. 4 to 3/4"			
	Coarse	No. 10 to No. 4			
Sand	Medium	No. 40 to No. 10			
	Fine	No. 200 to No. 40			

ENGINEERING SERVICES	GEOTECHNICAL SERVICES	PREPARED FOR THE STATE OF	DIVISION OF ENGINEERING SERVICES	BRIDGE NO.			SOIL	LEGEN	D		
PREPARED BY	KATHLEEN REYES	CALIFORNIA		POST MILES							
CHECKED BY	DAN JANKLY	DEPARTMENT OF TRANSPORTATION	DESIGN BRANCH	N/A	LOG	OF	TEST	BORING	S 2	OF	2
GS LOTB SOIL LEGEND	ORIGINAL SCALE IN INCHES FOR REDUCED PLANS		CU E A 07-187900	DISREGARD PRINT EARLIER REVISIO				REVISION DATES		SHEE	T OF

FILE => \$REQUEST

TIME PLOTTED =

REFERENCE: CALTRANS SOIL & ROCK LOGGING, CLASSIFICATION, AND PRESENTATION MANUAL (JUNE 2007), EXCEPT AS NOTED IN APPENDIX A.1 OF THE FINAL GEOTECHNICAL SUMMARY REPORT, SR-710 TUNNEL TECHNICAL STUDY, LOS ANGELES COUNTY, CALIFORNIA, DATED APRIL, 2010.



RELATIVE ST	RENGTH OF INTACT ROCK
Term	Uniaxial Compressive Strength (PSI)
Extremely Strong	> 30,000
Very Strong	14,500 - 30,000
Strong	7,000 - 14,500
Medium Strong	3,500 - 7,000
Weak	700 - 3,500
Very Weak	150 - 700
Extremely Weak	< 150

BEDDING	SPACING
Description	Thickness / Spacing
Massive	Greater than 10 ft
Very thickly bedded	3 to 10 ft
Thickly bedded	1 to 3 ft
Moderately bedded	3-5/8" to 1 ft
Thinly bedded	1-1/4" to 3-5/8"
Very thinly bedded	3/8" to 1-1/4"
Laminated	Less than 3/8"

WEATHERING DESCRIPTORS FOR INTACT ROCK

DIST	COUNTY	ROUTE	POST MILES TOTAL PROJECT	SHEET No	TOT SHEE
7	LA	710	N/A		
PLA The St	not be respons	AL DATE	MAI RA No. G	HESWARAN VEENDRA E 2743 6-30-11 F CALIFORN	ENG INEER
6 H	M HILL UTTON CEN TA ANA, C	NTRE DRIVE A 92707	, SUITE 700		
_					
LE	GEND O	F ROCK	MATERIALS		
	I	GNEOUS RO	СК		
		SEDIMENTAR	Y ROCK		
		METAMORPH:	C ROCK		

FAULT GOUGE

	ROCK HARDNESS
Description	Criteria
Extremely Hard	Specimen cannot be scratched with a pocket knife or sharp pick; can only be chipped with repeated heavy hammer blows.
Very Hard	Specimen cannot be scratched with a pocket knife or sharp pick. Breaks with repeated heavy hammer blows.
Hard	Specimen can be scratched with a pocket knife or sharp pick with difficulty (heavy pressure). Heavy hammer blows required to break specimen.
Moderately Hard	Specimen can be scratched with pocket knife or sharp pick with light or moderate pressure. Core breaks with moderate hammer pressure.
Moderately Soft	Specimen can be grooved 1/6" deep with a pocket knife or sharp pick with moderate or heavy pressure. Breaks with light hammer blow or heavy manual pressure.
Soft	Specimen can be grooved or gouged easily by a pocket knife or sharp pick with light pressure, can be scratched with fingernail. Breaks with light to moderate manual pressure.
Very Soft	Specimen can be readily indented, grooved or gouged with fingernail, or carved with a pocket knife. Breaks with light manual pressure.

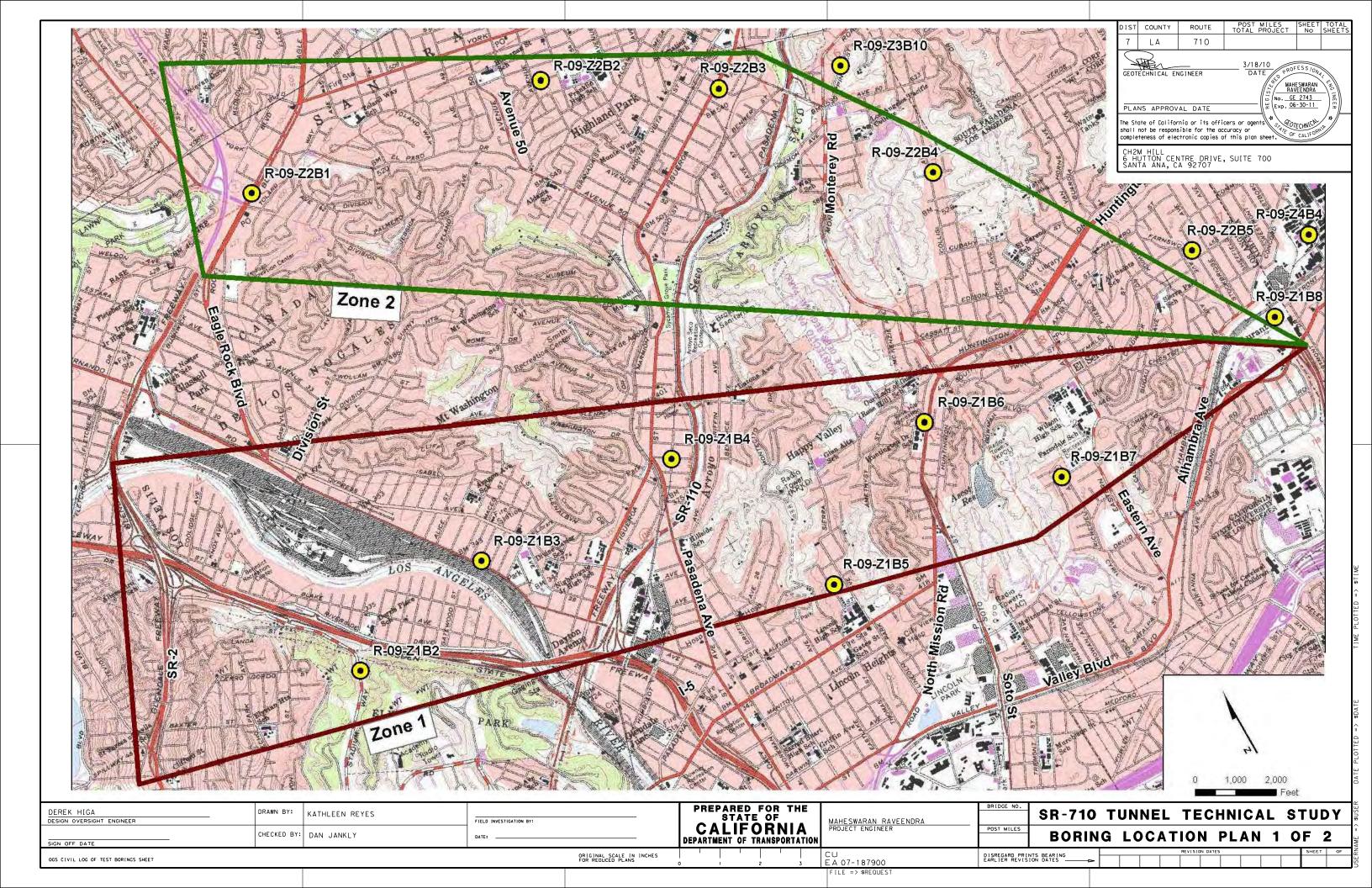
Description	Observed Fracture Density
Unfractured	No fractures.
Very slightly fractured	Lengths greater than 3 feet.
Slightly fractured	Lengths from 1 to 3 feet with few lengths less than 1 foot or greater than 3 feet.
Moderately fractured	Lengths mostly in 4" to 1 foot range with most lengths about 8"
Intensely fractured	Lengths average from 1 to 4" with scattered fragmented intervals with lengths less than 4"
Very intensely fractured	Mostly chips and fragments with a few scattered short core lengths.

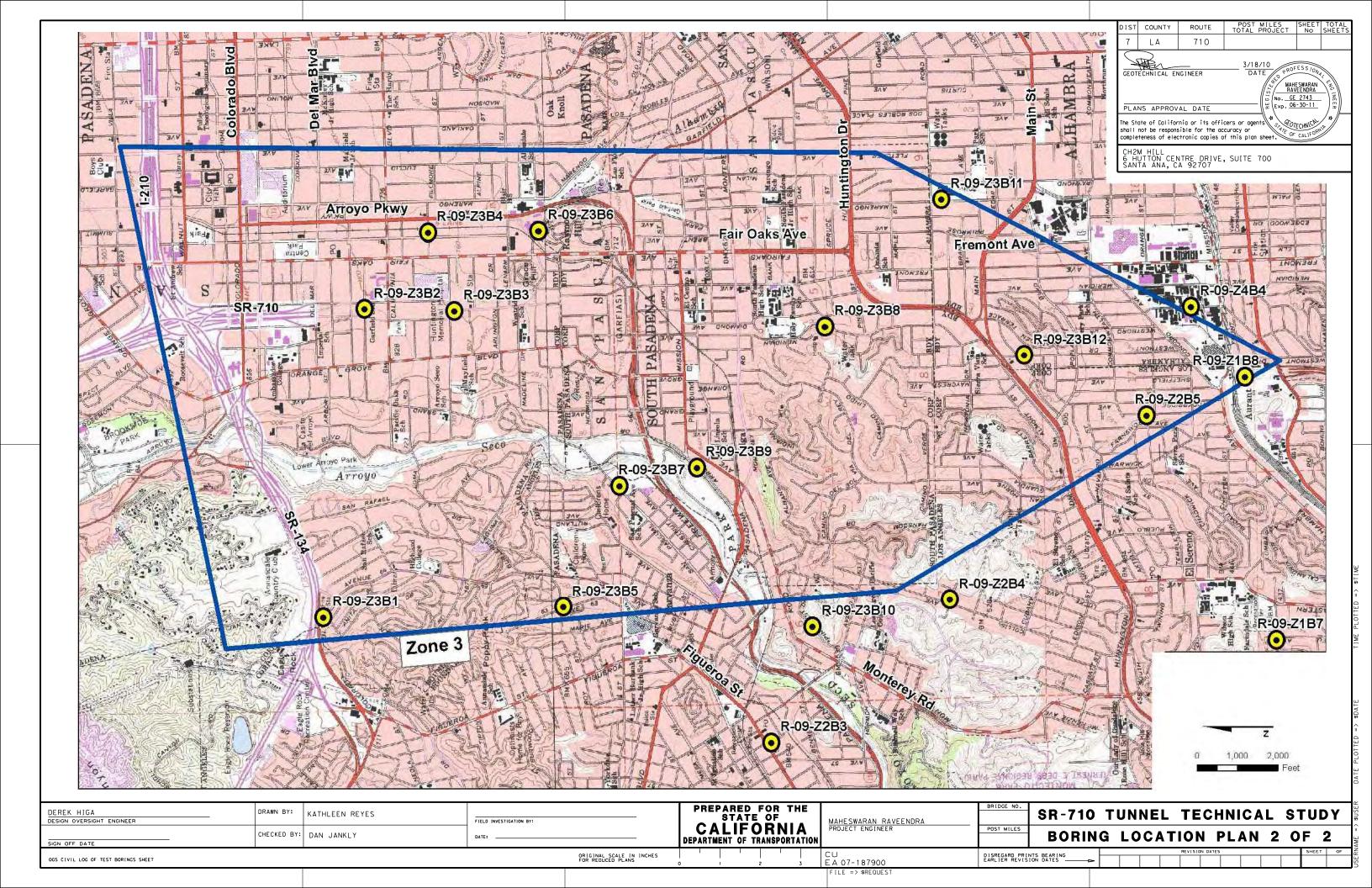
descriptors may be combined.

		Diagr	nostic features			
Description	Chemical Weathering and/or oxid	-Discoloration dation	Mechanical Weathering- Grain boundary condi-	Texture a	nd Solutioning	General Characteristics
	Body of Rock	Fracture Surfaces	tions (disaggregation) primarily for granitics and some coarse-grained sediments	Texture	Solutioning	General Characteristics
Fresh	No discoloration, not oxidized.	No discoloration or oxidation.	No separation, intact (tight).	No change.	No solutioning.	Hammer rings when crystalline rocks are struck.
Slightly Weathered	Discoloration or oxidation is limited to surface of, or short distance from, fractures; some feldspar crystals are dull.	Minor to complete discoloration or oxidation of most surfaces.	No visible separation, intact (tight).	Preserved.	Minor leaching of some solu- ble minerals may be noted.	Hammer rings when crystalline rocks are struck. Body of rock not weakened.
Moderately Weathered	Discoloration or oxidation extends from fractures usually throughout; Fe-Mg minerals are "rusty," feldspar crystals are "cloudy."	All fracture surfaces are discolored or oxidized.	Partial separation of boundaries visible.	Generally preserved.	Soluble min- erals may be mostly leached.	Hammer does not ring when rock is struck. Body of rock is slightly weakened.
Intensely Weathered	Discoloration or oxidation throughout; all feldspars and Fe-Mg minerals are altered to clay to some extent; or chemical alteration produces in-situ disaggregation, see grain boundary conditions.	All fracture surfaces are discolored or oxidized, surfaces friable.	Partial separation, rock is friable; in semiarid conditions granitics are disaggregated.	Texture altered by chemical disintegra- tion (hy- dration, argillation).	Leaching of soluble min- erals may be complete.	Dull sound when struck with hammer, usually can be broken with moderate to heavy manual pressure or by light hammer blow without reference to planes of weakness such as incipient or hairline fractures, or veinlets. Rock is significantly weakened.
Decomposed	Discolored or oxidized throughout, but resistant minerals such as quartz may be unaltered; all feldspars and Fe-Mg minerals are completely altered to clay.		Complete separation of grain boundaries (disaggregated).	Resembles a s or complete i structure ma leaching of s minerals usuc	remnant rock y be preserved; oluble	Can be granulated by hand. Resistant minerals such as quartz may be present as "stringers" or "dikes."

Combination descriptors (such as "slightly weathered to fresh") are permissible where equal distribution of both weathering characteristics is present over significant intervals or where characteristics present are "in between" the diagnostic feature. However, combination descriptors should not be used where significant, identifiable zones can be delineated. Only two adjacent descriptors may be combined. "Very intensely weathered" is the combination descriptor for "intensely weathered to decomposed."

ENGINEERING SERVICES	GEOTECHNICAL SERVICES	PREPARED FOR THE State of	DIVISION OF ENGINEERING SERVICES	BRIDGE NO.		ROCK L	EGEND	USER
PREPARED BY	KATHLEEN REYES	CALIFORNIA	STRUCTURE DESIGN	POST MILES				
CHECKED BY	DAN JANKLY	DEPARTMENT OF TRANSPORTATION	DESIGN BRANCH	N/A	LO	G OF TES	T BORINGS	AME
GS LOTB SOIL LEGEND	OR[G]NAL SCALE IN INCHES FOR REDUCED PLANS	0 1 2 3	CU EA 07-187900	DISREGARD PRINT EARLIER REVISIO	S BEARING N DATES	REVISIO	N DATES	SHEET OF NA LINE





NOTES: BENCHMARK: ΙΔ 710 1) This LOTB sheet (Boring Record) was prepared in accordance with the Caltrans Soil and Rock Logging, Classification and Presentation Manual (June 2007) except as noted in Appendix A.1 of Gody A. Castle 3/18/09 NGS Benchmark used. Pt MF 36F: N1847799.06, E6512460.75, Elevation 407.29 the Final Geotechnical Summary Report SR-710 Tunnel Technical Study Los Angeles County, PROFESSIONAL GEOLOGIST MF36F: A 3" Brass disk stamped "Metropolitan Water Deistrict of Southern California MF 36F 1989, California dated (April, 2010). set in the top of the bridge curb at the northwest corner of the Hellman Avenue overcrossing of the Long Beach Freeway (I-710). 68' west of the center of the south bound lanes of the freeway, 25' north of the centerline of Hellman Avenue, 13' east of the west end of the bridge, 1' north of the J. CASTLE No. PG 8162 Exp. 12-31-10 2) HQ&PQ core samplers were used to obtain soil and rock samples. PLANS APPROVAL DATE north curb face. NGS PID EW9453. THE STATE OF CALIFORNIA OR ITS OFFICERS
OR AGENTS SHALL NOT BE RESPONSIBLE FOR
THE ACCURACY OR COMPLETENESS OF ELECTRONIC
COPIES OF THIS PLAN SHEET. 3) Boring is inclined at 60° from Horizontal, bedding and structural measurements are relative to core axis, depth is down hole distance. Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. Vertical control based on North American Vertical Datum 1988. Pt 0153: N1845410.50, E6509860.21 Pt 0617: N1858044.30, E6491094.23 4) Plan view shown on Boring Location Plan sheet 1 of 2. STATE OF CALIFORNIA DEPARTMENT OF TRANSPORTATION, DISTRICT-7 100 S. MAIN STREET, LOS ANGELÉS, CA 90012 EARTH MECHANICS, INC. 17660 NEWHOPE STREET, SUITE E FOUNTAIN VALLEY, CA 92708 Units are in U.S survey feet. R-09-Z1B2 N 1854817.64 E 6488834.09 6'' EL. 498.9 ASPHALT 6" thick.
Road Base; 12" thick; silty angular gravel.
Poorly graded SAND (SP); yellow to brown; moist; trace coarse to fine GRAVEL; fine SAND. REC=67% SEDIMENTARY ROCK (SANDTONE); fine-grained; thickly bedded to massive; yellow to brown; intensely weathered; very soft: (Poorly graded SAND-SP; light brown, moist, about 5% coarse sand and fine angular GRAVEL, max.  $\frac{1}{2}$ " dia.) [PUENTE FORMATION].

Subangular GRAVEL, max. 2" dia.; fine SAND, decomposed siltstone fragment. 130 ¬ REC=98% REC=73% FeO lined bedding joint. REC=92% Minor faults inclined 75°, sand filling. REC=42% Tones of very soft and moderately hard sandstone,  $\frac{1}{2}$  to 2'thick between 135'-141' depth. Soft, yellowish brown to gray, subangular GRAVEL, max. 1.5" dia.; coarse to fine SAND, decomposed REC=100% RQD=23% REC=4% siltstone fragments. 140-REC=90% ~Laminations of shale, gray, decomposed. REC=40% SEDIMENTARY ROCK: (SANDSTONE with SILTSTONE), fine-grained to MUNICIO SEDIMENTARY ROCK: (SANDSTONE WITH SILISTONE), THE GLOWES TO dark gray, .... REC=100 REC=70% SEDIMENTARY ROCK: (SANDSTONE), fine grained, laminated, gray, decomposed, very soft, trace of RQD=48% 150-SILTSTONE laminations. moderately weathered to fresh, intensely to slightly fractured, CA)UW(SA) 30 -REC=95% ∼Mottled yellowish brown, strong brown, and gray, trace of shale fragments max. 1" dia. sandstone 85%. REC=75% RQD=60% (CADUW)(SA) `Becomes light gray, hard, fresh zone 1.5' thick. Becomes brown to yellowish brown oxidation 1 .75" thick on each side of joint, inclination 25°. REC=77% ¬Very soft, highly oxidized. REC=73% RQD=10% 160 40 -REC=98% —Bedding inclination 66°. Lecomes moderately soft, slightly fractured, bedding inclination 25°. Bedding inclination 65°. REC=42% REC=100% Trine-grained SANDSTONE, varies from yellowish brown to strong brown. REC=72% RQD=17% 170-REC=100 CAJUWENUC fractured. Trace of shale and sandstone fragments to 1.5" dia. REC=82% RQD=90% Concretion, fine SANDSTONE, light gray to yellowish brown, slightly weathered, hard, strong reaction to HCL. Laminations inclined 75°. ——SANDSTONE with trace SILTSTONE, laminated, gray to light gray, decomposed, very soft, possible bedding inclination 90°. REC=100 프 180-REC=92% RQD=70% SILTSTONE laminations, gray, slightly weathered, soft, bedding and bedding plane parting inclined 75° to 80°, soft sediment Decomposed SHALE laminations, inclined 0°. SANDSTONE, fine-grained, yellowish brown to gray, decomposed, trace of SHALE fragments 60 REC=100% REC=53% deformation and worm burrows.

— Becomes gray, slightly weathered to fresh.
Siltstone/sandstone lamination, 4" thick. RQD=47% up to 1/4" dia.

— Thin bedding inclined 63°. REC=73% REC=88% RQD=38% SILTSTONE and SANDSTONE, laminated to thinly bedded, fine-grained, dark gray, 190 (M)(W) -Bedding inclined 60°. 70 -REC=100 very soft, decomposed. REC=65% RQD=63% Intensely weathered, moderately hard. opBedding inclination 90 $^\circ$ , joint inclination 25 $^\circ$ . REC=100 SEDIMENTARY ROCK: (SANDSTONE), fine-grained to medium-grained, moderately bedded, gray, fresh, hard, strong reaction to HCL, concretion.

Bedding inclined 75°, joint inclined 80°.

SEDIMENTARY ROCK (SILTSTONE/CLAYSTONE), fine-grained, laminated siltstone, thinly to thickly bedded, dark gray to very dark gray, fresh, soft, moderately fractured, bedding inclined 75°- 90°, sandstone 50% interbedded with siltstone 50%.

Siltstone rip-up fragments.

Joint inclined 20°.

SEDIMENTARY ROCK (SANDSTONE with SILTSTONE), fine grained to medium-grained, laminated to thinly bedded, gray to light gray, fresh, soft to moderately hard, slightly to moderately fractured, 90% of Sandstone, trace of hard zone, bedding and bedding plane parting inclined 70°- 90°.

Siltstone laminations inclined 60°. REC=97% RQD=53% 200 REC=97% 80 RQD=77% REC=78% REC=87% ∼Grass and wood pieces. Very dark gray to black, moist, medium plasticity. RQD=67% REC=100% MUWPA REC=92% —\_Wood fragment. REC=100% ROD=55% RQD=43% REC=92% Medium dense, yellowish brown, moist, about 81% coarse to fine SAND, about 19% nonplastic fines. ROD=55% REC=83% ROD=33% REC=87% RCD=2%  $\longrightarrow$  Joint inclined 80 $^\circ.$ REC=100% Moderately weathered, oxidized zone. 100 —Becomes SILTSTONE laminations, bedding inclination 90°. REC=95% SILTSTONE laminations inclined 85°. Siltstone laminations inclined 60°. REC=100% MUNUC — Joint inclinations 30°, slight HCL reaction along joint plane. Graded bedding, coarse to fine-grained zone. Joints inclined 30° to 35°, rough surfaces. REC=80% Becomes reddish yellow. 230-110 Soft zone, joints inclined 35°, weak HCL reaction, slightly rough CADUW(EM)UC)  $\overline{\phantom{a}}$ SILTSTONE/SHALE laminations, light gray, inclination 80°- 90°. REC=85% to moderately rough surfaces. SEDIMENTARY ROCK: (SILTSTONE and SANDSTONE), fine-grained, laminated to thickly bedded, dark yellowish brown and very dark gray, slightly weathered, soft to very soft, bedding and bedding plane parting inclined 70° to 90°. Hard cemented zone, calcite cement, healed calcite, joints inclined 35°. REC=72% Siltstone laminations inclined 70°, coarse to medium-grained zone. Joint inclined 20°. 240-RQD=23% 120-Joint inclined 15°, polished surface, 0.1" aperture, filled with clay.

| MUW SDUD Faults inclined 80° - 90°, partially healed, 1/3" displacements. REC=100% SEDIMENTARY ROCK: (SANDSTONE with SILTSTONE and CLAYSTONE), fine-grained to medium-grained, thinly bedded to massive, dark gray, intensely weathered, soft to moderately soft, intensely to slightly fractured, bedding inclined 70° to 90°, 80% sandstone. RQD=100% 130-**PROFILE** See Sheet 2 FOR CONTINUATION VERTICAL 1" = 10' BRIDGE NO. PREPARED FOR THE DEREK HIGA B. SCHELL, J. CASTLE SR-710 TUNNEL TECHNICAL STUDY DRAWN BY: K. THANT LAW N/A STATE OF DESIGN OVERSIGHT ENGINEER FIELD INVESTIGATION BY: **CALIFORNIA** PROJECT ENGINEER POST MILE 3/25/09 - 4/3/09 LOG OF TEST BORING SHEET 1 OF 2 CHECKED BY B. SCHELL N/A DEPARTMENT OF TRANSPORTATION OGS CIVIL LOG OF TEST BORINGS SHEET ORIGINAL SCALE IN INCHES FOR REDUCED PLANS DISREGARD PRINTS BEARING EARLIER REVISION DATES EA 07-187900

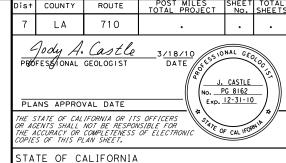
NGS Benchmark used. Pt MF 36F: N1847799.06, E6512460.75, Elevation 407.29 MF36F: A 3" Brass disk stamped "Metropolitan Water Deistrict of Southern California MF 36F 1989, set in the top of the bridge curb at the northwest corner of the Hellman Avenue overcrossing of the Long Beach Freeway (I-710). 68' west of the center of the south bound lanes of the freeway, 25' north of the centerline of Hellman Avenue, 13' east of the west end of the bridge, 1' north of the north curb face. NGS PID EW9453.

Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. Vertical control based on North American Vertical Datum 1988. Pt 0153: N1845410.50, E6509860.21 Pt 0617: N1858044.30, E6491094.23

Units are in U.S survey feet.

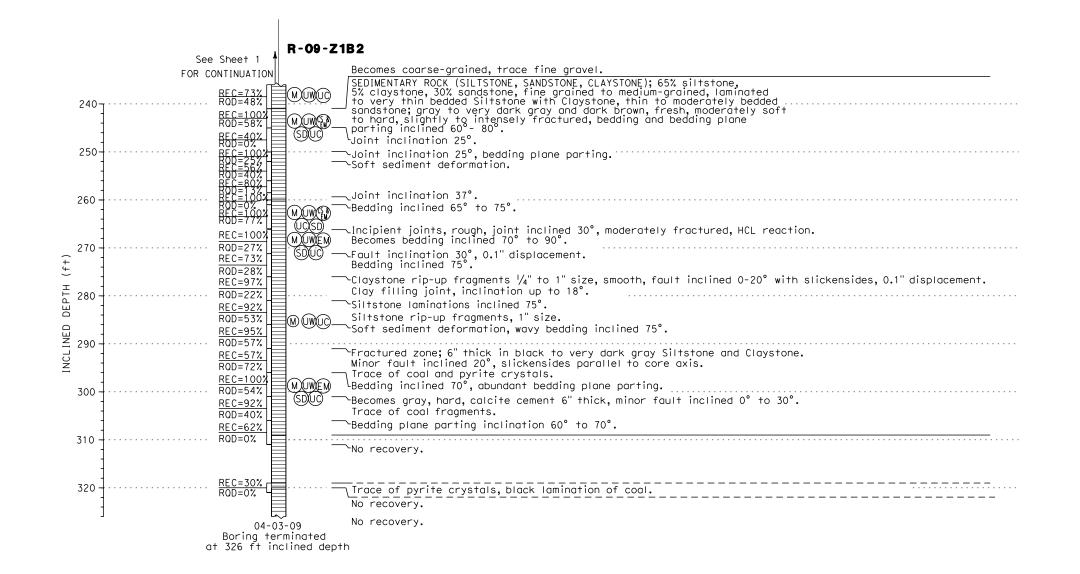
#### NOTES:

- 1) This LOTB sheet (Boring Record) was prepared in accordance with the Caltrans Soil and Rock Logging, Classification and Presentation Manual (June 2007) except as noted in Appendix A.4 of the Final Geotechnical Summary Report SR-710 Tunnel Technical Study Los Angeles County, California dated (April, 2010).
- 2) HQ&PQ core samplers were used to obtain soil and rock samples.
- 3) Boring is inclined at  $60^{\circ}$  from Horizontal, bedding and structural measurements are relative to core axis, depth is down hole distance.
- 4) Plan view shown on Boring Location Plan sheet 1 of 2.



STATE OF CALIFORNIA
DEPARTMENT OF TRANSPORTATION, DISTRICT-7
100 S. MAIN STREET, LOS ANGELES, CA 90012

EARTH MECHANICS, INC. 17660 NEWHOPE STREET, SUITE E FOUNTAIN VALLEY, CA 92708



PROFILE
VERTICAL 1" = 10'

DEREK HIGA DESIGN OVERSIGHT ENGINEER	DRAWN BY:	K. THANT	B. SCHELL, J. CASTLE  FIELD INVESTIGATION BY:	PREPARED FOR THE STATE OF	H. LAW	BRIDGE NO.	SR-71	O TUNNE	L TECHN	ICAL	STUDY	
SIGN OFF DATE	CHECKED BY:	B. SCHELL	3/25/09 - 4/3/09 DATE:	CALIFORNIA DEPARTMENT OF TRANSPORTATION	PROJECT ENGINEER	POST MILE N/A	LOG 0	F TEST I	BORING S	HEET	2 OF 2	1
OGS CIVIL LOG OF TEST BORINGS SHEET			ORIGINAL SCALE IN INCHES FOR REDUCED PLANS	0 1 2 3	CU . EA 07-187900	DISREGARD PR EARLIER REVI		REVISION DAT	ES (PRELIMINARY STAGE ONLY)		SHEET 0	<u>.                                    </u>

NGS Benchmark used. Pt MF 36F: N1847799.06, E6512460.75, Elevation 407.29 MF36F: A 3" Brass disk stamped "Metropolitan Water Deistrict of Southern California MF 36F 1989, set in the top of the bridge curb at the northwest corner of the Hellman Avenue overcrossing of the Long Beach Freeway (1-710). 68' west of the center of the south bound lanes of the freeway, 25' north of the centerline of Hellman Avenue, 13' east of the west end of the bridge, 1' north of the north curb face. NGS PID EW9453.

Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. Vertical control based on North American Vertical Datum 1988. P+ 0153: N1845410.50, E6509860.21 P+ 0617: N1858044.30, E6491094.23

Units are in U.S survey feet.

DEREK HIGA

DESIGN OVERSIGHT ENGINEER

OGS CIVIL LOG OF TEST BORINGS SHEET

SIGN OFF DATE

DRAWN BY

CHECKED BY

K. THANT

B. SCHELL

#### NOTES:

- 1) This LOTB sheet (Boring Record) was prepared in accordance with the Caltrans Soil and Rock Logging, Classification and Presentation Manual (June 2007) except as noted in Appendix A.1 of the Final Geotechnical Summary Report SR-710 Tunnel Technical Study Los Angeles County, California dated (April, 2010).
- 2) California ring, standard penetration test, and HQ&PQ core samplers were used to obtain soil and rock samples.
- 3) An automatic trip hammer system consisting of a hammer weight of 140 lbs falling a distance of 30" was used to advance the drive samplers.
- 4) To convert from California ring sampler blow count to equivalent standard penetration test sampler blow count, for granular and cohesive soil, multiply by 0.67.
- 5) All bedding and other structural angles are measured from horizontal.
- 6) Plan view shown on Boring Location Plan sheet 1 of 2.

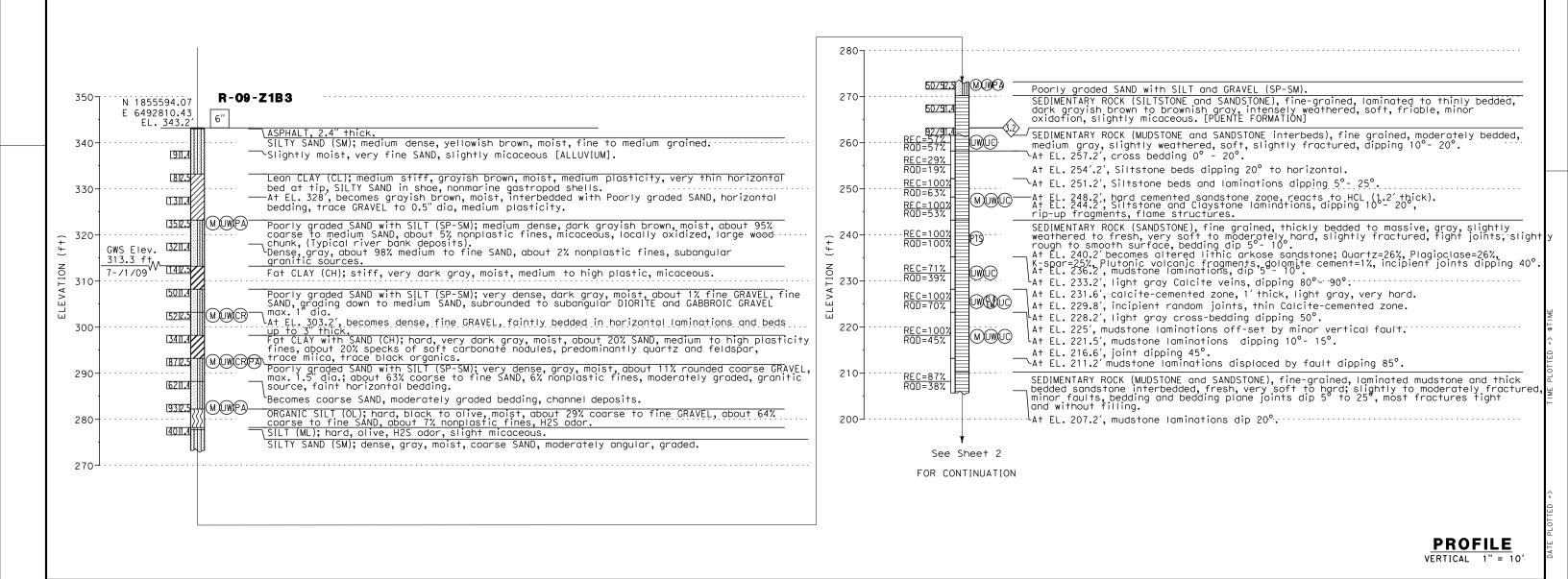
Dist	COUNTY	ROUTE	POST MILES TOTAL PROJECT	SHEET No.	TOTAL					
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PR/O	FESS ONAL		No F	G 8162	_)					
PLANS APPROVAL DATE  THE STATE OF CALIFORNIA OR ITS OFFICERS OF AGENTS SHALL NOT BE RESPONSIBLE FOR THE ACCURACY OR COMPLETENESS OF ELECTRONIC										
STA		ALIFORNIA		STRIC						

DEPARTMENT OF TRANSPORTATION, DISTRICT-7 100 S. MAIN STREET, LOS ANGELÉS, CA 90012

EARTH MECHANICS, INC. 17660 NEWHOPE STREET, SUITE E FOUNTAIN VALLEY, CA 92708

SR-710 TUNNEL TECHNICAL STUDY

LOG OF TEST BORING SHEET 1 OF 2



B. SCHELL, J. CASTLE

ORIGINAL SCALE IN INCHES FOR REDUCED PLANS

FIELD INVESTIGATION BY:

1/23/09 - 1/31/09

PREPARED FOR THE

PROJECT ENGINEER

STATE OF

CALIFORNIA

DEPARTMENT OF TRANSPORTATION DISREGARD PRINTS BEARING EARLIER REVISION DATES EA 07-187900 FILE => \$REQUEST

BRIDGE NO.

N/A

N/A

POST MILE

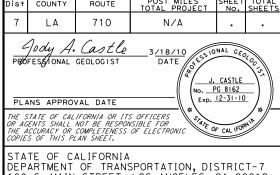
NGS Benchmark used. Pt MF 36F: N1847799.06, E6512460.75, Elevation 407.29 MF36F: A 3" Brass disk stamped "Metropolitan Water Deistrict of Southern California MF 36F 1989, set in the top of the bridge curb at the northwest corner of the Hellman Avenue overcrossing of the Long Beach Freeway (I-710). 68' west of the center of the south bound lanes of the freeway, 25' north of the centerline of Hellman Avenue, 13' east of the west end of the bridge, 1' north of the north curb face. NGS PID EW9453.

Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. Vertical control based on North American Vertical Datum 1988. Pt 0153: N1845410.50, E6509860.21 P+ 0617: N1858044.30, E6491094.23

Units are in U.S survey feet.

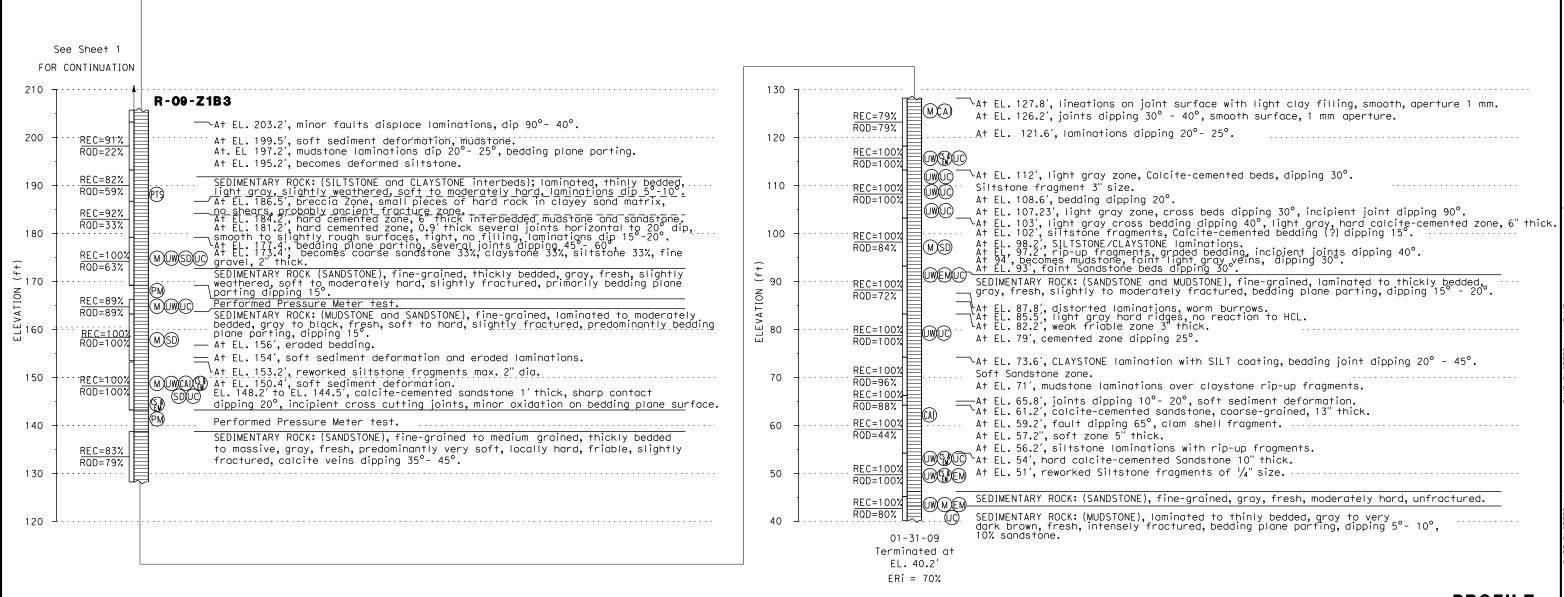
#### NOTES:

- 1) This LOTB sheet (Boring Record) was prepared in accordance with the Caltrans Soil and Rock Logging, Classification and Presentation Manual (June 2007) except as noted in Appendix A.1 of the Final Geotechnical Summary Report SR-710 Tunnel Technical Study Los Angeles County, California dated (April, 2010).
- 2) California ring, standard penetration test, and HQ&PQ core samplers were used to obtain soil and rock samples.
- 3) An automatic trip hammer system consisting of a hammer weight of 140 lbs falling a distance of 30" was used to advance the drive samplers.
- 4) To convert from California ring sampler blow count to equivalent standard penetration test sampler blow count, for granular and cohesive soil, multiply by 0.67.
- 5) All bedding and other structural angles are measured from horizontal.
- 6) Plan view shown on Boring Location Plan sheet 1 of 2.



100 S. MAIN STREET, LOS ANGELÉS, CA 90012

EARTH MECHANICS, INC. 17660 NEWHOPE STREET, SUITE E FOUNTAIN VALLEY, CA 92708



**PROFILE** VERTICAL 1" = 10"

DEREK HIGA DESIGN OVERSIGHT ENGINEER	DRAWN BY:	K. THANT	B. SCHELL, J. CASTLE FIELD INVESTIGATION BY:	PREPARED FOR THE STATE OF	H. LAW	BRIDGE NO.	SR-7	10	TUNNI	EL T	ECHN	IICAL	ST	UDY
SIGN OFF DATE	CHECKED BY:	B. SCHELL	1/23/09 - 1/31/09 DATE:	CALIFORNIA DEPARTMENT OF TRANSPORTATION	PROJECT ENGINEER	POST MILE N/A	LOG	OF	TEST	BOR	ING S	HEET	2 (	)F 2
OGS CIVIL LOG OF TEST BORINGS SHEET			ORIGINAL SCALE IN INCHES FOR REDUCED PLANS		CU . EA 07-187900	DISREGARD PR EARLIER REVI	INTS BEARING SION DATES		REVISION (	DATES (PRELIMIN	NARY STAGE ONLY		s	2 2

NGS Benchmark used. Pt MF36F: N1847799.06, E6512460.75, Elevation 407.29' MF 36F: A 3" Brass disk stamped "Metropolitan Water District of Southern California MF 36F 1989", set in the top of the bridge curb at the northwest corner of the Hellman Avenue overcrossing of the Long Beach Freeway (I-710). 68' west of the center of the south bound lanes of the freeway, 25' north of the centerline of Hellman Avenue, 13' east of the west end of the bridge, 1' north of the north curb face. NGS PID EW9453.

Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. Vertical control based on NAVD-88. Pt 0153 N1845410.50, E6509860.21, Pt 0617 N1858044.3, E6491094.23.

Units are in U.S. survey feet.

NOTES:

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- 4) To convert from California ring sampler blow count to equivalent standard penetration test sampler blow count, for granular and cohesive soil, multiply by 0.67.

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- 5) All bedding and other structural angles are measured from horizontal.
- 6) Plan view shown on Boring Location Plan sheet 1 of 2.

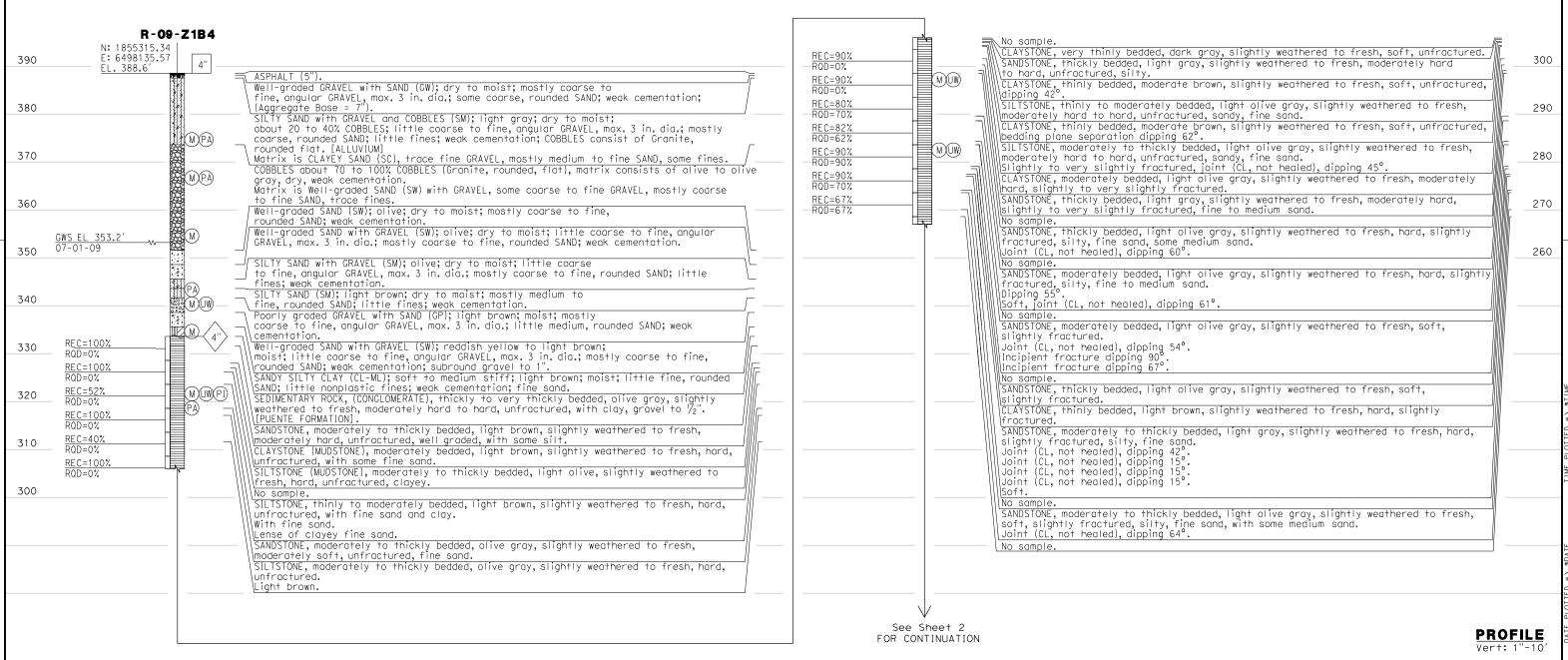
DIST COUNTY ROUTE POST MILES SHEET TOTAL SHEETS

7 LA 710 N/A

CERTIFIED ENGINEERING GEOLOGIST DATE

PLANS APPROVAL DATE

The State of California or its officers or agents shall not be responsible for the accuracy or completeness of electronic copies of this plan sheet.



**GEOTECHNICAL SERVICES ENGINEERING SERVICES** DIVISION OF ENGINEERING SERVICES SR-710 TUNNEL TECHNICAL STUDY STATE OF N/A STRUCTURE DESIGN UNCTIONAL SUPERVISOR FIELD INVESTIGATION BY: K. BARKER/ M. SALISBURY DRAWN BY: F. MIGUEL CALIFORNIA POST MILES LOG OF TEST BORING SHEET 1 OF 3 NAME: SHIVA KARIMI DATE: 2/12/09 - 2/27/09 DEPARTMENT OF TRANSPORTATION **DESIGN BRANCH** CHECKED BY: J. PRATT ORIGINAL SCALE IN INCHES DISREGARD PRINTS BEARING EARLIER REVISION DATES OGS CIVIL LOG OF TEST BORINGS SHEET EA 07-187900

NGS Benchmark used. Pt MF36F: N1847799.06, E6512460.75, Elevation 407.29' MF 36F: A 3" Brass disk stamped "Metropolitan Water District of Southern California MF 36F 1989", set in the top of the bridge curb at the northwest corner of the Hellman Avenue overcrossing of the Long Beach Freeway (I-710). 68' west of the center of the south bound lanes of the freeway, 25' north of the centerline of Hellman Avenue, 13' east of the west end of the bridge, 1' north of the north curb face. NGS PID EW9453.

Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. Vertical control based on NAVD-88. Pt 0153 N1845410.50, E6509860.21, Pt 0617 N1858044.3, E6491094.23.

Units are in U.S. survey feet.

NOTES:

- This LOTB sheet was prepared in accordance with the Caltrans Soil and Rock Logging, Classification and Presentation Manual (June 2007) except as noted in Appendix A.1 of the Final Geotechnical Summary Report SR-710 Tunnel Technical Study Los Angeles County, California dated (April, 2010).
- 2) California ring, standard penetration test, and HO&PQ core samplers were used to obtain soil and rock samples.
- 3) An automatic trip hammer system consisting of a hammer weight of 140 lbs falling a distance of 30" was used to advance the drive samplers.
- 4) To convert from California ring sampler blow count to equivalent standard penetration test sampler blow count, for granular and cohesive soil, multiply by 0.67.
- 5) All bedding and other structural angles are measured from horizontal.
- 6) Plan view shown on Boring Location Plan sheet 1 of 2.

DIST COUNTY ROUTE POST MILES SHEET TOTAL PROJECT NO SHEETS

7 LA 710 N/A

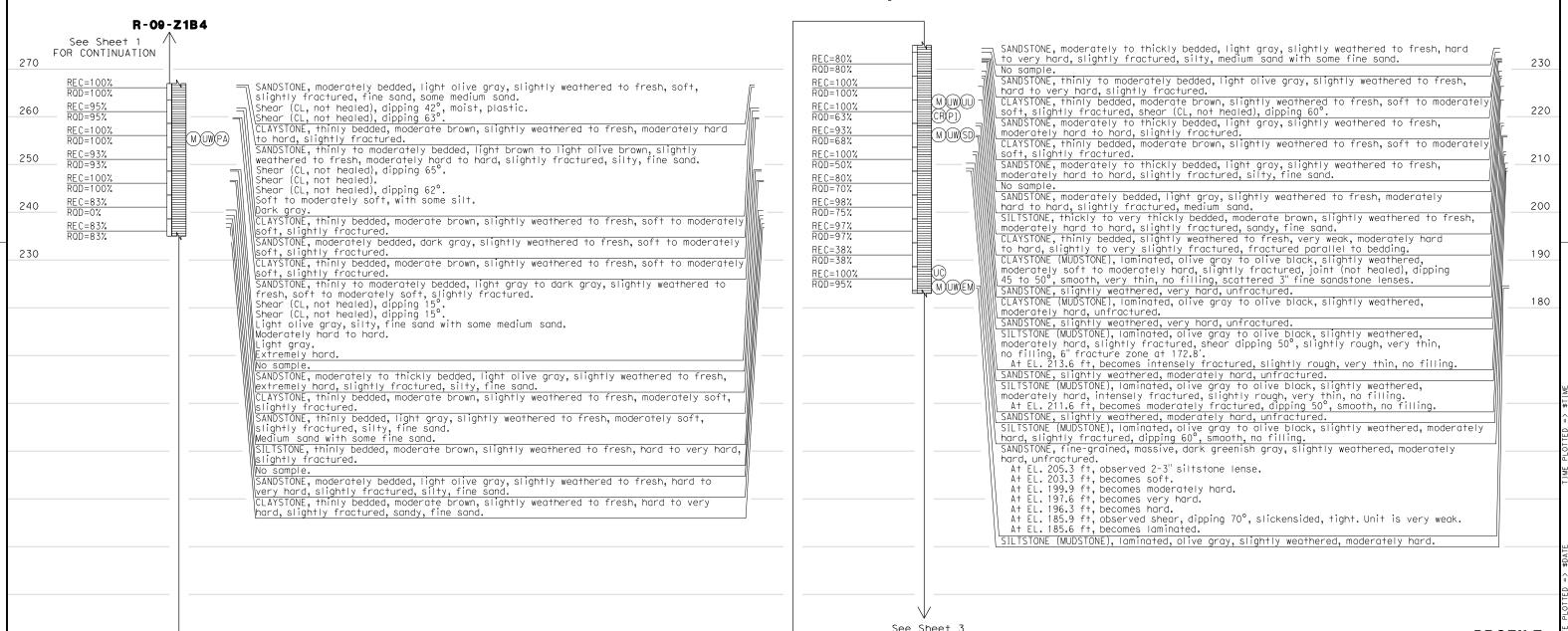
CERTIFIED ENGINEERING GEOLOGIST DATE

CERTIFIED ENGINEERING GEOLOGIST DATE

PLANS APPROVAL DATE

The State of California or its officers or agents shall not be responsible for the accuracy or completeness of electronic copies of this plan sheet.

**PROFILE** 



**ENGINEERING SERVICES GEOTECHNICAL SERVICES** DIVISION OF ENGINEERING SERVICES SR-710 TUNNEL TECHNICAL STUDY STATE OF N/A STRUCTURE DESIGN TIELD INVESTIGATION BY: K. BARKER/ UNCTIONAL SUPERVISOR DRAWN BY: EDELYNE MIGUEL CALIFORNIA POST MILES M. SALISBURY LOG OF TEST BORING SHEET 2 OF 3 NAME: SHIVA KARIMI DATE: 2/12/09 - 2/27/09 DEPARTMENT OF TRANSPORTATION **DESIGN BRANCH** CHECKED BY: JOSEPH PRATI ORIGINAL SCALE IN INCHES DISREGARD PRINTS BEARING EARLIER REVISION DATES OGS CIVIL LOG OF TEST BORINGS SHEET EA 07-187900

FILE => \$REQUEST

FOR CONTINUATION

#### COUNTY **BENCHMARK:** LΑ 710 NGS Benchmark used. Pt MF36F: N1847799.06, E6512460.75, Elevation 407.29' NOTES: Barber Kristonh 3/18/10 1) This LOTB sheet was prepared in accordance with the Caltrans Soil and MF 36F: A 3" Brass disk stamped "Metropolitan Water District of Southern California CERTIFIED ENGINEERING GEOLOGIST DATE MF 36F 1989", set in the top of the bridge curb at the northwest corner of the Hellman Rock Logging, Classification and Presentation Manual (June 2007) except as noted in Appendix A.1 of the Final Geotechnical Summary Report Avenue overcrossing of the Long Beach Freeway (I-710). 68' west of the center of the south bound lanes of the freeway, 25' north of the centerline of Hellman Avenue, 13' SR-710 Tunnel Technical Study Los Angeles County, California dated (April, 2010). east of the west end of the bridge, 1'north of the north curb face. NGS PID EW9453. PLANS APPROVAL DATE 2) California ring, standard penetration test, and HQ&PQ core samplers Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. The State of California or its officers or agents shall not be responsible for the accuracy or Vertical control based on NAVD-88. were used to obtain soil and rock samples. completeness of electronic copies of this plan sheet P+ 0153 N1845410.50, E6509860.21, 3) An automatic trip hammer system consisting of a hammer weight of 140 lbs Pt 0617 N1858044.3, E6491094.23. falling a distance of 30" was used to advance the drive samplers. Units are in U.S. survey feet. 4) To convert from California ring sampler blow count to equivalent standard penetration test sampler blow count, for granular and cohesive soil, multiply 5) All bedding and other structural angles are measured from horizontal. 6) Plan view shown on Boring Location Plan sheet 1 of 2. R-09-Z1B4 See Sheet 2 FOR CONTINUATION SANDSTONE, fine-grained, laminated, dark greenish gray, slightly weathered, weak, REC=100% moderately hard. At EL. 181.9 ft. observed 4" fracture zone. / SILTSTONE (MUDSTONE), massive, dark greenish gray, slightly weathered, moderately soft. CLAYSTONE (MUDSTONE), laminated, medium dark gray, slightly weathered, moderately soft, M)UW(SD) intensely to moderately fractured. SANDSTONE, massive, dark greenish gray, slightly weathered, soft, scattered 3" REC=98% RQD=78% REC=70% RQD=30% At EL. 180.1 ft, observed 6" fracture zone. 170 At EL. 180.1 ft, observed to tructure zone. At EL. 179.6 ft, becomes medium gray. At EL. 179.1 ft, observed joint, dipping 45°, polished, very thin, no filling. At EL. 178.6 ft, observed 1' fracture zone. At EL. 177.4 ft, observed shear, dipping 70°, slickensided, 1mm. At EL. 176.9 ft, observed 6" fracture zone. claystone lenses. At EL. 122.1 ft, becomes moderately hard. EL. 119.1 ft, becomes laminated, with thin black bedding. RQD=20% At EL. 118.6 ft, becomes massive, soft to moderately soft. SILTSTONE (MUDSTONE), laminated, olive gray, slightly weathered, moderately hard, unfractured, with scattered 2" lenses of fine sandstone. REC=85% RQD=62% MUWEM 160 At EL. 176.6 ft, observed 6" fracture zone. At EL. 110.6 ft, becomes slightly fractured, shear, dipping 45°, smooth, 1-2 mm. REC=100% RQD=55% REC=100% RQD=50% At EL. 175.9 ft, observed shear, dipping 45°, slickensided, 1mm. CLAYSTONE (MUDSTONE), thinly bedded, olive black, slightly weathered, moderately SANDSTONE, fine-grained, laminated, medium gray, slightly weathered, moderately soft, unfractured, interbedded with dark greenish gray, fine sandstone. soft, unfractured. PTS M UW SD SANDSTONE, dark greenish gray, slightly weathered, moderately soft, unfractured. At EL. 104.1 ft, observed 2" siltstone lense, moderately hard. REC=100% 150 RQD=85% At EL. 173.9 ft, becomes olive black. At EL. 173.6 ft, becomes massive, olive gray, moderately hard, slightly fractured. At EL. 173.1 ft, observed shear, dipping 10°, slickensided. At EL. 171.6 ft, becomes dark greenish gray. SILTSTONE (MUDSTONE), laminated, olive gray, slightly weathered, moderately hard, REC=100% RQD=66% MUWUU SANDSTONE, olive gray, slightly weathered, very hard. SILTSTONE (MUDSTONE), olive gray, slightly weathered, moderately hard, moderately to slightly fractured, joint dipping 40 to 50°, smooth, very thin, no filling. CLAYSTONE (MUDSTONE), fine-grained, massive, olive gray, slightly weathered, 140 RQD=90% moderately soft. SANDSTONE, fine-grained, massive, olive gray, slightly weathered, moderately hard, SANDSTONE, dark greenish gray, slightly weathered, moderately soft, unfractured. RQD=48% SILTSTONE (MUDSTONE), laminated, olive gray, slightly weathered, moderately hard, At EL. 166.6 ft, observed joint, dipping 45°, rough, 1mm, no filling,. SILTSTONE (MUDSTONE), massive, olive gray, slightly weathered, moderately soft to moderately hard, slightly fractured, scattered 1-3" sandstone lenses. At EL. 163.8 ft, observed shear, dipping 50°, slickensided, 1mm. Unit is weak. REC=100% RQD=55% REC=60% RQD=0% 130 MUWSD SANDSTONE, dark greenish gray, slightly weathered, moderately soft, slightly fractured. REC=100% RQD=42% At EL. 92.6 ft, observed joint, dipping 50°, smooth, 1 mm, no filling. SANDSTONE, fine-grained, massive, dark greenish gray, slightly weathered, moderately SILTSTONE (MUDSTONE), massive, olive gray, slightly weathered, moderately soft to REC=100% RQD=30% hard to hard, unfractured. 120 moderately hard, slightly fractured. SILTSTONE (MUDSTONE), olive gray, slightly weathered, moderately hard, unfractured. M)(UW)(EM) At EL. 90.8 ft, observed fracture zone, smooth, very thin, no filling. At EL. 88.8 ft, becomes very hard, shear, dipping 50°, slickensided. At EL. 88.6 ft, becomes moderately hard, dipping 40°, smooth, very thin. SANDSTONE, fine-grained, massive, dark greenish gray, slightly weathered, moderately soft, unfractured. SILTSTONE (MUDSTONE), olive gray, slightly weathered, moderately hard, intensely to moderately fractured, dipping 30 to 45°, smooth, 0-1 mm, no filling and clay. SANDSTONE, massive, olive gray, slightly weathered, moderately hard, unfractured. 02-27-09 Terminated at EL. 63.6′ ER; = 84% CLAYSTONE (MUDSTONE), laminated, olive gray, slightly weathered, moderately hard, slightly fractured, (clay), dipping 40°, smooth, very thin. SANDSTONE, fine-grained to silt, massive, dark greenish gray, slightly weathered, moderately soft, unfractured. At EL. 83.6 ft, observed shear, dipping 60°, smooth, 0.5mm. Unit is very weak. At EL. 82.3 ft, observed shear, dipping 60°, smooth, 0.5mm. At EL. 81.6 ft, observed shear, dipping 10°, 2mm. SILTSTONE (MUDSTONE), olive gray, slightly weathered, moderately hard, moderately to slightly fractured, shear (clay), dipping 50°, slickensided. SANDSTONE, massive, dark greenish gray, slightly weathered, soft to moderately soft, SANDSTONE, fine-grained, massive, dark greenish gray, slightly weathered, soft to moderately soft, unfractured. At EL. 78.6 ft, becomes moderately soft to moderately hard. At EL. 145.6 ft, becomes weak, moderately soft, very slightly fractured. At EL. 144.1 ft, observed joint, dipping 40°, slightly rough, 1 mm, no filling. At EL. 78.0 ft, becomes soft, shear, slickensided. SILTSTONE (MUDSTONE), laminated, olive gray, slightly weathered, moderately hard to hard, moderately fractured, shear (clay), dipping 15 to 60°, smooth, 0-1 mm. At EL. 143.6 ft, becomes moderately soft to moderately hard. At EL. 138.6 ft, becomes soft. SANDSTONE, massive, dark greenish gray, slightly weathered, soft to moderately At EL. 135.6 ft, becomes moderately hard. At EL. 134.2 ft, observed joint, dipping 40°, slightly rough, very thin, no filling. At EL. 133.6 ft, becomes soft. soft, unfractured. At EL. 73.6 ft, becomes unfractured. Unit is medium strong. At EL. 71.1 ft, becomes soft. At EL. 128.6 ft, becomes soft to moderately soft. At Fl. 69.8 ft. becomes hard. SILTSTONE (MUDSTONE), laminated, olive gray, slightly weathered, hard, intensely fractured, shear (clay), dipping 45°, smooth, 1 mm. At EL. 68.6 ft, becomes hard to very hard, moderately fractured, bedding plane separation, dipping 45°, smooth, very thin, no filling.

**ENGINEERING SERVICES GEOTECHNICAL SERVICES** DIVISION OF ENGINEERING SERVICES SR-710 TUNNEL TECHNICAL STUDY STATE OF N/A STRUCTURE DESIGN UNCTIONAL SUPERVISOR FIELD INVESTIGATION BY: K. BARKER/ M. SALISBURY DRAWN BY: EDFLYNE MIGUEL CALIFORNIA POST MILES LOG OF TEST BORING SHEET 3 OF 3 NAME: SHIVA KARIMI DATE: 2/12/09 - 2/27/09 DEPARTMENT OF TRANSPORTATION **DESIGN BRANCH** CHECKED BY: JOSEPH PRATT ORIGINAL SCALE IN INCHES DISREGARD PRINTS BEARING EARLIER REVISION DATES OGS CIVIL LOG OF TEST BORINGS SHEET EA 07-187900

FILE => \$REQUEST

KRISTOPHER P.

No. CEG 2383

Exp. 8/31/2011

CERTIFIED ENGINEERING GEOLOGIST

130

120

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**PROFILE** 

NGS Benchmark used. Pt MF36F: N1847799.06, E6512460.75, Elevation 407.29' MF 36F: A 3" Brass disk stamped "Metropolitan Water District of Southern California MF 36F 1989", set in the top of the bridge curb at the northwest corner of the Hellman Avenue overcrossing of the Long Beach Freeway (I-710), 68' west of the center of the south bound lanes of the freeway, 25' north of the centerline of Hellman Avenue, 13' east of the west end of the bridge, 1'north of the north curb face. NGS PID EW9453.

Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. Vertical control based on NAVD-88. P+ 0153 N1845410.50, E6509860.21, Pt 0617 N1858044.3, E6491094.23.

Units are in U.S. survey feet.

NOTES:

- 1) This LOTB sheet was prepared in accordance with the Caltrans Soil and Rock Logging, Classification and Presentation Manual (June 2007) except as noted in Appendix A.1 of the Final Geotechnical Summary Report SR-710 Tunnel Technical Study Los Angeles County, California dated (April, 2010).
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- 4) To convert from California ring sampler blow count to equivalent standard penetration test sampler blow count, for granular and cohesive soil, multiply
- 5) All bedding and other structural angles are measured from horizontal.

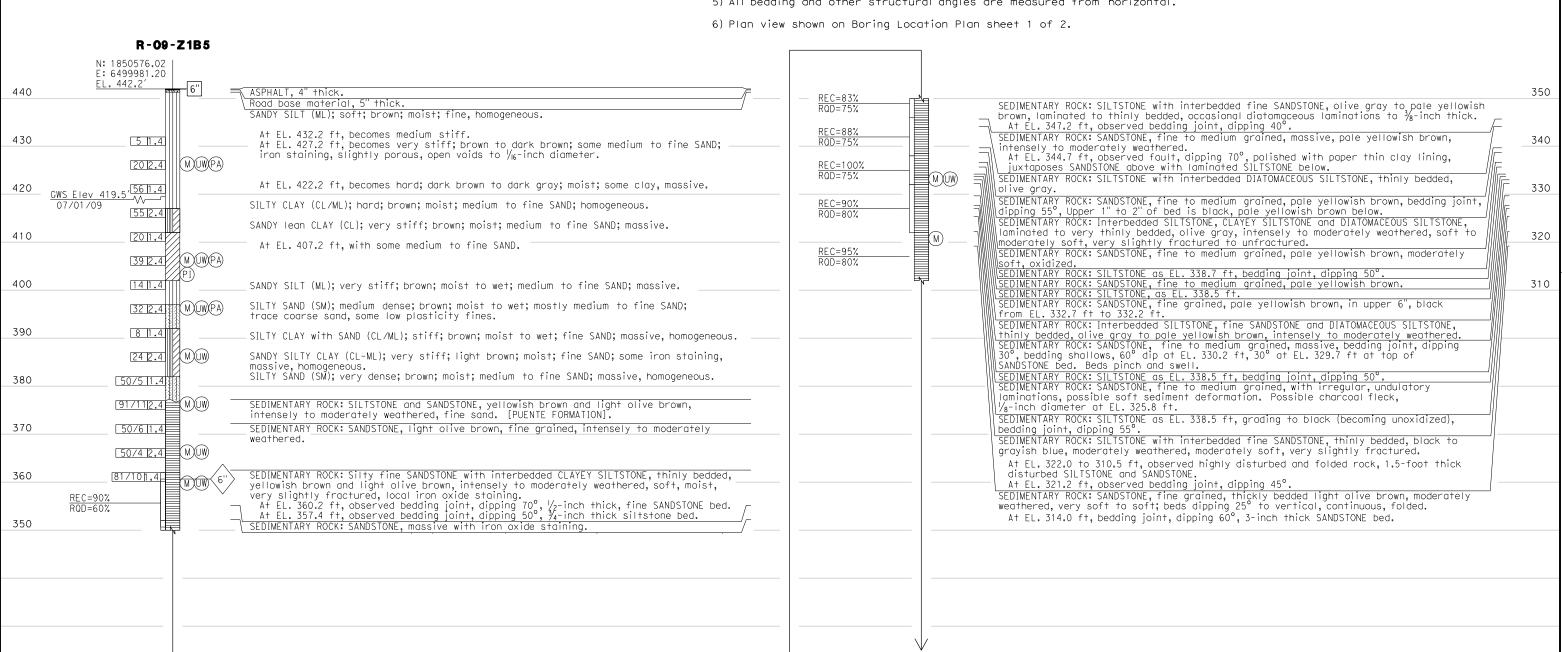
See Sheet 2

FOR CONTINUATION

FILE => \$REQUEST

IST COUNTY 710 ΙΑ 3/18/10 GEOTECHNICAL ENGINEER DATE MAHESWARAN RAVEENDRA No. GE 2743 Exp. 06-30-11 PLANS APPROVAL DATE The State of California or its officers or agents & OTECHNICAL shall not be responsible for the accuracy or completeness of electronic copies of this plan sheet. CH2M HILL 6 HUTTON CENTRE DRIVE, SUITE 700 SANTA ANA, CA 92707

**PROFILE** 



PREPARED FOR THE DRAWN BY: SR-710 TUNNEL TECHNICAL STUDY DEREK HIGA D. JANKI Y EDELYNE MIGUEL STATE OF N/A MAHESWARAN RAVEENDRA DESIGN OVERSIGHT ENGINEER FIELD INVESTIGATION BY CALIFORNIA PROJECT ENGINEER POST MILES DATE: 2/16/09 - 2/24/09 DAN JANKLY LOG OF TEST BORING SHEET 1 OF 4 DEPARTMENT OF TRANSPORTATION SIGN OFF DATE ORIGINAL SCALE IN INCHES DISREGARD PRINTS BEARING EARLIER REVISION DATES OGS CIVIL LOG OF TEST BORINGS SHEET EA 07-187900

DEREK HIGA
DESIGN OVERSIGHT ENGINEER

OGS CIVIL LOG OF TEST BORINGS SHEET

SIGN OFF DATE

NGS Benchmark used. Pt MF36F: N1847799.06, E6512460.75, Elevation 407.29' MF 36F: A 3" Brass disk stamped "Metropolitan Water District of Southern California MF 36F 1989", set in the top of the bridge curb at the northwest corner of the Hellman Avenue overcrossing of the Long Beach Freeway (I-710). 68' west of the center of the south bound lanes of the freeway, 25' north of the centerline of Hellman Avenue, 13' east of the west end of the bridge, 1' north of the north curb face. NGS PID EW9453.

DRAWN BY:

CHECKED BY:

EDELYNE MIGUEL

DAN JANKLY

Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. Vertical control based on NAVD-88. P+ 0153 N1845410.50, E6509860.21, P+ 0617 N1858044.3, E6491094.23.

Units are in U.S. survey feet.

NOTES:

- 1) This LOTB sheet was prepared in accordance with the Caltrans Soil and Rock Logging, Classification and Presentation Manual (June 2007) except as noted in Appendix A.1 of the Final Geotechnical Summary Report SR-710 Tunnel Technical Study Los Angeles County, California dated (April, 2010).
- 2) California ring, standard penetration test, and HQ&PQ core samplers were used to obtain soil and rock samples.
- 3) An automatic trip hammer system consisting of a hammer weight of 140 lbs falling a distance of 30" was used to advance the drive samplers.
- 4) To convert from California ring sampler blow count to equivalent standard penetration test sampler blow count, for granular and cohesive soil, multiply
- 5) All bedding and other structural angles are measured from horizontal.

COUNTY LΑ 710 3/18/10 GEOTECHNICAL ENGINEER MAHESWARAN RAVEENDRA No. GE 2743 Exp. 06-30-11 PLANS APPROVAL DATE S GOTECHNICAL shall not be responsible for the accuracy or completeness of electronic copies of this plan sheet. CH2M HILL 6 HUTTON CENTRE DRIVE, SUITE 700 SANTA ANA, CA 92707

R-09	-Z1B5 ∧	6) Pidn Vie	ew shown on Boring	y Locarion Fian	SHEET 1 01 2.	
See Sheet 1 FOR CONTINUATION					At EL.238.2 ft, observed moderately soft to hard, 6-inch thick section of silt fine	
			REC=88% RQD=88%		SANDSTONE.	2
REC=95% RQD=95%		At EL. 310.7 ft, bedding joint, dipping 50°.  At EL. 308.2 ft, observed bedding joint, dipping 50°, olive brown above, black below.  At EL. 302.2 ft, becomes fine to medium grained, some soft sediment deformation.			At EL. 236.9 ft, observed bedding joint, dipping 30°, very faint, cemented SANDSTONE lamination.	
		SEDIMENTARY ROCK: Interhodded SILISTONE and fine SANDSTONE yeary thinly hadded dark	REC=100% RQD=80%	<b>  ■</b>	At EL. 236.2 ft, becomes moderately soft. At EL. 235.2 ft, becomes soft, massive SANDSTONE.	
DE0.400W	MUW_	greenish gray to dark bluish gray, fresh, moderately soft to moderately hard, ´	- 100-00%		At El. 233.9 ft, observed fault, dipping 30°, truncates V <sub>B</sub> -inch thick SILTSTONE	
REC=100% RQD=94%		greenish gray to dark bluish gray, fresh, moderately soft to moderately hard, slightly fractured, sandstone beds pinch and swell. At EL. 299.2 ft, observed bedding joint, dipping 60°. At EL. 297.5 ft, observed 6-inch thick SILTSTONE bed, intensely fractured. At EL. 293.6 ft, observed bedding joint, dipping 55°, faint black lamination.	REC=100%		At EL. 233.9 ft, observed fault, dipping 30°, truncates V <sub>8</sub> -inch thick SILTSTONE lamination, fault has V <sub>8</sub> -inch thick brown, slightly clayey gouge, unknown offset.  At EL. 233.2 ft, observed fault, dipping 50°, V <sub>8</sub> -inch thick, brown, slightly clayey gouge unknown offset	
REC=100% RQD=100%		AT EL. 293.2 TT, ODSERVED SEDIMENTARY RUCK; INTERDEDDED SILISTONE DND TINE SANDSTONE,	REC=100% RQD=100%		gouge, unknown offset. At EL. 231.2 ft, becomes moderately soft, bedding joint, dipping 40°, faint SANDSTONE lamination.	
RQD=100% REC=100% RQD=100%		At EL. 292.2 ft, observed bedding joint, dipping 45°. At EL. 290.2 ft, observed 6-inch thick SANDSTONE bed, depositionally irregular contact at base.	REC=94% RQD=75%		At EL. 230.2 ft, becomes moderately soft to moderately hard, massive SANDSTONE, medium strong.  At EL. 229.7 ft, becomes soft to moderately soft, joint, dipping 60°, tight, planar,	
REC=100% RQD=100%		At EL. 288.9 ft, observed SANDSTONE bed, fine grained, with numerous siltstone rip-up	REC=100%		no lining.  SEDIMENTARY ROCK: SANDY SILTSTONE, laminated, bedding joint, dipping 45°.	
RQD=100%		At EL. 286.2 ft, observed bedding joint, dipping 60°, Decreasing SILTSTONE. At EL. 283.7 ft, observed bedding joint, dipping 50°. SEDIMENTARY ROCK: SANDSTONE with occasional SILTSTONE beds and rip-up clasts, sand is	RQD=88% RF C=100%	EMSD	At EL. 219.2 ft, observed 4-inch thick interbedded SILTSTONE and fine SANDSTONE,	
REC=100% ROD=89%	MUWUU	SEDIMENTARY ROCK: SANDSTONE WITH OCCUSIONAL SILISIONE BOOS and rip-up clasts, sand is fine to medium grained.  At EL. 281.2 ft, observed bedding joint, dipping 45°, SILTSTONE with interbedded	RQD=63% REC=100%		At EL. 216.2 ft, observed fault, dipping 70°, tight, no lining, silty fine SANDSTONE above and fine to medium SANDSTONE below.	
1140-05%		SANDSTONE. At EL. 279.9 ft. becomes SANDSTONE with some SILTSTONE rip-up clasts at top.	RQD=0%		SEDIMENTARY ROCK: Interbedded SILTSTONE and fine sandy SILTSTONE, thinly to moderately bedded, moderately soft to moderately hard, local beds with soft sediment deformation, pinch, swell and folded.	
REC=70% RQD=60%		SEDIMENTARY ROCK: Interbedded SILTSTONE and fine SANDSTONE, as EL. 293.0 ft.  At EL. 274.6 ft, observed 0.4′ thick cemented fine SANDSTONE bed, moderately hard, bedding dipping 45° at base.	REC=96% RQD=96%		At ÉL. 211.2 ft, observed to be very weak, bedding joint dipping 55°. SEDIMENTARY ROCK: SILTY SANDSTONE, bedding joint, dipping 55°.	
DE 0-00°		At EL. 270.8 ft, observed Silty fine to medium grained SANDSTONE with siltstone rip-up clasts. Unit is very weak.	REC=100% RQD=89%		SEDIMENTARY ROCK: SILTSTONE with interbedded fine SANDSTONE. At EL. 209.7 ft, observed bedding joint, dipping 60°. At EL. 209.2 ft, observed shear, dipping 30°, slightly polished, below is roughly 1.5	
REC=86% RQD=71%		At EL. 269.2 ft, observed shear, vertical juxtaposes SANDSTONE and SILTSTONE. Material highly disturbed as EL. 268.5 ft.	RQD=89%		<ul> <li>thick silty fine sandstone bed with soft sediment deformation (tightly folded siltstone — beds, rip-ups).</li> </ul>	
REC=89% RQD=67%		SEDIMENTARY ROCK: SANDSTONE, thickly bedded to very thickly bedded, dark greenish gray, fresh, soft to moderately soft, slightly fractured, very moist, bedding joint, dipping 50°, on black SANDSTONE laminations.			At EL. 207.2 ft, observed fault, dipping 70°, 1-inch to 2-inch thick gouge zone of intensely fractured SILTSTONE and soft SANDSTONE, material highly disturbed adjacent	
		At EL. 265.2 ft, becomes Silty fine SANDSTONE, occasional cross beds.  At EL. 262.2 ft, observed bedding joint, dipping 35°, SANDSTONE bed, 5-inches thick, very hard, fine to medium grained.	_		to fault. — At EL. 202.2 ft, observed bedding, dipping 50°, locally slightly brittle, intensley fractured.	
		very hard, fine to medium grained. At EL. 258.7 ft, observed SANDSTONE bed, 5-inches thick, very hard, fine to medium grained, joint in center, tight, rough, dipping 60-70°.  At EL. 255.2 ft, becomes fine to medium grained.			At EL. 200.7 ft, observed bedding joint, dipping 60°, material highly broken, fragments are moderately hard.	
	_	At EL. 255.2 ft, becomes fine to medium grained.  SEDIMENTARY ROCK: SILTSTONE, laminated, dippina 40°, Bed offset (reverse)2,5-inches by	_		SEDIMENTARY ROCK: Interbedded SILTSTONE and fine SANDSTONE, laminated to thinly bedded.  At EL. 195.7 ft, observed bedding joint, dipping 50°.  At EL. 194.7 ft, observed joint, dipping 55°, conjugate to bedding.	
		SEDIMENTARY ROCK: SILTSTONE, laminated, dipping 40°. Bed offset (reverse)2.5-inches by tight fault with fine sand lining. Fault dipping 60°.  At EL. 252.2 ft, observed Fault, undulatory, roughly 30° dip, tight, polished, striated			SEDIMENTARY ROCK: Silty fine SANDSTONE, beds highly folded above contact, soft sediment deformation.	
	_	along strike, unknown offset.  SEDIMENTARY ROCK: SANDSTONE, thickly bedded to very thickly bedded, dark greenish gray, fresh, soft to moderately soft, slightly fractured, very moist, bedding joint, dipping	_		At EL.192.2 ft, observed fault, dipping 70°, very tight, striated perpendicular to dip,	
		50 to 30°, cross bedded.  At EL. 248.2 ft, becomes Silty fine SANDSTONE.  At EL. 247.7 ft, observed bedding joint, dipping 45°, locally fractured, joints dipping			At EL.188.3 ft, observed bedding joint, dipping 55°, on 4" thick SILTSTONE bed. At EL.187.4 ft, observed 1.5′ thick laminated SILTSTONE bed. At EL.184.2 ft, observed shear, dipping 40 to 0°, undulatory, ½-inch thick, soft clay	
		70°, 30°, and 20°.	_		— lining. —	_
		At EL. 246.2 ft, becomes very thickly bedded to massive, bedding joint, dipping 40°, on $\frac{1}{8}$ -inch thick SILTSTONE bed. At EL. 245.7 ft, observed bedding joint, dipping 20°, tight, faint, no lining.			SEDIMENTARY ROCK: SILTSTONE, laminated, bedding joint, dipping 50°, on contact to SILTSTONE.  At EL. 181.2 ft, observed bedding joint, dipping 60°, Some interbedded fine SANDSTONE.	
	_	At EL. 242.2 ft, becomes very thickly bedded, locally laminated, soft.  At EL. 241.2 ft, observed bedding joint, dipping 45°, on /g-inch thick SILTSTONE lens.	_			
				W See Sheet 3		
			F	OR CONTINUATION	PRO Vert:	

PREPARED FOR THE STATE OF

CALIFORNIA

DEPARTMENT OF TRANSPORTATION

D. JANKLY

FIELD INVESTIGATION BY

DATE: 2/16/09 - 2/24/09

ORIGINAL SCALE IN INCHES FOR REDUCED PLANS

CU EA 07-187900

MAHESWARAN RAVEENDRA PROJECT ENGINEER

N/A

POST MILES

DISREGARD PRINTS BEARING EARLIER REVISION DATES \_

SR-710 TUNNEL TECHNICAL STUDY LOG OF TEST BORING SHEET 2 OF 4

DEREK HIGA

SIGN OFF DATE

DESIGN OVERSIGHT ENGINEER

OGS CIVIL LOG OF TEST BORINGS SHEET

NGS Benchmark used. Pt MF36F: N1847799.06, E6512460.75, Elevation 407.29' MF 36F: A 3" Brass disk stamped "Metropolitan Water District of Southern California MF 36F 1989", set in the top of the bridge curb at the northwest corner of the Hellman Avenue overcrossing of the Long Beach Freeway (I-710). 68' west of the center of the south bound lanes of the freeway, 25' north of the centerline of Hellman Avenue, 13' east of the west end of the bridge, 1' north of the north curb face. NGS PID EW9453.

DRAWN BY:

EDELYNE MIGUEL

DAN JANKLY

D. JANKLY

FIELD INVESTIGATION BY

DATE: 2/16/09 - 2/24/09

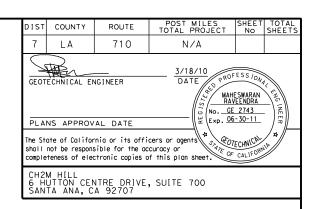
ORIGINAL SCALE IN INCHES

Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. Vertical control based on NAVD-88. Pt 0153 N1845410.50, E6509860.21, Pt 0617 N1858044.3, E6491094.23.

Units are in U.S. survey feet.

#### NOTES:

- This LOTB sheet was prepared in accordance with the Caltrans Soil and Rock Logging, Classification and Presentation Manual (June 2007) except as noted in Appendix A.1 of the Final Geotechnical Summary Report SR-710 Tunnel Technical Study Los Angeles County, California dated (April, 2010).
- 2) California ring, standard penetration test, and HQ&PQ core samplers were used to obtain soil and rock samples.
- 3) An automatic trip hammer system consisting of a hammer weight of 140 lbs falling a distance of 30" was used to advance the drive samplers.
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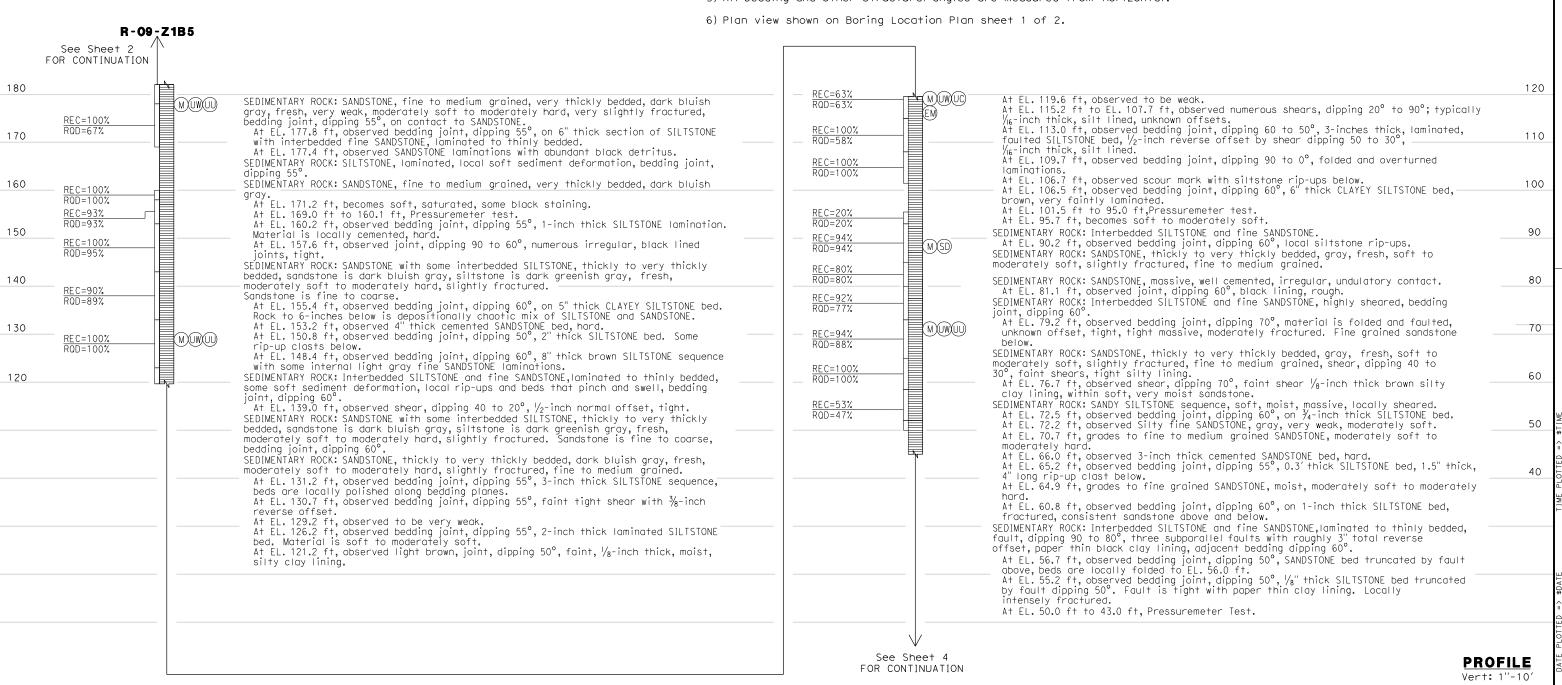
SR-710 TUNNEL TECHNICAL STUDY

LOG OF TEST BORING SHEET 3 OF 4

N/A

POST MILES

DISREGARD PRINTS BEARING EARLIER REVISION DATES -



PREPARED FOR THE

STATE OF

CALIFORNIA

DEPARTMENT OF TRANSPORTATION

MAHESWARAN RAVEENDRA PROJECT ENGINEER

EA 07-187900

#### **BENCHMARK:** LΑ 710 NGS Benchmark used. Pt MF36F: N1847799.06, E6512460.75, Elevation 407.29' NOTES: 3/18/10 1) This LOTB sheet was prepared in accordance with the Caltrans Soil and MF 36F: A 3" Brass disk stamped "Metropolitan Water District of Southern California GEOTECHNICAL ENGINEER DATE MF 36F 1989", set in the top of the bridge curb at the northwest corner of the Hellman Rock Logging, Classification and Presentation Manual (June 2007) Avenue overcrossing of the Long Beach Freeway (I-710). 68' west of the center of the south bound lanes of the freeway, 25' north of the centerline of Hellman Avenue, 13' except as noted in Appendix A.1 of the Final Geotechnical Summary Report MAHESWARAN RAVEENDRA SR-710 Tunnel Technical Study Los Angeles County, California No. GE 2743 east of the west end of the bridge, 1'north of the north curb face. NGS PID EW9453. dated (April, 2010). Exp. 06-30-11 PLANS APPROVAL DATE 2) California ring, standard penetration test, and HO&PO core samplers The State of California or its officers or agents Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. & OTECHNICAL shall not be responsible for the accuracy or Vertical control based on NAVD-88. were used to obtain soil and rock samples. completeness of electronic copies of this plan sheet. P+ 0153 N1845410.50, E6509860.21, P+ 0617 N1858044.3, E6491094.23. 3) An automatic trip hammer system consisting of a hammer weight of 140 lbs CH2M HILL 6 HUTTON CENTRE DRIVE, SUITE 700 SANTA ANA, CA 92707 falling a distance of 30" was used to advance the drive samplers. Units are in U.S. survey feet. 4) To convert from California ring sampler blow count to equivalent standard penetration test sampler blow count, for granular and cohesive soil, multiply 5) All bedding and other structural angles are measured from horizontal. 6) Plan view shown on Boring Location Plan sheet 1 of 2. R-09-Z1B5 See Sheet 3 FOR CONTINUATION SEDIMENTARY ROCK: SANDSTONE, thickly to very thickly bedded, gray, fresh, soft to moderately soft, slightly fractured, fine to coarse grained, shear, dipping 60 to 55°, tight, light gray silty lining. 40 40 At EL. 42.2 ft, observed bedding joint, dipping 65 to 55°, on dark gray SANDSTONE At EL. 40.3 ft to EL. 23.0 ft, observed numerous shears, dipping 20° to 70°; typically tight, with $\frac{1}{16}$ -inch to $\frac{1}{4}$ -inch thick light gray silty lining. ft, becomes moderately soft to moderately hard. 30 30 At EL. 35.2 ft, becomes moderately hard. At EL. 34.2 ft, becomes weak, moderately hard. At EL. 34.0 ft, observed bedding joint, dipping 55°, 3" thick sequence of interbedded SILTSTONE and SANDSTONE, faulted, intensely fractured. At EL. 28.2 ft, observed numerous SILTSTONE fragments, juxtaposed to massive SANDSTONE. At EL. 26.9 ft, observed SANDSTONE, dark bluish gray, fine grained, moist. 20 20 REC=94% RQD=80% SEDIMENTARY ROCK: SANDSTONE with some interbedded SILTSTONE, massive to thickly bedded. dark bluish gray, fresh, moderately soft, moist to saturated. Sand is fine to medium grained. 10 At EL. 12.2 ft, observed fresh, moderately soft. At EL. 10.2 ft, observed fresh, moderately soft. At EL. 10.2 ft, observed bedding joint, dipping 65°, on 2-inch thick SILTSTONE bed, SANDSTONE to 1.2′ below is soft, tightly folded, soft sediment deformation. At EL. 6.7 ft, observed shear, dipping 20°, faint, tight gray silty lining. 10 **■**PA) 0 0 REC=97% RQD=90% At EL. -1.6 ft, observed bedding joint, dipping 65°, Siltstone rip-up clasts to 1-inch thick, some fine SANDSTONE laminations below are depositionally folded. thick, some fine SANDSTONE laminations below are depositionally folded. SEDIMENTARY ROCK: SANDSTONE, massive, dark bluish gray, fresh, moderately soft, moist to saturated. Sand is fine to medium grained. At EL. -4.3 ft, observed bedding joint, dipping 70°, 2-inch thick SILTSTONE bed. At EL. -5.8 ft, grades to SILTY fine SANDSTONE, as above. At EL. -8.6 ft, observed bedding joint, dipping 50°, ½-inch to 1-inch thick SILTSTONE bed, offset ¾-inch by two shears which are very faint, no lining, dipping 40 and 50°. At El. -13.0, PTS: Coarse-grained granitic arkosic SANDSTONE with crystal and rock fragments up to 3mm in size, in a predominantly calcite matrix (50%). At EL. -12.8 ft, observed 7" thick SANDSTONE bed, moderately hard. At EL. -13.5 ft, observed bedding joint, dipping 60°, on silty bed at base of cemented SANDSTONE above. -10 -10 REC=100% RQD=100% REC=100% RQD=100% -20 -20 SANDSTONE abové. -30 -30 At EL. -14.4 ft to EL. -23.8 ft, observed numerous shears, dipping 15° to 55°, typically with 1/6" to 1/8" thick silty lining. At EL. -17.8 ft, observed possible bedding plane (dipping 60°) on contact from fine to medium grained SANDSTONE above to fine SANDSTONE below, faint. At EL. -40.8 ft to EL. -58.4, observed numerous shears, dipping 70 to 10°, typically faint, with up to 1/4" thick clay lining. REC=100% RQD=90% M)(UW)(PA) -40 -40 At EL. -42.3 ft, observed possible bedding (dipping 65°) on black sand bed which pinches out. At EL. -44.3 ft, Unit grades to Silty fine SANDSTONE, gray, massive, soft, moist. At EL. -46.3 ft, observed fault, dipping 55°, clay lined to 1/16" thick, 1" reverse offset of 1" thick SILTSTONE bed dipping 50°. -50 -50 At EL. -48.8 ft, observed bedding joint, dipping 55°, on 3/4-inch thick laminated SILTSTONE and fine SANDSTONE bed. At EL. -50.8 ft, becomes very soft. At EL. -54.8 ft, observed 3.5" thick SANDSTONE bed, hard. Below is SANDSTONE, greenish -60 -60 gray to gray, very soft to soft, fine to medium grained, moist to very moist. 02-24-09 Terminated at EL. -59.8' ER; = 70% **PROFILE** PREPARED FOR THE DEREK HIGA DRAWN BY: D. JANKIY SR-710 TUNNEL TECHNICAL STUDY EDELYNE MIGUEL STATE OF N/A MAHESWARAN RAVEENDRA DESIGN OVERSIGHT ENGINEER FIELD INVESTIGATION BY CALIFORNIA PROJECT ENGINEER POST MILES CHECKED BY DATE: 2/16/09 - 2/24/09 LOG OF TEST BORING SHEET 4 OF 4 DAN JANKLY DEPARTMENT OF TRANSPORTATION N/A SIGN OFF DATE ORIGINAL SCALE IN INCHES FOR REDUCED PLANS DISREGARD PRINTS BEARING EARLIER REVISION DATES OGS CIVIL LOG OF TEST BORINGS SHEET

EA 07-187900

FILE => \$REQUEST

COUNTY

4 | 4

IST

DESIGN OVERSIGHT ENGINEER

OGS CIVIL LOG OF TEST BORINGS SHEET

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NGS Benchmark used. Pt MF36F: N1847799.06, E6512460.75, Elevation 407.29' MF 36F: A 3" Brass disk stamped "Metropolitan Water District of Southern California MF 36F 1989", set in the top of the bridge curb at the northwest corner of the Hellman Avenue overcrossing of the Long Beach Freeway (I-710). 68' west of the center of the south bound lanes of the freeway, 25' north of the centerline of Hellman Avenue, 13' east of the west end of the bridge, 1'north of the north curb face. NGS PID EW9453.

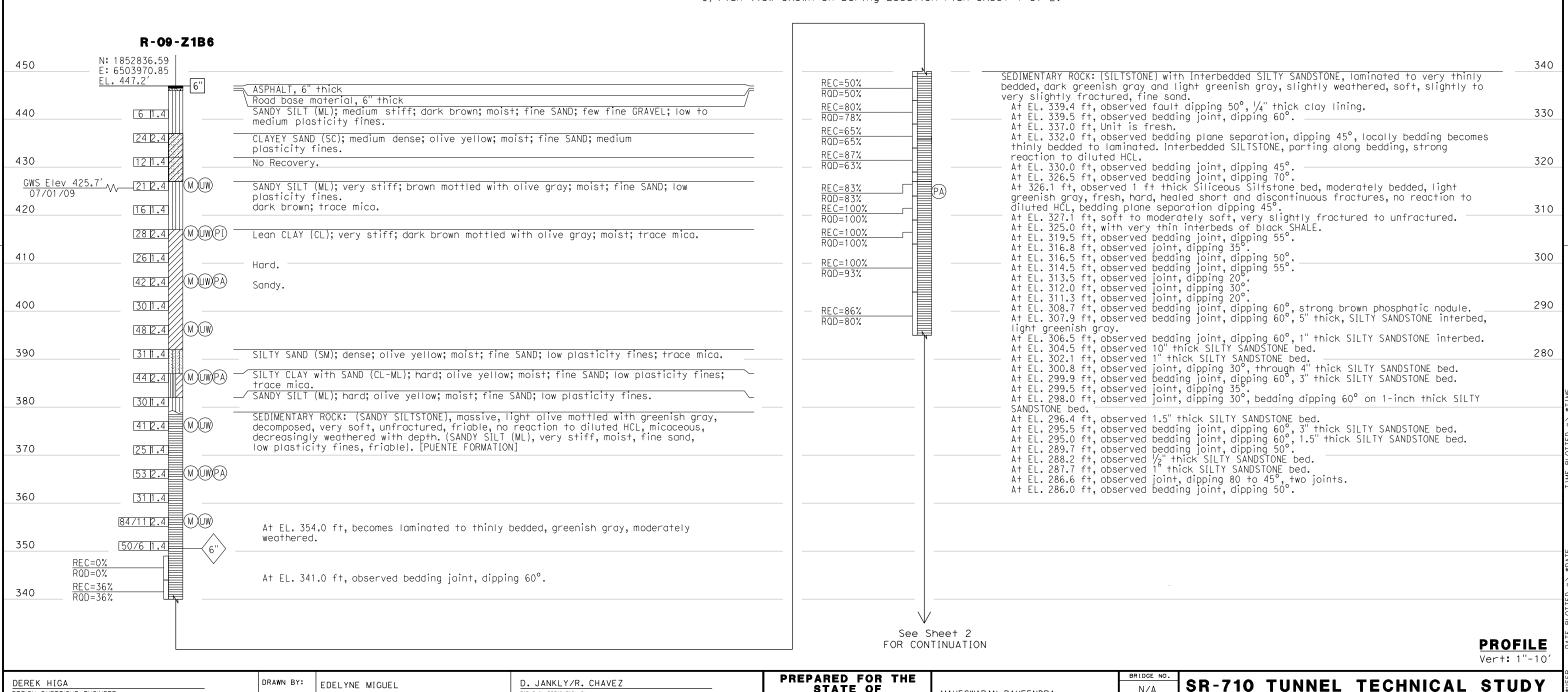
Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. Vertical control based on NAVD-88. P+ 0153 N1845410.50, E6509860.21, Pt 0617 N1858044.3, E6491094.23.

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COUNTY LΑ 710 3/18/10 GEOTECHNICAL ENGINEER DATE MAHESWARAN RAVEENDRA No. GE 2743 Exp. 06-30-11 PLANS APPROVAL DATE The State of California or its officers or agents hall not be responsible for the accuracy or completeness of electronic copies of this plan sheet. CH2M HILL 6 HUTTON CENTRE DRIVE, SUITE 700 SANTA ANA, CA 92707



STATE OF

CALIFORNIA

DEPARTMENT OF TRANSPORTATION

FIELD INVESTIGATION BY

DATE: 1/30/09 - 2/10/09

ORIGINAL SCALE IN INCHES FOR REDUCED PLANS

DAN JANKLY

MAHESWARAN RAVEENDRA PROJECT ENGINEER

EA 07-187900 FILE => \$REQUEST N/A

POST MILES

DISREGARD PRINTS BEARING EARLIER REVISION DATES

LOG OF TEST BORING SHEET 1 OF 3

DESIGN OVERSIGHT ENGINEER

OGS CIVIL LOG OF TEST BORINGS SHEET

SIGN OFF DATE

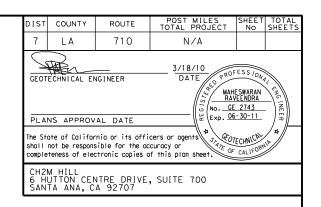
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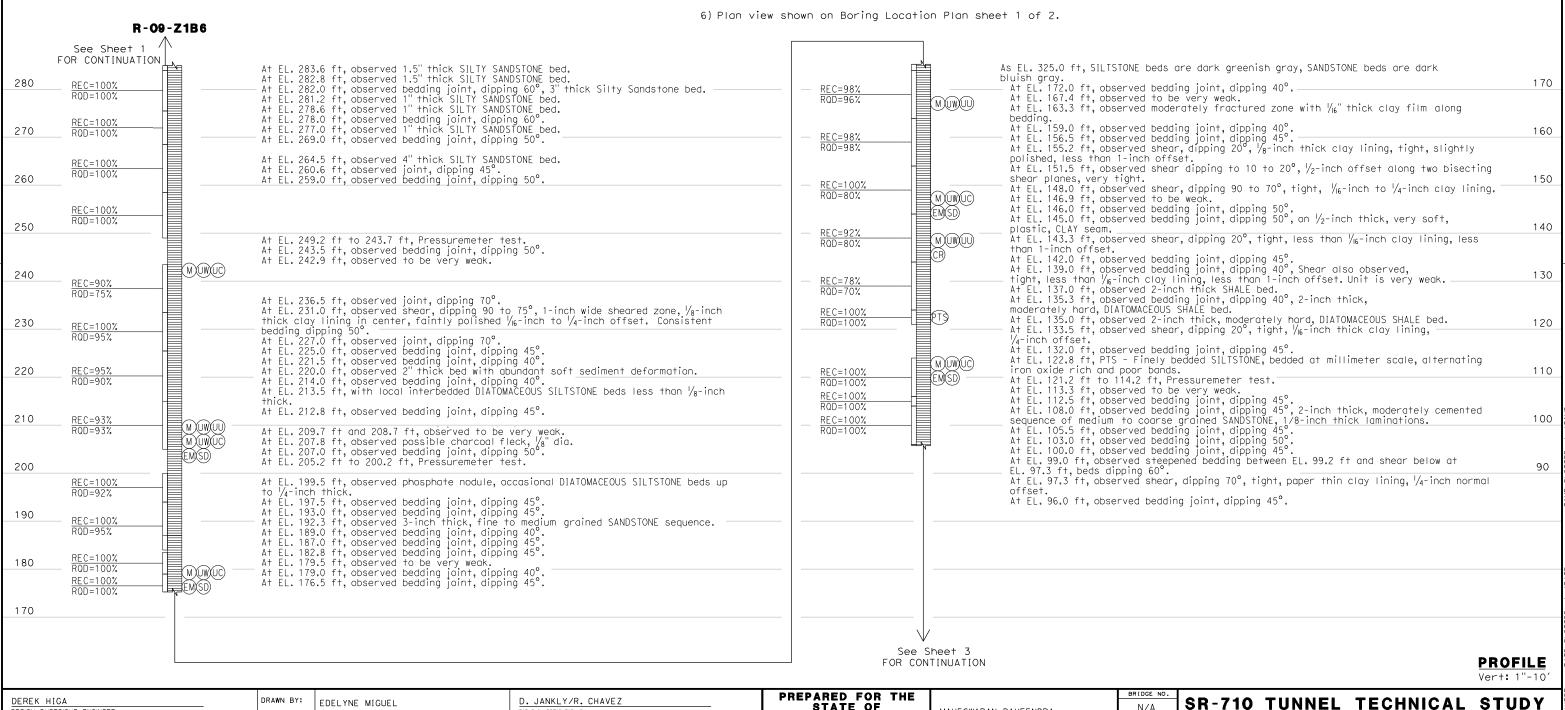
Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. Vertical control based on NAVD-88. P+ 0153 N1845410.50, E6509860.21, P+ 0617 N1858044.3, E6491094.23.

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STATE OF

CALIFORNIA

DEPARTMENT OF TRANSPORTATION

FIELD INVESTIGATION BY:

DATE: 1/30/09 - 2/10/09

ORIGINAL SCALE IN INCHES

DAN JANKLY

MAHESWARAN RAVEENDRA PROJECT ENGINEER

EA 07-187900 FILE => \$REQUEST N/A

POST MILES

N/A

DISREGARD PRINTS BEARING EARLIER REVISION DATES

LOG OF TEST BORING SHEET 2 OF 3

#### NGS Benchmark used. Pt MF36F: N1847799.06, E6512460.75, Elevation 407.29' NOTES: 3/18/10 1) This LOTB sheet was prepared in accordance with the Caltrans Soil and MF 36F: A 3" Brass disk stamped "Metropolitan Water District of Southern California GEOTECHNICAL ENGINEER DATE MF 36F 1989", set in the top of the bridge curb at the northwest corner of the Hellman Rock Logging, Classification and Presentation Manual (June 2007) Avenue overcrossing of the Long Beach Freeway (I-710). 68' west of the center of the south bound lanes of the freeway, 25' north of the centerline of Hellman Avenue, 13' east of the west end of the bridge, 1' north of the north curb face. NGS PID EW9453. except as noted in Appendix A.1 of the Final Geotechnical Summary Report MAHESWARAN RAVEENDRA SR-710 Tunnel Technical Study Los Angeles County, California No. GE 2743 dated (April. 2010). Exp. 06-30-11 PLANS APPROVAL DATE 2) California ring, standard penetration test, and HQ&PO core samplers The State of California or its officers or agents Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. QOTECHNICAL OTECHNICAL shall not be responsible for the accuracy or Vertical control based on NAVD-88. were used to obtain soil and rock samples. completeness of electronic copies of this plan sheet. Pt 0153 N1845410.50, E6509860.21, P+ 0617 N1858044.3, E6491094.23. 3) An automatic trip hammer system consisting of a hammer weight of 140 lbs CH2M HILL 6 HUTTON CENTRE DRIVE, SUITE 700 SANTA ANA, CA 92707 falling a distance of 30" was used to advance the drive samplers. Units are in U.S. survey feet. 4) To convert from California ring sampler blow count to equivalent standard penetration test sampler blow count, for granular and cohesive soil, multiply 5) All bedding and other structural angles are measured from horizontal. 6) Plan view shown on Boring Location Plan sheet 1 of 2. R-09-Z1B6 See Sheet 2 FOR CONTINUATION At EL. 93.7 ft, observed shear/fault zone, dipping 90 to 70°, ¼-inch to ½-inch thick clay lining, irregular, undulatory shear, bedrock within shear zone is moderately contorted, beds are folded and irregular, beds dip up to 60°. At EL. 91.0 ft, observed bedding joint, dipping 45°. At EL. 89.0 ft, observed shear, dipping 65°, tight, paper-thin clay lining. At EL. 88.0 ft, observed bedding joint, dipping 50°, interbedded fine SANDSTONE and SHALE, gradually increasing cemented, moderately hard to EL. 81.7 ft. At EL. 86.0 ft, observed bedding joint, dipping 55°, well cemented, moderately fractured. At EL. 85.0 ft, observed local bedding planes with chrome colored minerals. At EL. 83.0 ft, observed roughly 1.5′ thick, moderately hard, laminated, SILTSTONE bed, moderately fractured. 90 RQD=81% REC=100% RQD=63% 80 REC=92% RQD=85% moderately fractured. At EL. 81.5 ft, observed bedding joint, dipping 45°, soft to moderately soft. 70 MUWUC At EL. 70.5 ft, Unit is very weak. At EL. 68.5 ft, observed 3-inch thick moderately cemented zone, moderately hard. At EL. 63.0 ft, observed bedding joint, dipping 45°, occasional SHALE beds up to EMSD $\frac{1}{4}$ -inch thick. At EL. 61.5 ft, observed $\frac{1}{4}$ -inch thick, cross-bedded, fine SANDSTONE bed, pinches and swells. Increasing SANDSTONE beds. At EL. 61.0 ft, observed $\frac{1}{4}$ -inch thick, cross-bedded, fine SANDSTONE bed, pinches and 60 swells. At EL. 60.5 ft, observed 2" thick SANDSTONE bed, laminated. 50 At EL. 59.5 ft, observed bedding joint, dipping 50°. At EL. 57.0 ft, observed 1.5" diameter granitic clast, subrounded. At EL. 56.6 ft, becomes moderately soft to moderately hard, SANDSTONE is light At EL. 56.6 ft, becomes moderately soft to moderately hard, SANDSIONE is light greenish gray, SILTSTONE is black. At EL. 54.5 ft, observed bedding joint, dipping 45°, 2" thick SANDSTONE bed, well cemented, 1/4-inch thick SILTSTONE lamination in center. At EL. 53.0 ft, observed 1-inch thick SANDSTONE bed, pinches out at center of core. At EL. 52.5 ft, observed bedding joint, dipping 50°, SANDSTONE beds exhibit soft sediment deformation: cross bedding, pinch and swell. At EL. 49.5 ft, observed bedding joint, dipping 45°. At EL. 47.0 ft, observed bedding joint, dipping 50°. 02-10-09 Terminated at 40 30 20 10 0 -10 -20 **PROFILE** Vert: 1"-10 PREPARED FOR THE D. JANKLY/R. CHAVEZ DEREK HIGA DRAWN BY: SR-710 TUNNEL TECHNICAL STUDY EDELYNE MIGUEL STATE OF N/A DESIGN OVERSIGHT ENGINEER MAHESWARAN RAVEENDRA PROJECT ENGINEER FIELD INVESTIGATION BY **CALIFORNIA** POST MILES CHECKED BY: LOG OF TEST BORING SHEET 3 OF 3 DAN JANKLY DATE: 1/30/09 - 2/10/09 DEPARTMENT OF TRANSPORTATION N/A SIGN OFF DATE ORIGINAL SCALE IN INCHES FOR REDUCED PLANS DISREGARD PRINTS BEARING EARLIER REVISION DATES OGS CIVIL LOG OF TEST BORINGS SHEET

EA 07-187900

**BENCHMARK:** 

COUNTY

LΑ

710

IST

DEREK HIGA

SIGN OFF DATE

DESIGN OVERSIGHT ENGINEER

OGS CIVIL LOG OF TEST BORINGS SHEET

NGS Benchmark used. Pt MF36F: N1847799.06, E6512460.75, Elevation 407.29' MF 36F: A 3" Brass disk stamped "Metropolitan Water District of Southern California MF 36F 1989", set in the top of the bridge curb at the northwest corner of the Hellman Avenue overcrossing of the Long Beach Freeway (I-710). 68' west of the center of the south bound lanes of the freeway, 25' north of the centerline of Hellman Avenue, 13' east of the west end of the bridge, 1'north of the north curb face. NGS PID EW9453.

DRAWN BY:

CHECKED BY:

EDELYNE MIGUEL

DAN JANKLY

R. CHAVEZ

FIELD INVESTIGATION BY

DATE: 1/19/09 - 1/23/09

ORIGINAL SCALE IN INCHES

Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. Vertical control based on NAVD-88. P+ 0153 N1845410.50, E6509860.21, Pt 0617 N1858044.3, E6491094.23.

Units are in U.S. survey feet.

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- 5) All bedding and other structural angles are measured from horizontal.
- 6) Plan view shown on Boring Location Plan sheet 1 of 2.

COUNTY LΑ 710 3/18/10 GEOTECHNICAL ENGINEER DATE MAHESWARAN RAVEENDRA No. GE 2743 Exp. 06-30-11 PLANS APPROVAL DATE The State of California or its officers or agents & OTECHNICAL hall not be responsible for the accuracy or completeness of electronic copies of this plan sheet. CH2M HILL 6 HUTTON CENTRE DRIVE, SUITE 700 SANTA ANA, CA 92707

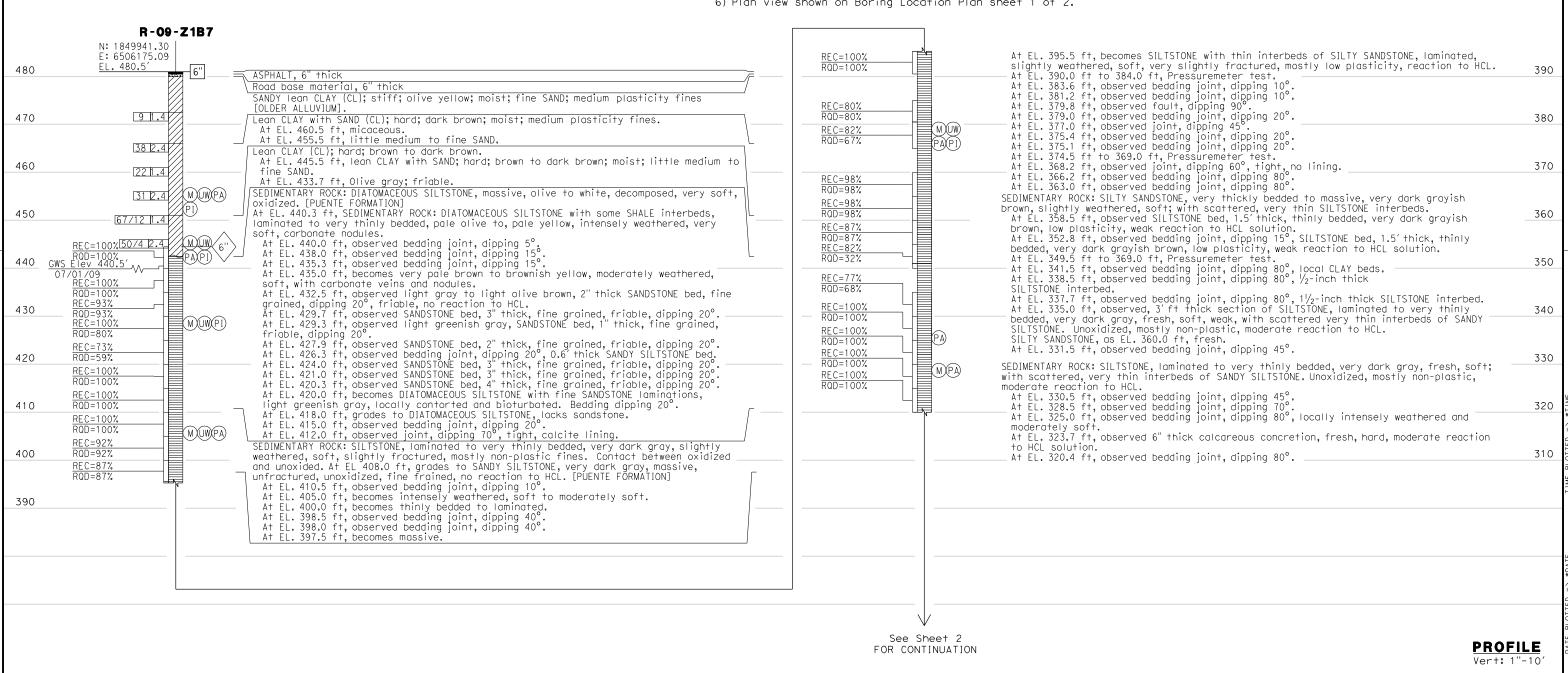
SR-710 TUNNEL TECHNICAL STUDY

LOG OF TEST BORING SHEET 1 OF 2

N/A

POST MILES

DISREGARD PRINTS BEARING EARLIER REVISION DATES



PREPARED FOR THE

STATE OF

CALIFORNIA

DEPARTMENT OF TRANSPORTATION

EA 07-187900 FILE => \$REQUEST

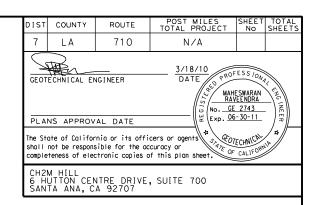
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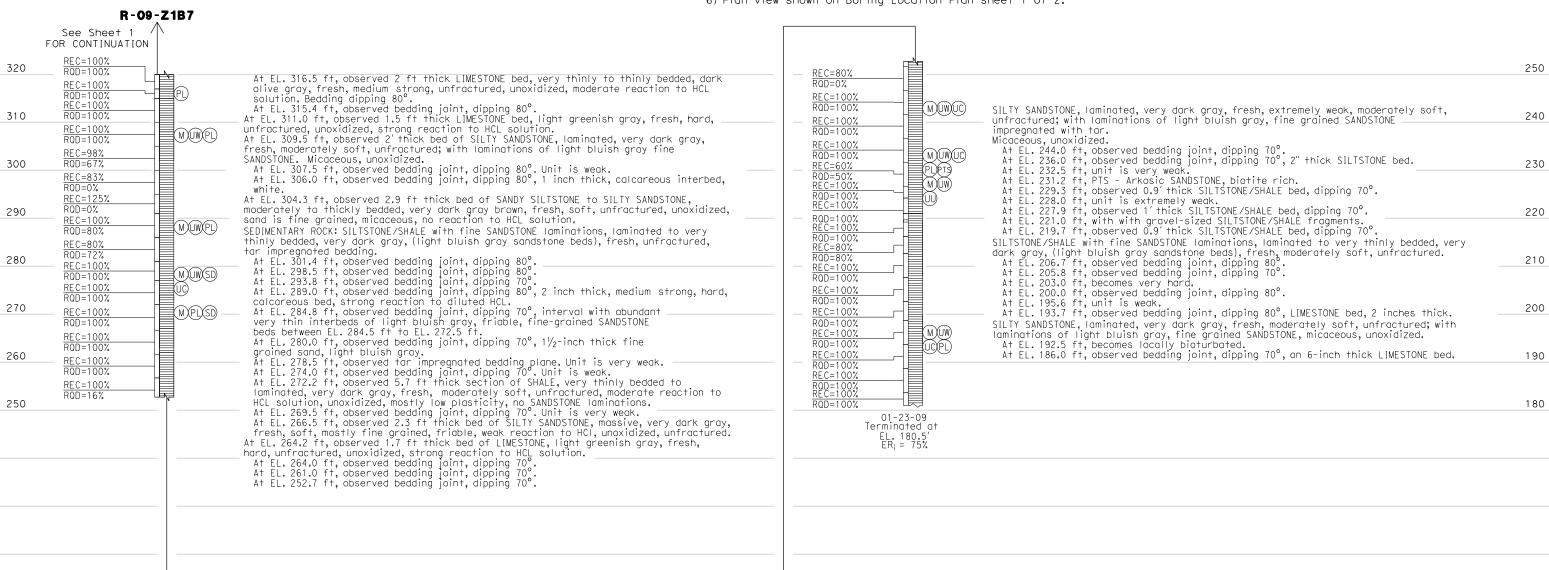
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- 6) Plan view shown on Boring Location Plan sheet 1 of 2.





**PROFILE** Vert: 1"-10

DEREK HIGA DESIGN OVERSIGHT ENGINEER	DRAWN BY:	EDELYNE MIGUEL	R. CHAVEZ FIELD INVESTIGATION BY:	PREPARED FOR THE STATE OF	MAHESWARAN RAVEENDRA	BRIDGE NO.	SR-710	TUNNEL TECH	INICAL S	STUDY
SIGN OFF DATE	CHECKED BY:	DAN JANKLY	DATE: 1/19/09 - 1/23/09	CALIFORNIA DEPARTMENT OF TRANSPORTATION	PROJECT ENGINEER	POST MILES	LOG OF	TEST BORING	SHEET 2	2 OF 2
OGS CIVIL LOG OF TEST BORINGS SHEET			ORIGINAL SCALE IN INCHES FOR REDUCED PLANS	0 1 2 3	CU EA 07-187900	DISREGARD PRI	NTS BEARING ION DATES	REVISION DATE	is .	2 2

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Units are in U.S. survey feet.

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- 6) Plan view shown on Boring Location Plan sheet 1 of 2.

DIST COUNTY ROUTE POST MILES SHEET TOTAL PROJECT NO SHEETS

7 LA 710 N/A

CERTIFIED ENGINEERING GEOLOGIST 3/18/10

DATE

PLANS APPROVAL DATE

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SHEET NOTAL SHEETS

NO KRISTOPHER P. 0. CEC 2383

Exp. 8/31/2011

CERTIFIED

ENGINEERING

GEOLOGIST

SAGGEOLOGIST

TOTAL

NO SHEETS

NO S

Vert: 1"-10'

	R-09-Z1B8					
N: E:	1850582.92 6512724.74 . 419.6'					
<u>EL</u> .		SILTY, CLAYEY SAND (SC-SM); dense; light olive brown; dry; fine SAND; little low to medium plasticity fines [RECENT ALLUVIUM].  At EL. 414.6 ft, contains trace fine GRAVEL.	REC=83% RQD=83%			
	[22]1.4	At EL. 412.6 ft, olive yellow mottled with light gray.	REC=% RQD=%			
		At EL. 404.6 ft, becomes black.	REC=100% RQD=100%	M UWPI PAUUCR	At EL. 299.6 ft, becomes moderately soft. Unit is very weak.	
	9 11.4 M PA	—/ SANDY SILT (ML); stiff; dark grayish brown; dry; few GRAVEL; little coarse to fine SAND; mostly low plasticity fines.	REC=83% RQD=83%			
WC E1 70		— SILTY SAND (SM); dark yellowish brown; medium SAND.	REC=100% RQD=100%		At EL. 289.6 ft, becomes laminated.	
WS Elev 39 07/01/09	1.4 MPI	_/ Lean CLAY (CL); soft; very dark grayish brown slightly mottled with rust staining; medium plasticity fines.	REC=100% RQD=100%	M UWSD EM)		
		SILTY SAND (SM); very dark grayish brown; fine SAND.	RQD=100% REC=100% RQD=0%		At EL. 281.6 ft, contains 6" lense of fresh, very strong, very hard.  At EL. 279.1 ft, becomes very slightly fractured, bedding plane separation.	
	[20]1.4	Poorly graded SAND (SP); medium dense; grayish brown; coarse to medium SAND; weak cementation.		PT3(SD(EM)	aa. a.o, occamer,gga.a., accame p.a	
		— SILTY, CLAYEY SAND (SC-SM); medium dense; grayish brown; trace fine GRAVEL;  → Control of the control of	REC=100% RQD=100% REC=100%	MUW -		
	33   1.4 M (W)	<ul> <li>mostly medium to fine SAND; some low plasticity fines.</li> <li>SEDIMENTARY ROCK: (SILTSTONE)/MUDSTONE, olive gray, intensely weathered, soft, unfractured, with thin (1") clay lenses. [PUENTE FORMATION].</li> </ul>	RQD=100%	UWPICR		
		WITH THIN (1 ) CIGS TOTISCS. [FOLKIE FORWATION].	REC=100% ROD=100%	MUW	At EL. 259.6 ft, becomes soft.	
	MUW		REC=98% RQD=98%		AT EL. 259.6 TT, Decomes Sott.	
	43   1.4	At EL. 364.6 ft, contains pebble to cobble.	REC=95% RQD=95%			
			REC=100% RQD=100%		At EL. 249.6 ft, with few coarse sand.	
	32 11.4	Massive, medium dark gray to dark gray, moderately weathered, soft, unfractured.	REC=100% RQD=100%	M UW SDEM	At EL. 243.1 ft, contains 3" lens of fresh, very hard.	
			REC=100% RQD=100%			
RFC=100%	79/1211.4	<b>&gt;</b>	REC=93% RQD=93%		At EL. 235.1 ft, contains 6" lens of fresh, very hard.	
RQD=0%		—— At EL. 339.3 ft, unit is very weak.	RCD=95% REC=95% RQD=95%	MUWUU	At EL. 230.0 ft, unit is very weak.	
REC=97% RQD=47% REC=0%	41 11 . 4 PAYUU		RQD=95% REC=100% RQD=100%	M UWSD EM		
REC=0% RQD=0% REC=100%		At EL. 330.6 ft, grades to greenish black.  At EL. 329.6 ft, becomes slightly fractured, not healed, dipping 60 to 50°.	RQD=100%			
REC=100% RQD=100%			Termino	01-12-09 ated at EL. 219.6' ER; = 87%		
REC=100% RQD=100%		At EL. 318.6 ft. becomes unfractured.		LIN; - 017%		
REC=50% RQD=50%		At EL. 314.6 ft, becomes slightly fractured, not healed, dipping 50°. Unit is very weak.				
REC=100% RQD=100%	PDPAUU M)UW)	At EL. 309.6 ft, becomes unfractured.				

ENGINEERING SERVICES DIVISION OF ENGINEERING SERVICES Structure design **GEOTECHNICAL SERVICES** SR-710 TUNNEL TECHNICAL STUDY STATE OF N/A FUNCTIONAL SUPERVISOR DRAWN BY: E. MIGUEL FIELD INVESTIGATION BY: K. BARKER **CALIFORNIA** POST MILES LOG OF TEST BORING SHEET 1 OF 1 NAME: SHIVA KARIMI DATE: 1/06/09 - 1/12/09 DEPARTMENT OF TRANSPORTATION **DESIGN BRANCH** CHECKED BY: J. PRATT ORIGINAL SCALE IN INCHES FOR REDUCED PLANS CU EA 07-187900 DISREGARD PRINTS BEARING EARLIER REVISION DATES \_ OGS CIVIL LOG OF TEST BORINGS SHEET

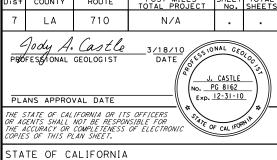
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Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. Vertical control based on North American Vertical Datum 1988. Pt 0153: N1845410.50, E6509860.21 Pt 0617: N1858044.30, E6491094.23

Units are in U.S survey feet.

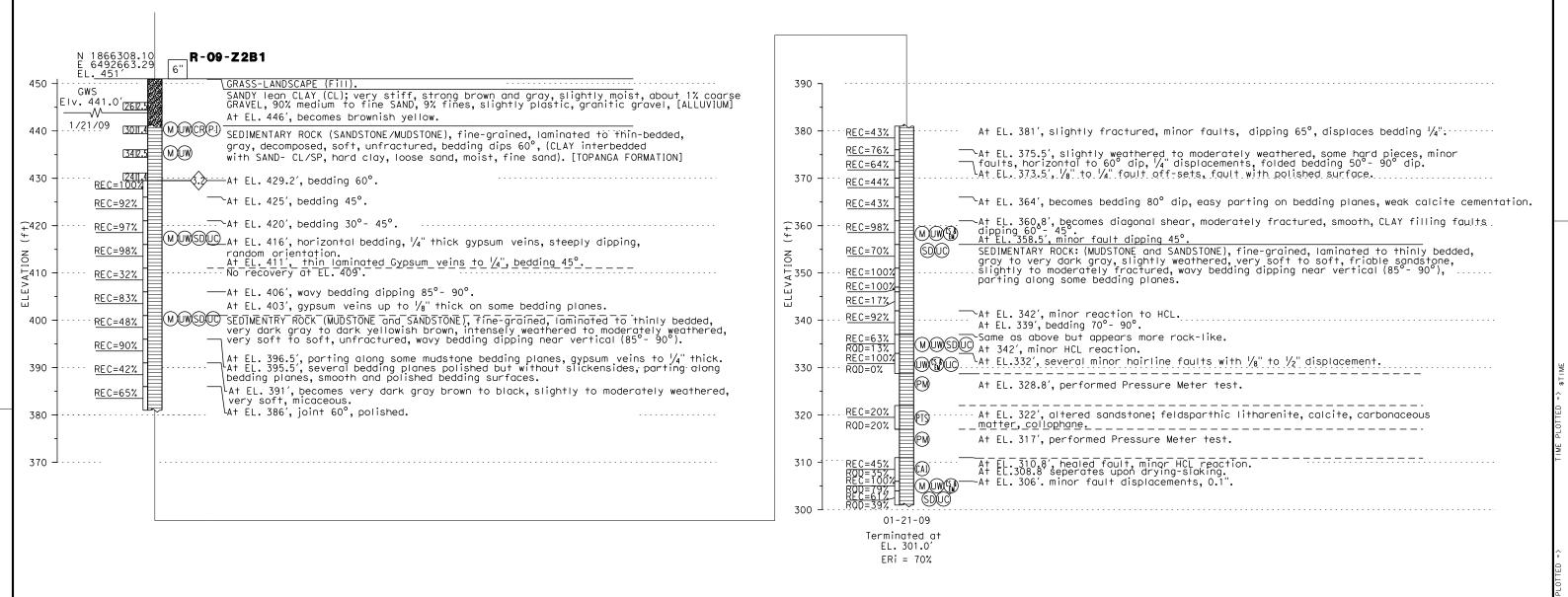
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- 6) Plan view shown on Boring Location Plan sheet 1 of 2.



STATE OF CALIFORNIA DEPARTMENT OF TRANSPORTATION, DISTRICT-7 100 S. MAIN STREET, LOS ANGELES, CA 90012

EARTH MECHANICS, INC. 17660 NEWHOPE STREET, SUITE E FOUNTAIN VALLEY, CA 92708



PROFILE
VERTICAL 1" = 10'

DEREK HIGA DESIGN OVERSIGHT ENGINEER	DRAWN BY:	K. THANT	B. SCHELL, J. CASTLE FIELD INVESTIGATION BY:			ARED FO	F	H. LAW	BRIDGE NO.	SR-7	10	TUNNE	L TEC	HNICAL	ST	UDY
SIGN OFF DATE	CHECKED BY:	B. SCHELL	1/13/09 - 1/21/09 DATE:			LIFOR		PROJECT ENGINEER	POST MILE N/A	LOG	OF	TEST	BORING	SHEET	· 1 (	OF 1
OGS CIVIL LOG OF TEST BORINGS SHEET				ORIGINAL SCALE IN INCHES FOR REDUCED PLANS	1 1	'	, ,	CU .	DISREGARD PR EARLIER REVI	INTS BEARING		REVISION DA	ATES (PRELIMINARY STAC	E ONLY)		SHEET OF

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FILE => \$REQUEST

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DIST COUNTY ROUTE POST MILES TOTAL PROJECT NO SHEETS

7 LA 710 N/A

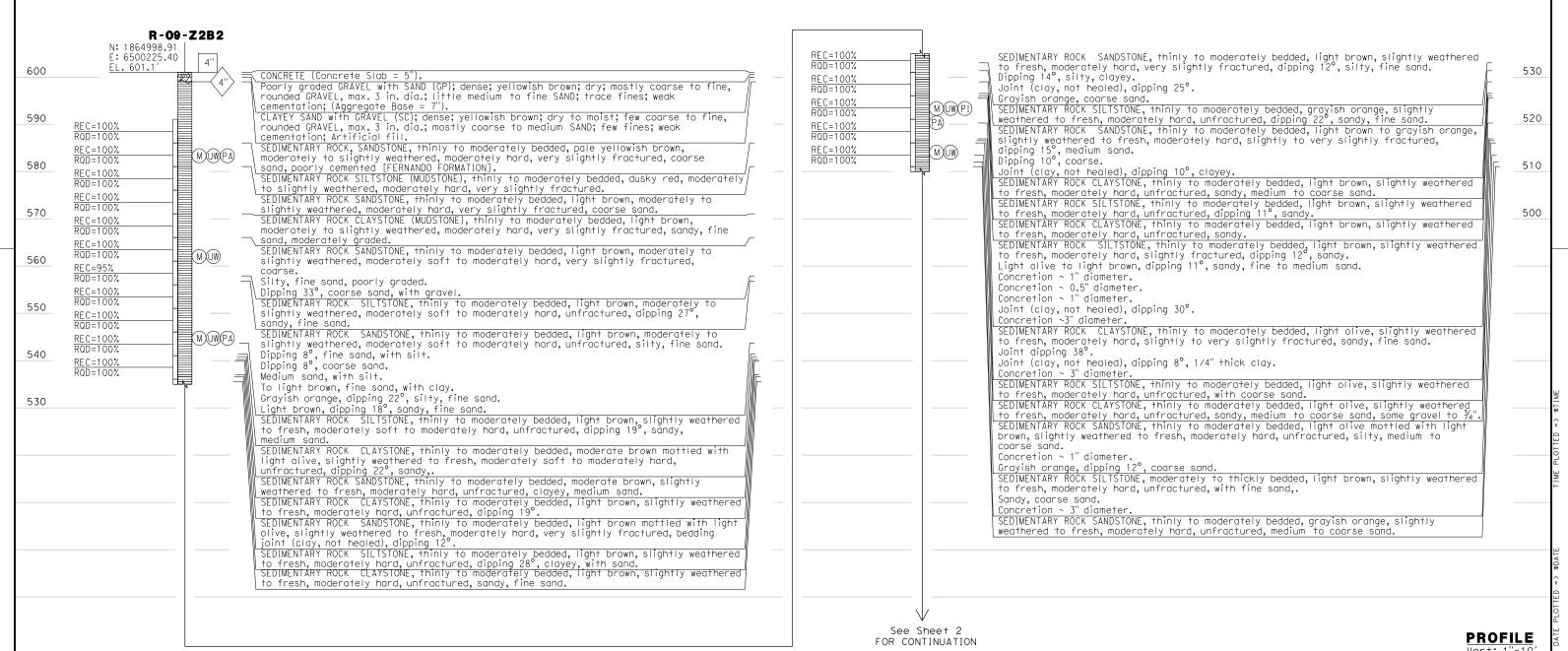
3/18/10

DATE

CERTIFIED ENGINEERING GEOLOGIST DATE

PLANS APPROVAL DATE

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**ENGINEERING SERVICES GEOTECHNICAL SERVICES** DIVISION OF ENGINEERING SERVICES SR-710 TUNNEL TECHNICAL STUDY STATE OF N/A STRUCTURE DESIGN UNCTIONAL SUPERVISOR DRAWN BY: F. MIGUEL FIELD INVESTIGATION BY: M. SALISBURY CALIFORNIA POST MILES LOG OF TEST BORING SHEET 1 OF 4 NAME: SHIVA KARIMI DATE: 1/26/09 - 2/11/09 DEPARTMENT OF TRANSPORTATION **DESIGN BRANCH** CHECKED BY: K. BARKER ORIGINAL SCALE IN INCHES FOR REDUCED PLANS DISREGARD PRINTS BEARING EARLIER REVISION DATES OGS CIVIL LOG OF TEST BORINGS SHEET EA 07-187900

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See Sheet 3

FOR CONTINUATION

FILE => \$REQUEST

6) Plan view shown on Boring Location Plan sheet 1 of 2.

DIST COUNTY ROUTE POST MILES SHEET TOTAL PROJECT NO SHEETS

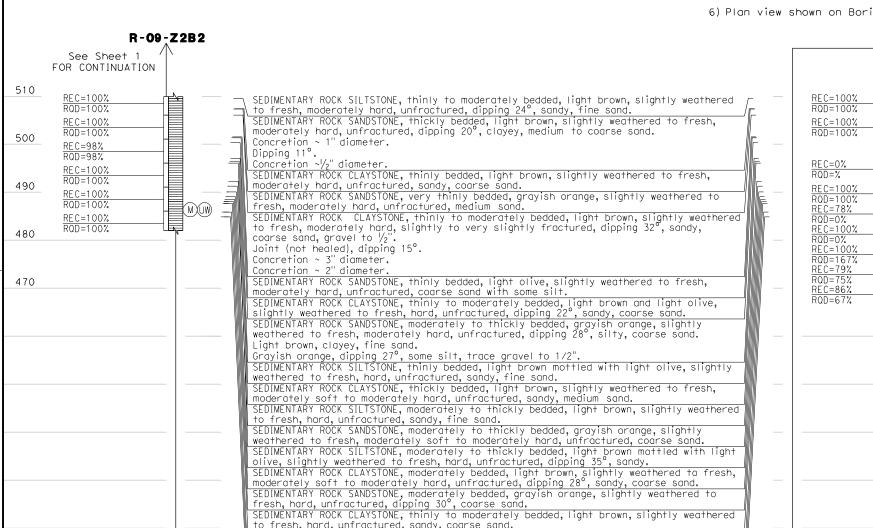
7 LA 710 N/A

CERTIFIED ENGINEERING GEOLOGIST DATE

PLANS APPROVAL DATE

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**PROFILE** 



to fresh, hard, unfractured, sandy, coarse sand.

SEDIMENTARY ROCK SANDSTONE, thickly bedded, dark yellowish brown to pale olive, slightly weathered to fresh, hard, unfractured, dipping 22°, coarse.

SEDIMENTARY ROCK SILTSTONE, moderately bedded, dark yellowish brown, slightly weathered

SEDIMENTARY ROCK SANDSTONE, thinly bedded, dusky yellow, slightly weathered to fresh, moderately soft to moderately hard, unfractured, dipping 39°, medium to coarse sand

Laminated to very thinly bedded.

to fresh, hard, unfractured.

SEDIMENTARY ROCK CONGLOMERATE, moderately bedded, grayish orange, slightly weathered to 480 fresh, moderately soft to moderately hard, unfractured, clast supported, sand matrix, gravel to 3", cobbles to 4". SEDIMENTARY ROCK CLAYSTONE, thickly bedded, light brown to light olive, slightly weathered to fresh, hard, unfractured, sandy, coarse sand. 470 SEDIMENTARY ROCK SANDSTONE, moderately bedded, light brown, slightly weathered to fresh, moderately soft to moderately hard, unfractured, silty, clayey, coarse sand. SEDIMENTARY ROCK SILTSTONE, moderately bedded, light brown, slightly weathered to fresh, hard, unfractured, sandy, fine sand.

SEDIMENTARY ROCK SANDSTONE, thickly bedded, light brown, slightly weathered to fresh, 460 moderately soft to moderately hard, unfractured, clayey, coarse sand. Dusky yellow, dipping 32°, coarse sand. Light brown to light olive, hard, coarse sand, with silt. 450 SEDIMENTARY ROCK CONGLOMERATE, thinly to moderately bedded, grayish orange, slightly weathered to fresh, hard, unfractured, clast supported, coarse sand matrix, gravel to 1/2" SEDIMENTARY ROCK SILTSTONE, thinly to moderately bedded, light olive, slightly weathered to fresh, hard, unfractured, sandy, fine sand. SEDIMENTARY ROCK CLAYSTONE, thinly to moderately bedded, light brown, slightly weathered 440 to fresh, moderately hard, unfractured, sandy, coarse sand. SEDIMENTARY ROCK SANDSTONE, thinly to moderately bedded, dusky yellow green, slightly weathered to fresh, very weak, moderately hard, unfractured, coarse sand.

SEDIMENTARY ROCK CLAYSTONE, thickly bedded, light brown, slightly weathered to fresh, 430 moderately hard, very slightly fractured, sandy, coarse sand. Light brown mottled with light olive, joint (CL, not healed), dipping 38° SEDIMENTARY ROCK SILTSTONE, thinly to moderately bedded, light olive, slightly weathered to fresh, moderately hard, unfractured, sandy, fine sand.
SEDIMENTARY ROCK CLAYSTONE, very thinly to thinly bedded, light brown, slightly weathered to fresh, moderately hard, unfractured. SEDIMENTARY ROCK SANDSTONE, thickly bedded, grayish orange to dusky yellow green, slightly weathered to fresh, moderately hard, unfractured, with silt and clay, coarse sand with subround gravel to 1". SEDIMENTARY ROCK CONGLOMERATE, moderately to thickly bedded, dusky yellow green, slightly weathered to fresh, moderately hard, unfractured, clast supported, coarse sond matrix, gravel to  $\frac{1}{2}$ . SEDIMENTARY ROCK SANDSTONE, thickly bedded, pale yellowish brown to grayish orange, slightly weathered to fresh, moderately soft, unfractured, dipping 32°. Coarse sand with subround gravel to  $\frac{1}{2}$ . Grayish orange, coarse sand with subround gravel to  $\frac{1}{2}$ ". SEDIMENTARY ROCK CONGLOMERATE, moderately to thickly bedded, grayish orange, slightly weathered to fresh, moderately soft, unfractured, round gravel to 2", clast supported, coarse sand matrix. SEDIMENTARY ROCK SANDSTONE, thickly bedded, grayish orange, slightly weathered to fresh, moderately soft, unfractured, coarse sand. SEDIMENTARY ROCK CONGLOMERATE, moderately to thickly bedded, yellowish gray, slightly weathered to fresh, moderatelý soft, unfractured, mátrix supported subround grável to 1", medium to coarse sand mátrix.

**ENGINEERING SERVICES GEOTECHNICAL SERVICES** DIVISION OF ENGINEERING SERVICES SR-710 TUNNEL TECHNICAL STUDY STATE OF N/A STRUCTURE DESIGN UNCTIONAL SUPERVISOR DRAWN BY: F. MIGUEL FIELD INVESTIGATION BY: M. SALISBURY CALIFORNIA POST MILES LOG OF TEST BORING SHEET 2 OF 4 NAME: SHIVA KARIMI DATE: 1/26/09 - 2/11/09 DEPARTMENT OF TRANSPORTATION **DESIGN BRANCH** CHECKED BY: K. BARKER ORIGINAL SCALE IN INCHES FOR REDUCED PLANS DISREGARD PRINTS BEARING EARLIER REVISION DATES OGS CIVIL LOG OF TEST BORINGS SHEET EA 07-187900

NGS Benchmark used. Pt MF36F: N1847799.06, E6512460.75, Elevation 407.29' MF 36F: A 3" Brass disk stamped "Metropolitan Water District of Southern California MF 36F 1989", set in the top of the bridge curb at the northwest corner of the Hellman Avenue overcrossing of the Long Beach Freeway (I-710). 68' west of the center of the south bound lanes of the freeway, 25' north of the centerline of Hellman Avenue. 13' east of the west end of the bridge, 1'north of the north curb face. NGS PID EW9453.

Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. Vertical control based on NAVD-88. P+ 0153 N1845410.50, E6509860.21, Pt 0617 N1858044.3, E6491094.23.

**ENGINEERING SERVICES** 

UNCTIONAL SUPERVISOR

Units are in U.S. survey feet.

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- 4) To convert from California ring sampler blow count to equivalent standard penetration test sampler blow count, for granular and cohesive soil, multiply

DIVISION OF ENGINEERING SERVICES

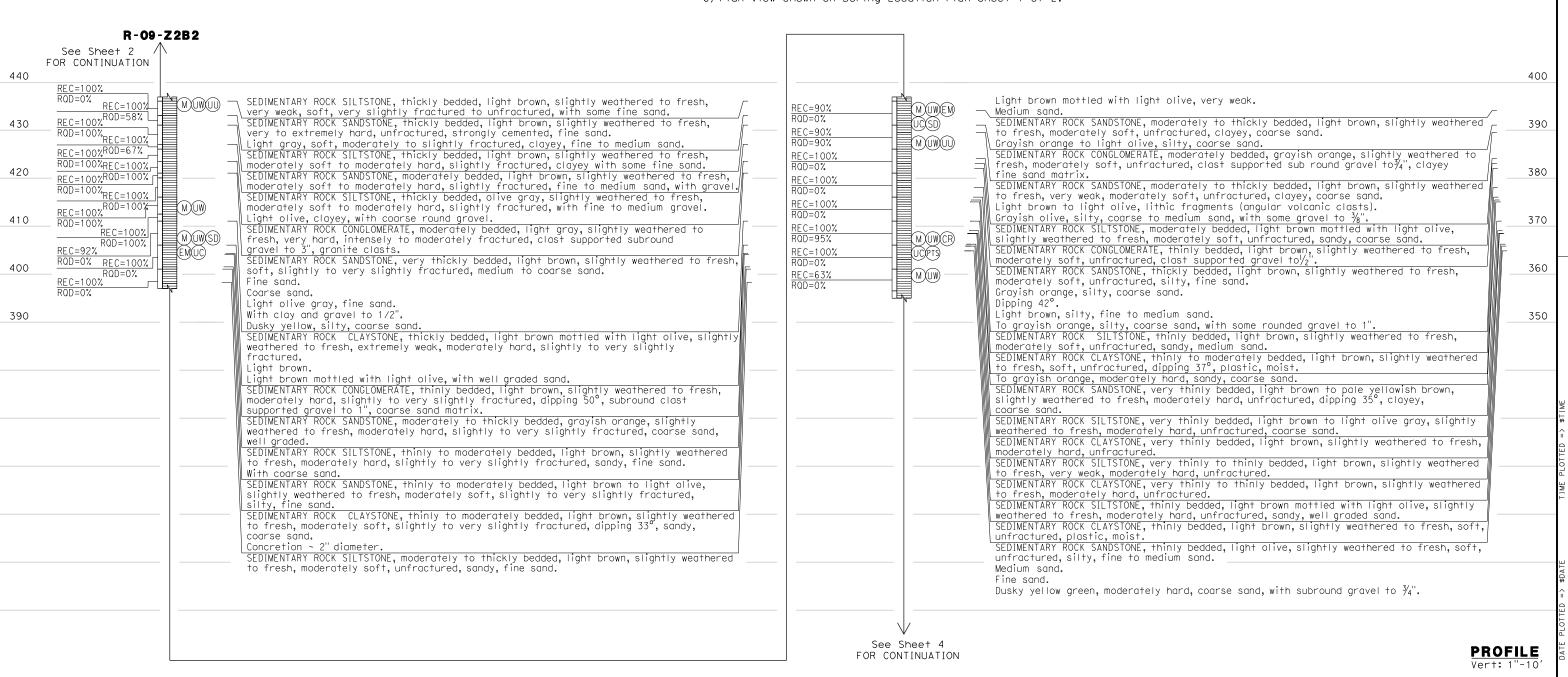
STRUCTURE DESIGN

N/A

- 5) All bedding and other structural angles are measured from horizontal.
- 6) Plan view shown on Boring Location Plan sheet 1 of 2.

COUNTY LΑ 710 CERTIFIED ENGINEERING GEOLOGIST MICHAEL A. SALISBURY No. CEG 2462 Exp. 2/28/2011 PLANS APPROVAL DATE CERTIFIED ENGINEERING GEOLOGIST he State of California or its officers or agents hall not be responsible for the accuracy or completeness of electronic copies of this plan sheet

SR-710 TUNNEL TECHNICAL STUDY



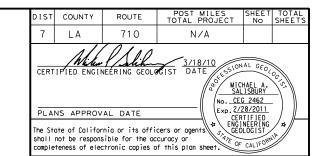
DRAWN BY: F. MIGUEL FIELD INVESTIGATION BY: M. SALISBURY CALIFORNIA POST MILES LOG OF TEST BORING SHEET 3 OF 4 NAME: SHIVA KARIMI DATE: 1/26/09 - 2/11/09 DEPARTMENT OF TRANSPORTATION **DESIGN BRANCH** CHECKED BY: K. BARKER ORIGINAL SCALE IN INCHES FOR REDUCED PLANS DISREGARD PRINTS BEARING EARLIER REVISION DATES OGS CIVIL LOG OF TEST BORINGS SHEET EA 07-187900 FILE => \$REQUEST

STATE OF

**GEOTECHNICAL SERVICES** 

# **BENCHMARK:** NGS Benchmark used. Pt MF36F: N1847799.06, E6512460.75, Elevation 407.29' MF 36F: A 3" Brass disk stamped "Metropolitan Water District of Southern California MF 36F 1989", set in the top of the bridge curb at the northwest corner of the Hellman Avenue overcrossing of the Long Beach Freeway (I-710). 68' west of the center of the south bound lanes of the freeway, 25' north of the centerline of Hellman Avenue, 13' east of the west end of the bridge, 1' north of the north curb face. NGS PID EW9453. Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. Vertical control based on NAVD-88. P+ 0153 N1845410.50, E6509860.21, P+ 0617 N1858044.3, E6491094.23. Units are in U.S. survey feet.

- NOTES:
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- 6) Plan view shown on Boring Location Plan sheet 1 of 2.



	R-09-Z	282	
360	See Sheet 3 FOR CONTINUATION		360
350	REC=0% RQD=% REC=73% REC=100% RQD=0% RQD=0% REC=65%	No sample.  Silty, medium to coarse sand, with some subround gravel to 1/2".  SEDIMENTARY ROCK CLAYSTONE, moderately to thickly bedded, light brown, slightly weathered to fresh, very weak, moderately hard, unfractured, sandy, coarse sand.	350
340	REC=48% RQD=0%	SEDIMENTARY ROCK CLAYSTONE, moderately bedded, light brown, slightly weathered to fresh, very weak, moderately bedded, dusky yellow green, slightly weathered to fresh, soft, unfractured, dipping 44°, silty, medium sand.  SEDIMENTARY ROCK CONGLOMERATE, thinly bedded, dusky yellow green, slightly weathered to fresh, soft, unfractured, clast supported subround gravel to 1" medium sand matrix.	340
330	REC=83% RQD=0% REC=83% RQD=0%	SEDIMENTARY ROCK SANDSTONE, Thickly bedded, dusky yethow green, stigntly wedthered to fresh, soft, unfractured, medium sand, with subround gravel to ½".    SEDIMENTARY ROCK CONGLOMERATE, thinly to moderately bedded, dusky yellow green, slightly wedthered to fresh, soft, unfractured, clast supported subround gravel to 1", medium sand matrix.	330
320	REC=50%	SEDIMENTARY ROCK SANDSTONE, thickly bedded, dusky yellow green, slightly weathered to fresh, soft, unfractured, fine sand.  Silty.	320
310	RQD=0%	soft, unfractured, clast supported round gravel to 1", coarse sand matrix.  SEDIMENTARY ROCK SANDSTONE, very thickly bedded, dusky green, slightly weathered to fresh, soft, unfractured, silty, coarse sand.  Silty, fine sand.	310
		Medium sand. Fine sand.  Dusky yellow green, silty, fine sand.	

PROFILE	
Vert: 1"-10'	

ENGINEERING SERVICES		GEOTECHNICAL SERVICES		DIVISION OF ENGINEERING SERVICES	BRIDGE NO.	00 - 40			A =
	<u> </u>	GEOTEONINIONE SERVICES	STATE OF	STRUCTURE DESIGN	N/A	SR-710	TUNNEL	TECHNICAL	STUDY
FUNCTIONAL SUPERVISOR	DRAWN BY: E. MIGUEL	FIELD INVESTIGATION BY: M. SALISBURY	CALIFORNIA		POST MILES				
NAME: SHIVA KARIMI	CHECKED BY: K. BARKER	DATE: 1/26/09 - 2/11/09	DEPARTMENT OF TRANSPORTATION	DESIGN BRANCH	N/A	LOG OF	TEST BO	RING SHEET	4 OF 4
OGS CIVIL LOG OF TEST BORINGS SHEET		ORIGINAL SCALE IN INCHES FOR REDUCED PLANS	0 1 2 3	CU EA 07-187900	DISREGARD PRI EARLIER REVIS	INTS BEARING SION DATES		REVISION DATES	4 4

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#### NOTES:

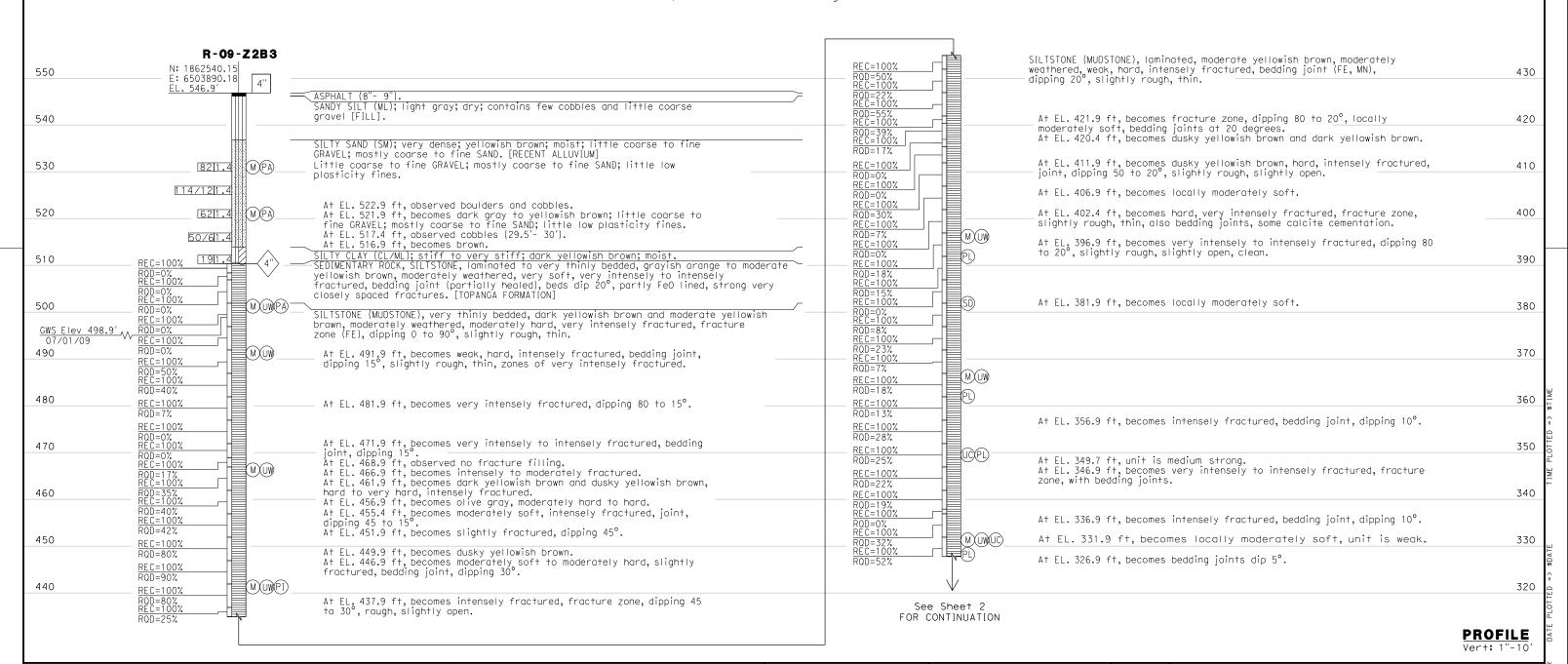
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- 6) Plan view shown on Boring Location Plan sheet 1 of 2.

PLANS APPROVAL DATE

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POST MILES
TOTAL PROJECT
No SHEET NOTAL SHEETS

A/18/10



**ENGINEERING SERVICES GEOTECHNICAL SERVICES** DIVISION OF ENGINEERING SERVICES SR-710 TUNNEL TECHNICAL STUDY STATE OF N/A STRUCTURE DESIGN FUNCTIONAL SUPERVISOR FIELD INVESTIGATION BY: K. LAI, T. HALDA, DRAWN BY: K. REYES CALIFORNIA POST MILES DATE: 3/3/09 - 3/12/09 K. BAKER AME: SHIVA KARIMI LOG OF TEST BORING SHEET 1 OF 2 CHECKED BY: M. SALISBURY DEPARTMENT OF TRANSPORTATION **DESIGN BRANCH** ORIGINAL SCALE IN INCHES DISREGARD PRINTS BEARING EARLIER REVISION DATES OGS CIVIL LOG OF TEST BORINGS SHEET EA 07-187900

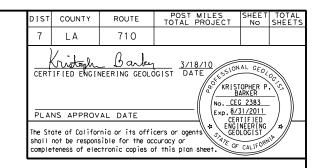
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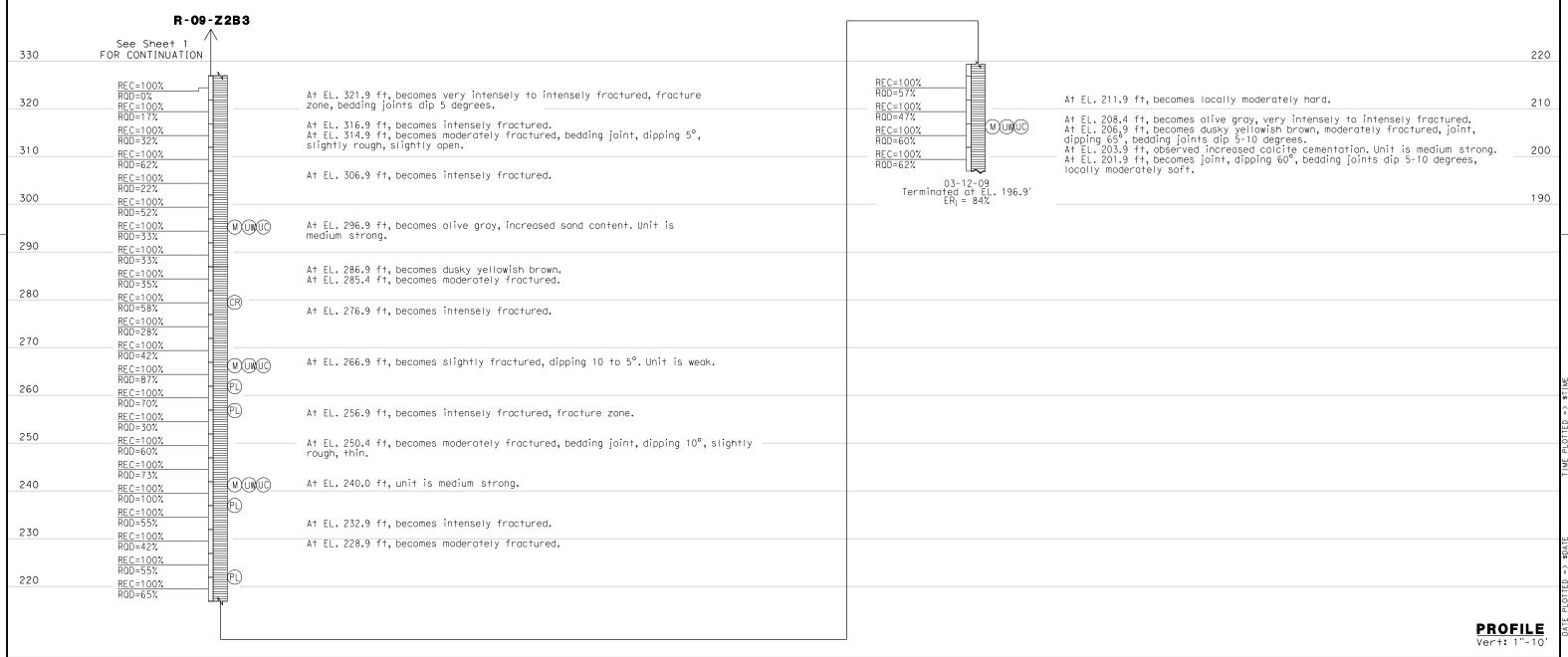
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**ENGINEERING SERVICES** DIVISION OF ENGINEERING SERVICES STRUCTURE DESIGN **GEOTECHNICAL SERVICES** SR-710 TUNNEL TECHNICAL STUDY STATE OF N/A FUNCTIONAL SUPERVISOR FIELD INVESTIGATION BY: K. LAI, T. HALDA DRAWN BY: K. REYES CALIFORNIA POST MILES LOG OF TEST BORING SHEET 2 OF 2 AME: SHIVA KARIMI DATE: 3/3/09 - 3/12/09 CHECKED BY: M. SALISBURY DEPARTMENT OF TRANSPORTATION **DESIGN BRANCH** ORIGINAL SCALE IN INCHES DISREGARD PRINTS BEARING EARLIER REVISION DATES OGS CIVIL LOG OF TEST BORINGS SHEET EA 07-187900

OGS CIVIL LOG OF TEST BORINGS SHEET

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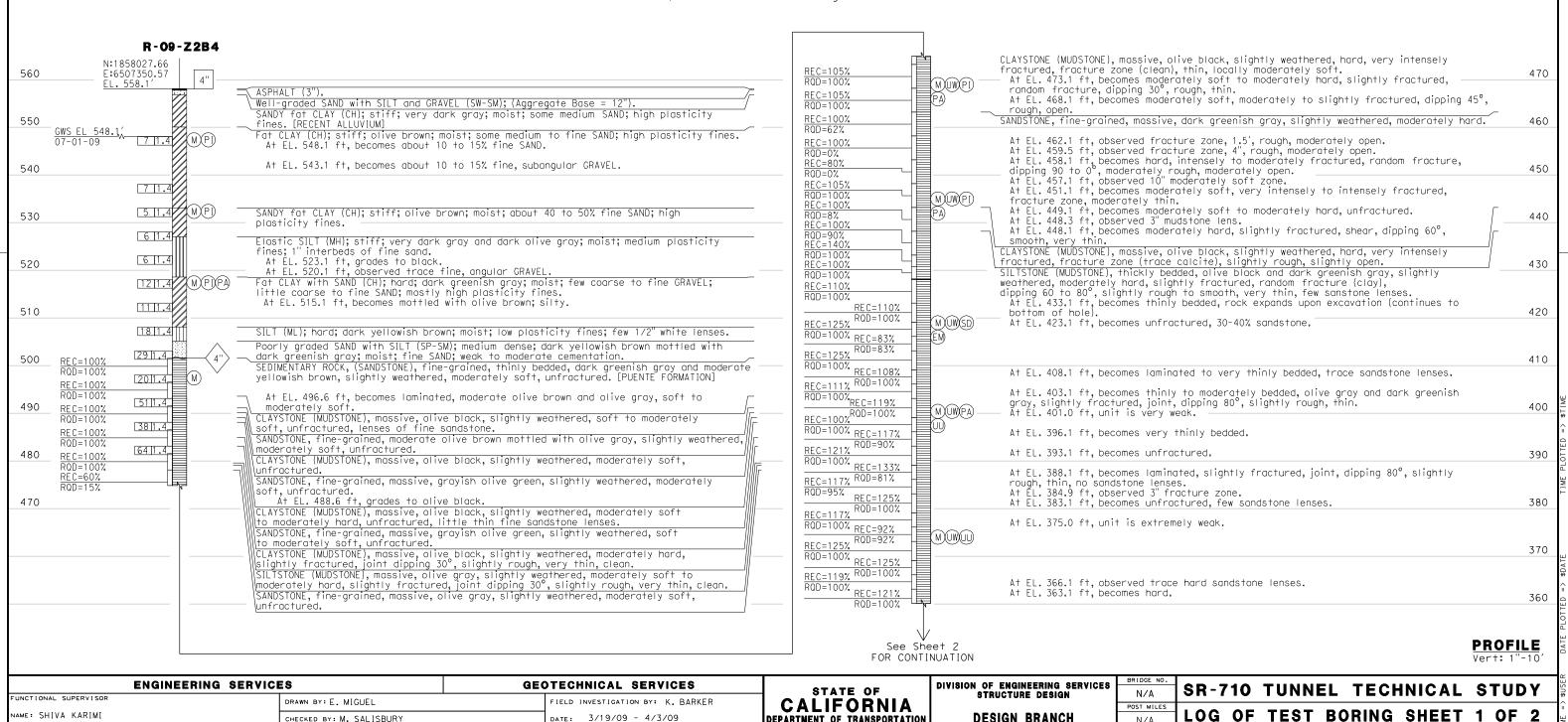
CHECKED BY: M. SAI ISBURY

Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. Vertical control based on NAVD-88. P+ 0153 N1845410.50, E6509860.21, Pt 0617 N1858044.3, E6491094.23.

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COUNTY ROUTE LΑ 710 3/18/10 CERTIFIED ENGINEERING GEOLOGIST DATE KRISTOPHER P. No. CEC 2383 Exp. 8/31/2011 PLANS APPROVAL DATE CERTIFIED ENGINEERING GEOLOGIST he State of California or its officers or agents hall not be responsible for the accuracy or completeness of electronic copies of this plan sheet



DEPARTMENT OF TRANSPORTATION

**DESIGN BRANCH** 

EA 07-187900 FILE => \$REQUEST DISREGARD PRINTS BEARING EARLIER REVISION DATES

DATE: 3/19/09 - 4/3/09

ORIGINAL SCALE IN INCHES FOR REDUCED PLANS

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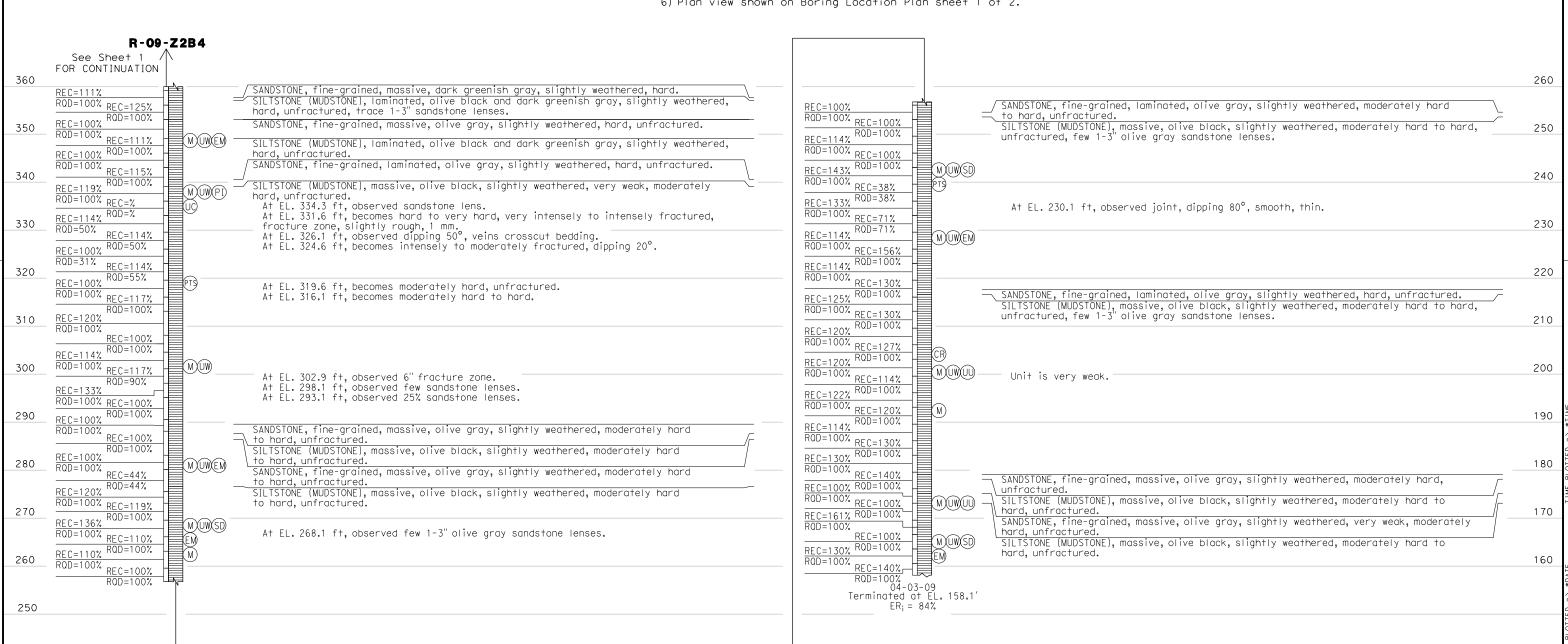
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COUNTY LΑ 710 3/18/10 CERTIFIED ENGINEERING GEOLOGIST DATE KRISTOPHER P. No. CEC 2383 Exp. 8/31/2011 PLANS APPROVAL DATE CERTIFIED ENGINEERING GEOLOGIST The State of California or its officers or agents hall not be responsible for the accuracy or completeness of electronic copies of this plan sheet

**PROFILE** 



**ENGINEERING SERVICES GEOTECHNICAL SERVICES** DIVISION OF ENGINEERING SERVICES SR-710 TUNNEL TECHNICAL STUDY STATE OF N/A STRUCTURE DESIGN FUNCTIONAL SUPERVISOR FIELD INVESTIGATION BY: K. RARKER DRAWN BY: F. MIGUEL CALIFORNIA POST MILES NAME: SHIVA KARIMI LOG OF TEST BORING SHEET 2 OF 2 DATE: 3/19/09 - 4/3/09 DEPARTMENT OF TRANSPORTATION **DESIGN BRANCH** CHECKED BY: M. SAI ISBURY N/A ORIGINAL SCALE IN INCHES FOR REDUCED PLANS DISREGARD PRINTS BEARING EARLIER REVISION DATES OGS CIVIL LOG OF TEST BORINGS SHEET EA 07-187900

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**ENGINEERING SERVICES** 

DRAWN BY: F. MIGUEL

CHECKED BY: M. SALISBURY

FUNCTIONAL SUPERVISOR

OGS CIVIL LOG OF TEST BORINGS SHEET

NAME: SHIVA KARIMI

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DIVISION OF ENGINEERING SERVICES
STRUCTURE DESIGN

**DESIGN BRANCH** 

CU EA 07-187900

FILE => \$REQUEST

N/A

POST MILES

DISREGARD PRINTS BEARING EARLIER REVISION DATES

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POST MILES SHEET TOTAL NO N/A

A 710 N/A

A 710 N/A

A 710 N/A

A 710 DATE

A 710 N/A

A 710 DATE

SR-710 TUNNEL TECHNICAL STUDY

LOG OF TEST BORING SHEET 1 OF 2

		6) Plan vie	w shown on Boring Location Plar	n sheet 1 of 2.	
	R-09-Z2B5				
	N: 1853050.81 E: 6511831.88				
450	EL. 452.4'	ASPHALT (4").		CLAYEY SAND (SC); medium dense; light brown; moist; fine SAND; nonplastic to low	7.40
450		SILTY CLAY (CL/ML); soft to medium stiff; dark brown; moist; low plasticity fines. [ALLUVIUM]	1411.4	plasticity fines.	340
	[14]1.4	SANDY SILT (ML); medium stiff; dark brown; moist; fine SAND; nonplastic fines.	[11]1.4	SANDY lean CLAY (CL); stiff; light brown to light gray; moist to wet; fine SAND; low plasticity fines.	
440 <u>G</u>	SWS EL 441.6 7 11.4 MPI	Lean CLAY with SAND (CL); stiff; dark brown; moist.	[14]1.4	At EL. 332.4 ft, grades to very stiff.	330
170 0		SILTY SAND (SM); dense; yellowish brown; moist; fine SAND.		Fat CLAY (CH); very stiff; light brown to light gray; moist to wet.	330
	[32]1.4 M		MUWPI		
430	[33]1.4	At EL. 431.4 ft, contains coarse to fine SAND.	[27]1.4	SANDY lean CLAY (CL); very stiff; light brown to light gray; moist to wet.	320
	[22]1.4	CANDY CLIT (All ) be been been been been been been been	REC=100% 43 1.4	very stiff to bord	
		SANDY SILT (ML); hard; light brown; moist; some fine SAND; mostly nonplastic to low plasticity fines; interbedded with silty sand (SM).	REC=100% [43]1.4 4" RQD=100% REC=100% PROD=100%	very stiff to hard SEDIMENTARY ROCK: CLAYSTONE (MUDSTONE), thinly bedded, moderate olive	
420	[19]1.4 MPA	CILITY CAND (CM): modium dooses vellowish brows moist; modium to fine CAND; populartie	RQD=100%	brown and olive gray, slightly weathered, extremely weak, very soft, unfractured. [PUENTE FORMATION].	310
	[14]1.4	SILTY SAND (SM); medium dense; yellowish brown; moist; medium to fine SAND; nonplastic fines.	REC=90% ROD=90%	At EL. 312.4 ft, becomes laminated, soft.	
			ROD=90% REC=100% MUWUU PIXPA)	At EL. 302.4 ft, becomes unfractured.	-
410	[13]1.4	CANDY CHT (AH) and CC to make a difficultive expert on the Con Class CDAVELL Class CANDS	REC=100% PIPA PIPA PIPA PIPA PIPA PIPA PIPA PIP	AT EL. 302.4 TT, Decomes diffractured.	300
	[18]1.4	SANDY SILT (ML); stiff to very stiff; olive gray; moist; few fine GRAVEL; fine SAND; nonplastic to low plasticity fines.	REC=100%_U	At EL. 297.9 ft, becomes soft to moderately soft.	-
400	[18]1.4 M/PA	SILTY SAND (SM); medium dense; light brown; moist; little fine GRAVEL; fine SAND. At EL. 401.4 ft, with trace fine GRAVEL; mostly fine SAND; some low plasticity fines.	REC=100% ROD=100%	SEDIMENTARY ROCK: CLAYSTONE (MUDSTONE), massive, black, slightly weathered, soft to moderately soft, unfractured.	290
	[34]1.4	At EL. 397.4 ft, grades to medium to fine SAND.	REC=120%	At EL. 287.4 ft, becomes very slightly fractured, shear, dipping $45^{\circ}$ .	
390	1911.4	SILTY CLAY (CL/ML); stiff to very stiff; olive brown to olive gray; moist; low plasticity fines.	REC=100% RQD=100% REC=108%	At EL. 282.4 ft, becomes moderately soft, unfractured.	280
	[14]1.4		REC=100% WWW WWW MWWW	At EL. 277.4 ft, becomes very slightly fractured, joint, dipping $70^{\circ}$ , few subhorizontal silty laminations. Extremely weak.	<u>u</u>
380	[24]1.4		REC=100% REC=100%ROD=33%		270
	2011.4 MPI	Fat CLAY (CH); very stiff; olive gray; moist; medium to high plasticity fines.	RQD=100%	At EL. 264.4 ft, unit is very weak.	î
370	1211.4	SILTY CLAY (CL/ML); stiff to very stiff; light brown; moist; low plasticity fines.	REC=100% MUWUC		260
	[20]1.4 (M)		REC=104% ROD=100% REC=100%	At EL. 258.4 ft, contains about 50% siltstone, weakly subvertically laminated.	
		very stiff; olive gray; low to medium plasticity fines.	REC=106% ROD=75% ROD=100%	At EL. 253.4 ft, becomes unfractured.	ĬŽ
360	1711.4		RQD=100%		250
	[16]1.4				
350	2211.4				
-	PA CUI				u E
		At EL. 347.4 ft, with few fine SAND.			₩
340					
					H
			See Sheet 2		
			FOR CONTINUATION		<b>DFILE</b> : 1"-10'
				VCI 16	• 1 10

STATE OF

**CALIFORNIA** 

DEPARTMENT OF TRANSPORTATION

GEOTECHNICAL SERVICES

FIELD INVESTIGATION BY: K. LAI, M.ISLAM, K. BARKER
DATE: 4/06/09 - 4/14/09

ORIGINAL SCALE IN INCHES FOR REDUCED PLANS

NGS Benchmark used. Pt MF36F: N1847799.06, E6512460.75, Elevation 407.29' MF 36F: A 3" Brass disk stamped "Metropolitan Water District of Southern California MF 36F 1989", set in the top of the bridge curb at the northwest corner of the Hellman Avenue overcrossing of the Long Beach Freeway (I-710). 68' west of the center of the south bound lanes of the freeway, 25' north of the centerline of Hellman Avenue, 13' east of the west end of the bridge, 1' north of the north curb face. NGS PID EW9453.

Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. Vertical control based on NAVD-88. Pt 0153 N1845410.50, E6509860.21, Pt 0617 N1858044.3, E6491094.23.

Units are in U.S. survey feet.

NOTES:

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FILE => \$REQUEST

- 5) All bedding and other structural angles are measured from horizontal.
- 6) Plan view shown on Boring Location Plan sheet 1 of 2.

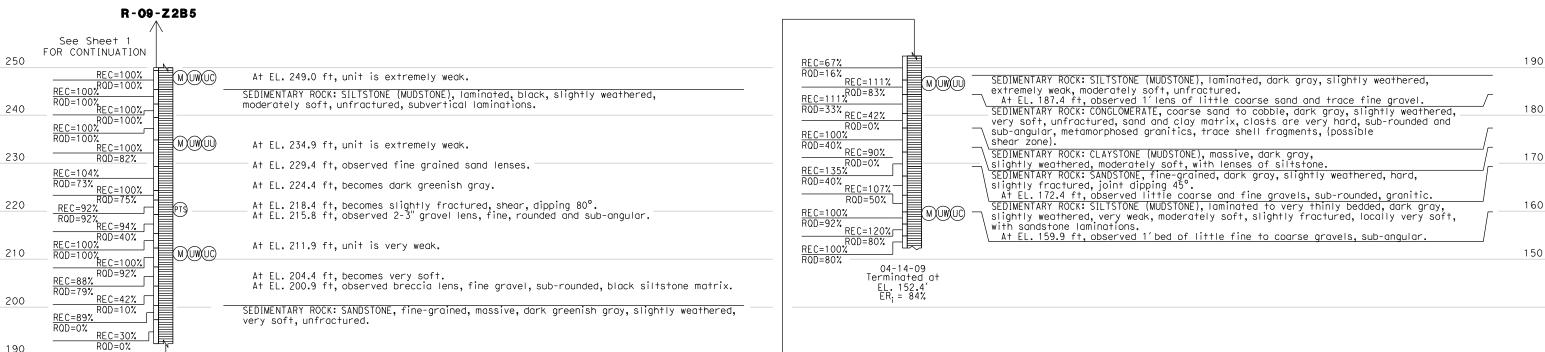
DIST COUNTY ROUTE POST MILES SHEET TOTAL PROJECT NO SHEETS

7 LA 710 N/A

CERTIFIED ENGINEERING GEOLOGIST DATE

PLANS APPROVAL DATE

The Stote of California or its officers or agents shall not be responsible for the occuracy or completeness of electronic copies of this plan sheet.



PROFILE
Vert: 1"-10'

ENGINEERING SERVICES		GEOTECHNICAL SERVICES	STATE OF DIVISION OF ENGINEERING SERVICES		BRIDGE NO.	SR-710	TUNNEL TECHNICAL	STUDY
FUNCTIONAL SUPERVISOR	DRAWN BY: E. MIGUEL	FIELD INVESTIGATION BY: K. LAI, M. ISLAM,	CALIFORNIA	STRUCTURE DESIGN	N/A POST MILES			
NAME: SHIVA KARIMI	CHECKED BY: M. SALISBURY	DATE: 4/06/09 - 4/14/09	DEPARTMENT OF TRANSPORTATION	DESIGN BRANCH	N/A	LOG OF	TEST BORING SHEET	2 OF 2
OGS CIVIL LOG OF TEST BORINGS SHEET		ORIGINAL SCALE IN INCHES		CU	DISREGARD PRI	NTS BEARING	REVISION DATES	SHEET OF

DEREK HIGA

SIGN OFF DATE

DESIGN OVERSIGHT ENGINEER

OGS CIVIL LOG OF TEST BORINGS SHEET

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DRAWN BY:

EDELYNE MIGUEL

DAN JANKLY

M. TORSIELLO

FIELD INVESTIGATION BY

DATE: 3/12/09 - 3/24/09

ORIGINAL SCALE IN INCHES

Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. Vertical control based on NAVD-88. Pt 0153 N1845410.50, E6509860.21, Pt 0617 N1858044.3, E6491094.23.

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DIST COUNTY ROUTE POST MILES SHEET TOTAL NO N/A

TOTAL PROJECT NO SHEETS

7 LA 710 N/A

3/18/10 PROJECT NO SHEETS

7 LA 710 N/A

3/18/10 PROJECT NO SHEETS

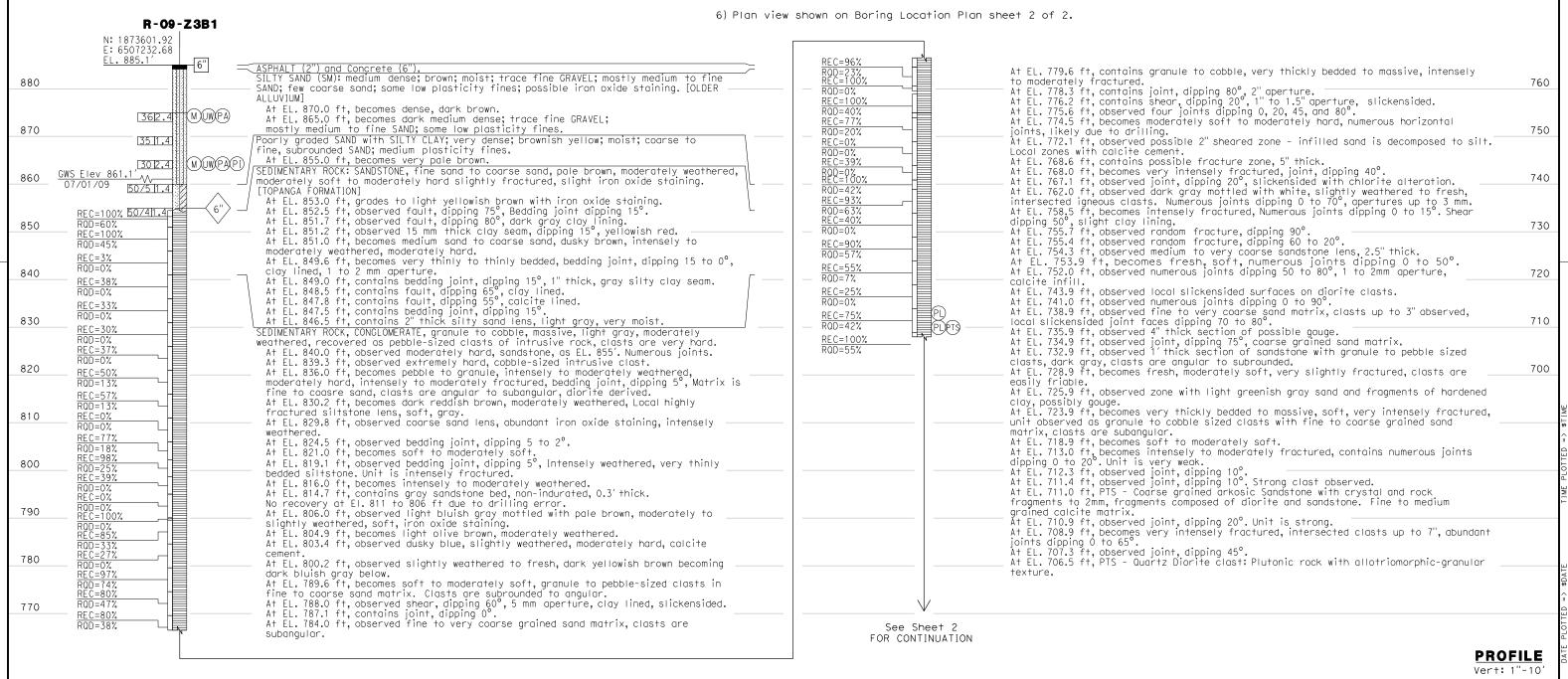
3/18/10 PROFESS 10 N/A

PLANS APPROVAL DATE

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CH2M HILL 6 HUTTON CENTRE DRIVE, SUITE 700

SANTA ANA, CA 92707



SR-710 TUNNEL TECHNICAL STUDY
LOG OF TEST BORING SHEET 1 OF 2

2 3 EA 07-187900 | FILE => \$REQUEST

MAHESWARAN RAVEENDRA PROJECT ENGINEER N/A

POST MILES

DISREGARD PRINTS BEARING EARLIER REVISION DATES

PREPARED FOR THE

STATE OF

CALIFORNIA

DEPARTMENT OF TRANSPORTATION

DEREK HIGA DESIGN OVERSIGHT ENGINEER

OGS CIVIL LOG OF TEST BORINGS SHEET

SIGN OFF DATE

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COUNTY POST MILES IST LΑ 710 GEOTECHNICAL ENGINEER DATE MAHESWARAN RAVEENDRA No. GE 2743 Exp. 06-30-11 PLANS APPROVAL DATE The State of California or its officers or agents & OTECHNICAL shall not be responsible for the accuracy or completeness of electronic copies of this plan sheet. CH2M HILL 6 HUTTON CENTRE DRIVE, SUITE 700 SANTA ANA, CA 92707

PROFILE Vert: 1"-10'



M. TORSIELLO

FIELD INVESTIGATION BY

DATE: 3/12/09 - 3/24/09

ORIGINAL SCALE IN INCHES

DRAWN BY:

CHECKED BY:

EDELYNE MIGUEL

DAN JANKLY

_	REC=28% RQD=7% REC=88% ROD=26%  At EL. 588.9 ft, observed pebble to cobble sized clasts. At EL. 588.4 ft, observed numerous joints dipping 0 to 80°	590
	03-24-09 Terminated at EL. 585.1' ER; = 75%	580
_		
		I NE
		TIME PLOTTED => \$TIME
		TIME PL
		<u> </u>
		PLOTTED => \$DATE
		PL01

MAHESWARAN RAVEENDRA
PROJECT ENGINEER

N/A
POST MILES
N/A
LOG OF TEST BORING SHEET 2 OF 2

CU
EA 07-187900

PARTIES BEARING
EARLIER REVISION DATES
PROJECT ENGINEER

OF TEST BORING SHEET 2 OF 2

FILE => \$REQUEST

PREPARED FOR THE

STATE OF

CALIFORNIA

DEPARTMENT OF TRANSPORTATION

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DIST COUNTY ROUTE POST MILES SHEET TOTAL PROJECT NO SHEETS

7 LA 710 N/A

CERTIFIED ENGINEERING GEOLOGIST DATE

PLANS APPROVAL DATE

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TOTAL PROJECT NO SHEET TOTAL SHEETS

3/18/10

DATE

SECULOSIONAL CEO.

PRATT

NO. CEG 2141

EXP. 5/31/2011

EXP. 5/31/2011

EXP. 5/31/2011

EXP. 5/31/2011

EXP. 5/31/2011

EXP. 5/31/2011

CERTIFIED

ROUTE

TOTAL PROJECT

NO SHEETS

TOTAL PROJECT

TOTAL PROJECT

TOTAL PROJECT

TOTAL PROJECT

NO SHEETS

TOTAL PROJECT

NO SHEETS

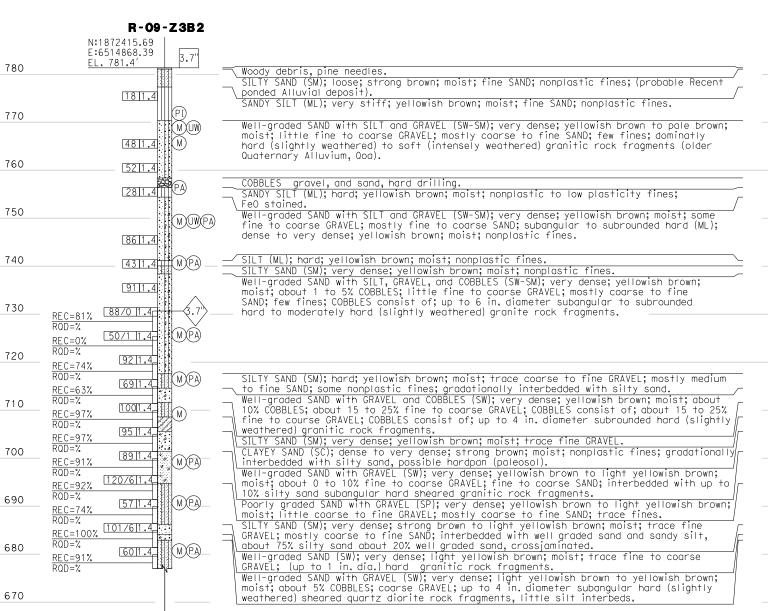
TOTAL PROJECT

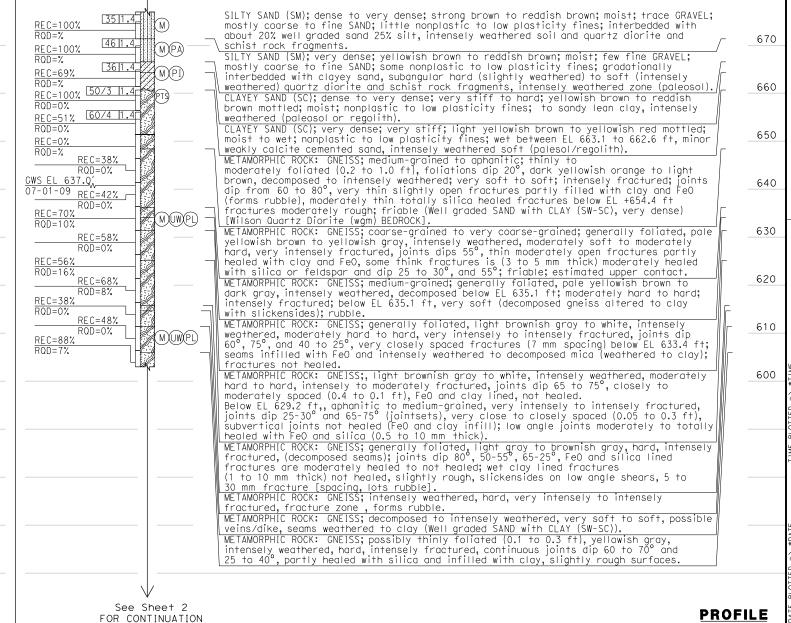
TOTAL PROJECT

NO SHEETS

TOTAL PROJECT

T





**ENGINEERING SERVICES GEOTECHNICAL SERVICES** DIVISION OF ENGINEERING SERVICES SR-710 TUNNEL TECHNICAL STUDY STATE OF N/A STRUCTURE DESIGN UNCTIONAL SUPERVISOR FIELD INVESTIGATION BY: J. PRATT, T. HALDA DRAWN BY: F. MIGUEL **CALIFORNIA** POST MILES LOG OF TEST BORING SHEET 1 OF 3 NAME: SHIVA KARIMI DATE: 1/13/09 - 1/22/09 DEPARTMENT OF TRANSPORTATION **DESIGN BRANCH** CHECKED BY: K. BARKER ORIGINAL SCALE IN INCHES FOR REDUCED PLANS DISREGARD PRINTS BEARING EARLIER REVISION DATES OGS CIVIL LOG OF TEST BORINGS SHEET EA 07-187900

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slighly rough surfaces, 35% rubble.

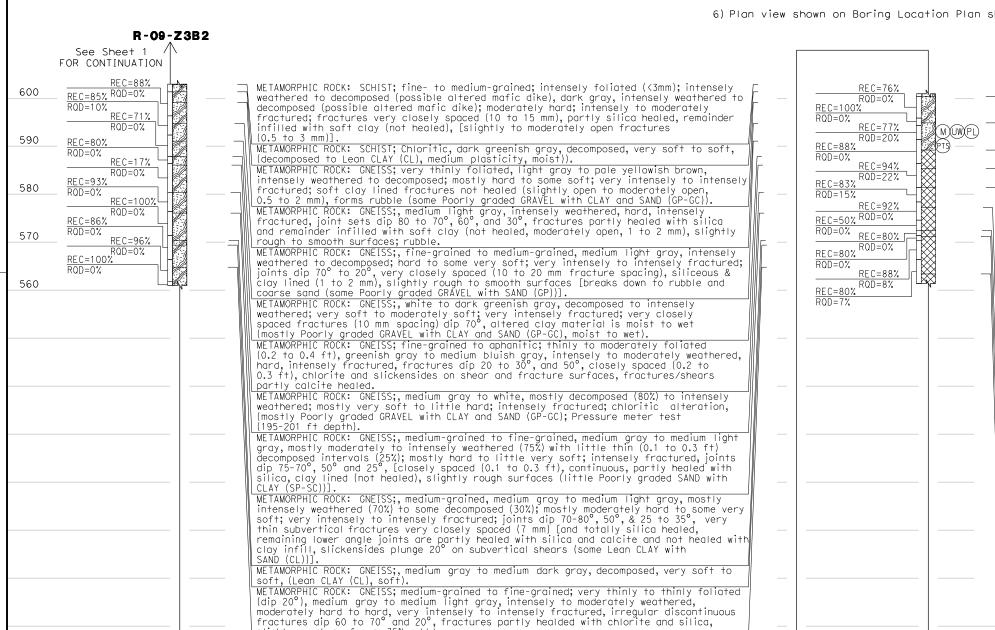
Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. Vertical control based on NAVD-88. P+ 0153 N1845410.50, E6509860.21, Pt 0617 N1858044.3, E6491094.23.

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- 6) Plan view shown on Boring Location Plan sheet 2 of 2.

COUNTY LΑ 710 D. Fratt CERTIFIED ENGINEERING GEOLOGIST 3/18/10
DATE // JOSEPH S. No. CEG 2141 Exp. 5/31/2011 PLANS APPROVAL DATE CERTIFIED ENGINEERING GEOLOGIST he State of California or its officers or agents hall not be responsible for the accuracy or completeness of electronic copies of this plan sheet

**PROFILE** 



560 METAMORPHIC ROCK: GNEISS;, medium-grained, medium light gray to medium gray, mostly intensely to moderately weathered (80%) to little decomposed (20%); mostly moderately hard to little very soft; intensely fractured; joints dip 75°, 50°, and 25°, subvertical joints are very closely spaced (30 mm), low angle joints [are closely spaced (60 to 70 mm), continuous not healed fractures are lined with minor chlorite and rare calcite, moderately to slightly rough surfaces (little Lean CLAY (CL))].

METAMORPHIC ROCK: GNEISS; medium-grained; foliated, medium gray, mostly moderately to intensely weathered (5% decomposed); mostly moderately hard to few soft; very intensely to intensely fractured; joints dip 70° and 20°, closely to very closely spaced 550 (<0.1 to 0.2 ft) [subvertical very thin (0.5 mm thick) fractures moderately healed with silica, moderately rough surfaces].

Below EL 559.2 ft becomes, dark gray, (Pressuremeter test from 221.0 to 227.0 ft depth). 540 METAMORPHIC ROCK: GNEISS; medium-grained; foliated, medium light gray to medium gray, moderately to intensely weathered; moderately hard; very intensely to intensely fractured; joints dip 70 to 80° & 20-25°, continuous subvertical joints partly healed with silica and not healed with clay infill [slightly rough surfaces, 80% soft decomposed 530 IGNEOUS ROCK QUARTZ DIORITE; medium-grained, medium light gray, moderately to slightly weathered, hard to very hard, intensely fractured, low angle moderately open joints lined with waxey chlorite, slickensides show 65° rake from horizontal in 50° dip shear planes; continuous joints dip 75°, 50°, and 20°, closely (40 mm) to very closely (10 mm) spaced, [moderate and low angle joints closely spaced, subvertical joints very closely spaced and moderately healed with very thin silica (0.5 to 1 mm thick)]. 520 METAMORPHIC ROCK MARBLE (partly replaced with QUARTZ DIORITE in patches); medium to coarse-grained, light gray to medium light gray, moderately to intensely weathered; moderately hard; intensely fractured, low angle moderately open joints lined with waxey chlorite, high and low angle fractures moderately healed with calcite [fractures very closely spaced (20 to 25 mm), possible crinoidal debris in recrystalized mass].

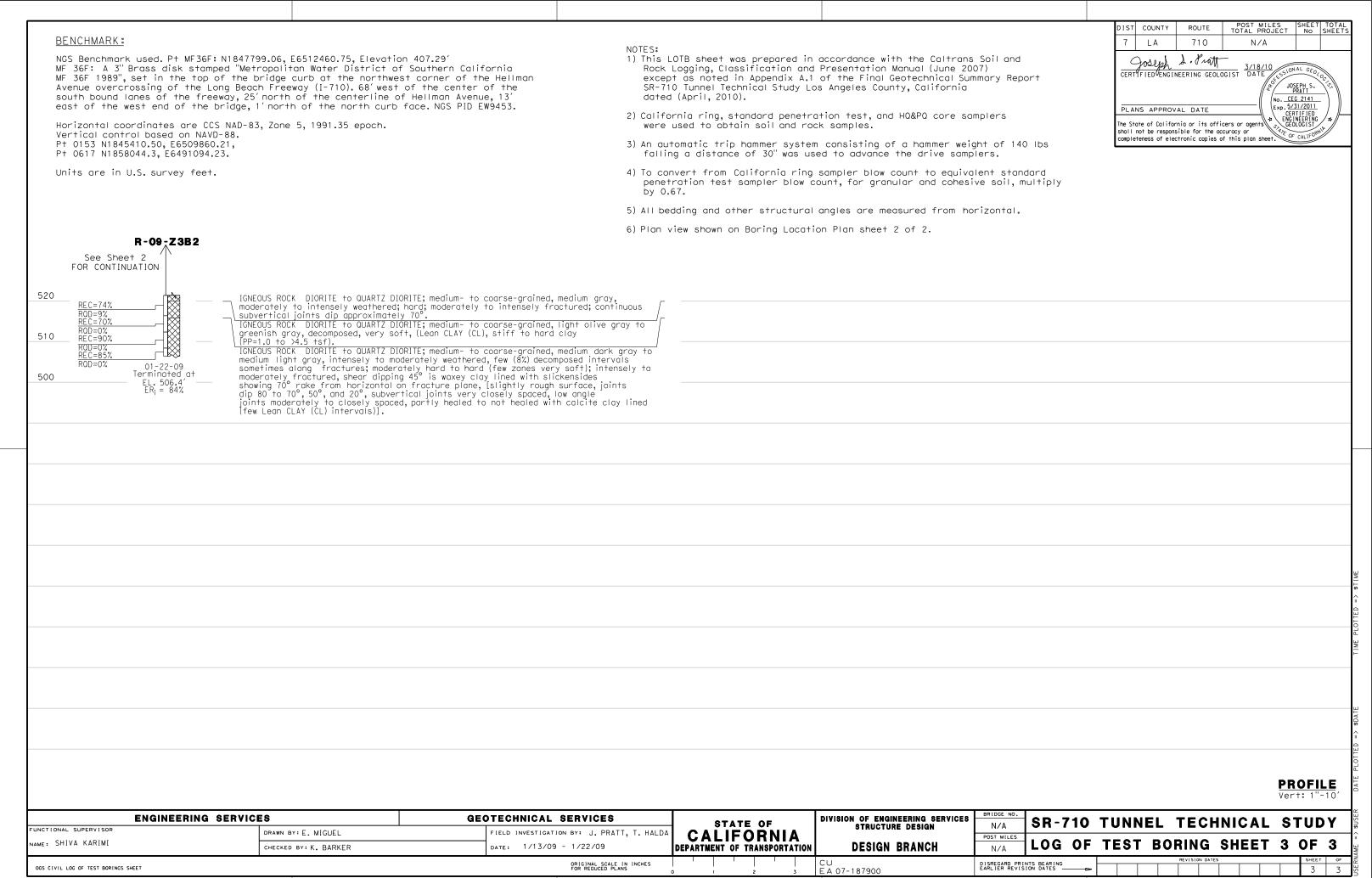
At EL. 542.4 ft, becomes dark greenish gray, intensely fractured, joints dip 70°,
50 to 45°, and 20°, joints partly healed with calcite and chlorite infill to totally
healed with calcite, 10% very soft decomposed interval altered to clay METAMORPHIC ROCK: MARBLE (partly replaced with QUARTZ DIORITE in patches); medium-grained, dark greenish gray to light brownish gray, moderately to intensely weathered; moderately soft; intensely fractured, joints dip 80°, 50°, and 20°, partly to totally healed with calcite, moderately thin (1 to 2 mm) subvertical calcite veins very closely spaced (15 mm). METAMORPHIC ROCK: MARBLE (partly replaced with QUARTZ DIORITE in mixed patches and layers); medium- to coarse-grained, medium light gray to greenish gray, intensely to moderately weathered, moderately soft to moderately hard, intensely fractured, joints dip 75 to 60°, 50°, and 20°, joints partly to totally calcite healed (1 to 2 mm calcite veins), all continuous, some clay lined not healed open joints. METAMORPHIC ROCK: MARBLE;, medium-grained, greenish gray, intensely to moderately weathered, moderately hard, very intensely to intensely fractured, joints dip 20 to 30° and 60°, subvertical joints very closely spaced, low angle joints closely spaced, partly to totally calcife healed. GNEOUS ROCK DIORITE to QUARTZ DIORITE; medium- to coarse-grained, medium gray, intensely to moderately weathered, hard to very hard, intensely fractured, continuous joints dip 85 to 70° and 30°, very closely spaced subvertical joints partly healed with calcite and some infilled with clay and slightly open, low angle joints partly healed to totally healed with calcite (1 to 2mm thick).

**ENGINEERING SERVICES GEOTECHNICAL SERVICES** DIVISION OF ENGINEERING SERVICES SR-710 TUNNEL TECHNICAL STUDY STATE OF N/A STRUCTURE DESIGN UNCTIONAL SUPERVISOR FIELD INVESTIGATION BY: J. PRATT, T. HALDA DRAWN BY: F. MIGUEL CALIFORNIA POST MILES LOG OF TEST BORING SHEET 2 OF 3 NAME: SHIVA KARIMI DATE: 1/13/09 - 1/22/09 DEPARTMENT OF TRANSPORTATION **DESIGN BRANCH** CHECKED BY: K. BARKER ORIGINAL SCALE IN INCHES FOR REDUCED PLANS DISREGARD PRINTS BEARING EARLIER REVISION DATES OGS CIVIL LOG OF TEST BORINGS SHEET EA 07-187900

FILE => \$REQUEST

See Sheet 3

FOR CONTINUATION



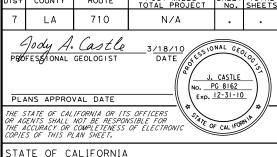
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Units are in U.S survey feet.

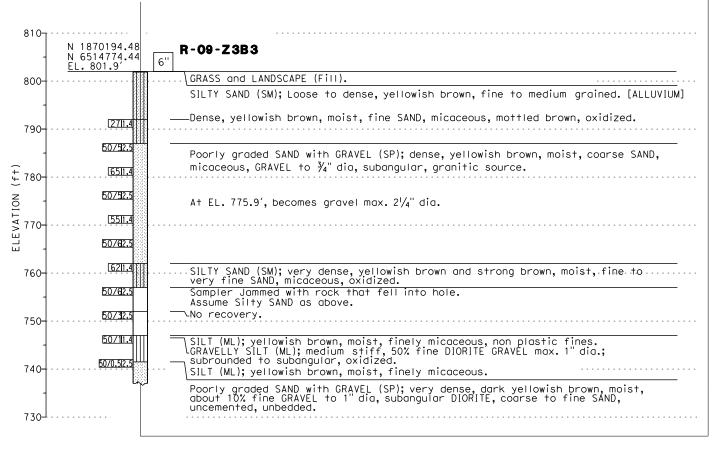
#### NOTES:

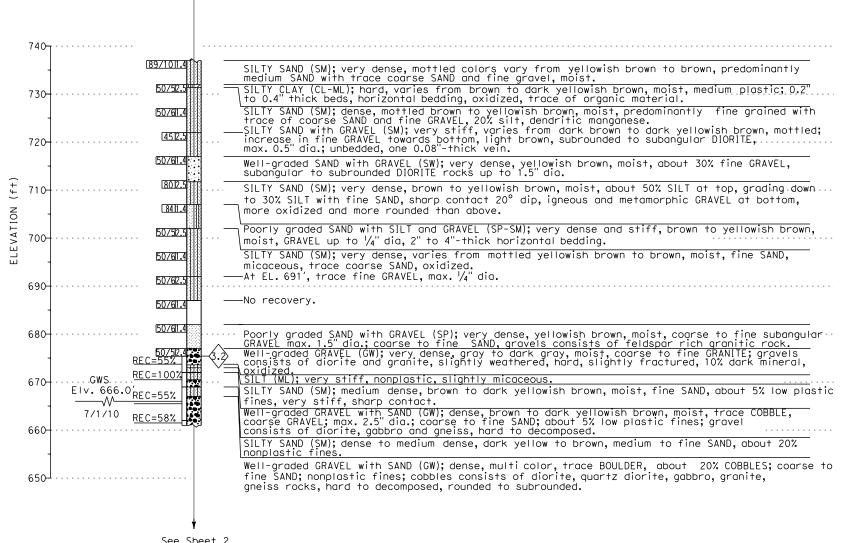
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STATE OF CALIFORNIA DEPARTMENT OF TRANSPORTATION, DISTRICT-7 100 S. MAIN STREET, LOS ANGELES, CA 90012

EARTH MECHANICS, INC. 17660 NEWHOPE STREET, SUITE E FOUNTAIN VALLEY, CA 92708





PROFILE

VERTICAL 1" = 10'

DEREK HIGA DESIGN OVERSIGHT ENGINEER	DRAWN BY:	K. THANT	B. SCHELL, J. CASTLE  FIELD INVESTIGATION BY:	PREPARED FOR THE STATE OF	H. LAW	BRIDGE NO.	SR-710	TUNNEL	TECHNICAL	. STUDY	
SIGN OFF DATE	CHECKED BY:	B. SCHELL	2/4/09 - 2/13/09 DATE:	CALIFORNIA DEPARTMENT OF TRANSPORTATION	PROJECT ENGINEER	POST MILE N/A	LOG O	F TEST B	ORING SHEE	Г 1 OF 2	ļ
OGS CIVIL LOG OF TEST BORINGS SHEET			ORIGINAL SCALE IN INCHES FOR REDUCED PLANS	0 1 2 3	CU . EA 07-187900	DISREGARD PR EARLIER REVIS	INTS BEARING BION DATES	REVISION DATES	(PRELIMINARY STAGE ONLY)	SHEET 0	- Nanoi

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FOR CONTINUATION

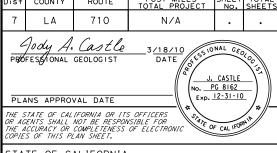
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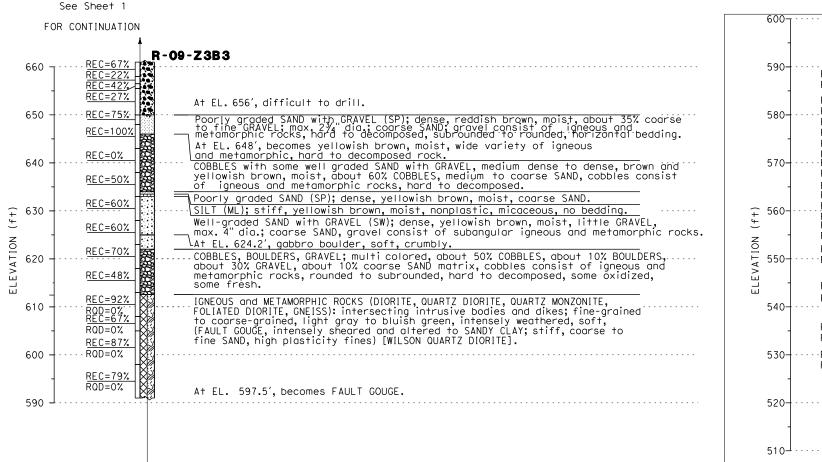
#### NOTES:

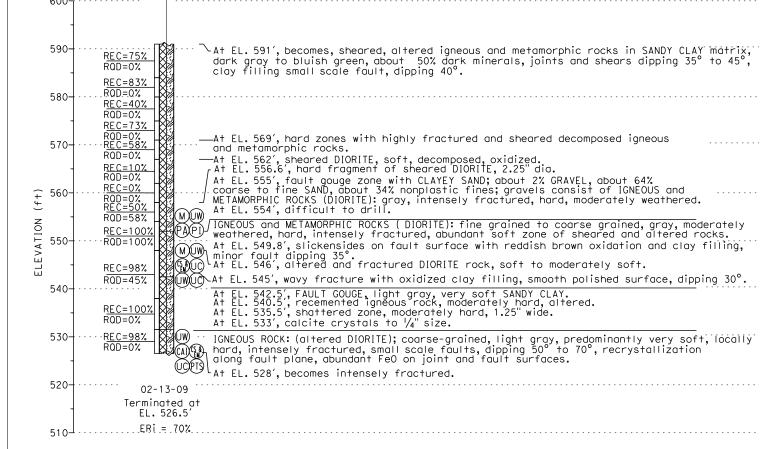
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EARTH MECHANICS, INC. 17660 NEWHOPE STREET, SUITE E FOUNTAIN VALLEY, CA 92708





PROFILE
VERTICAL 1" = 10'

DEREK HIGA DESIGN OVERSIGHT ENGINEER	DRAWN BY:	K. THANT	B. SCHELL, J. CASTLE FIELD INVESTIGATION BY:	PREPARED FOR THE STATE OF	H. LAW	BRIDGE NO.	SR-710	TUNNEL TECHNICAL S	STUDY
SIGN OFF DATE	CHECKED BY:	B. SCHELL	2/4/09 - 2/13/09 DATE:	CALIFORNIA DEPARTMENT OF TRANSPORTATION	PROJECT ENGINEER	POST MILE	LOG OF	TEST BORING SHEET 2	2 OF 2
OGS CIVIL LOG OF TEST BORINGS SHEET			ORIGINAL SCALE IN INCHES FOR REDUCED PLANS	0 1 2 3	CU . EA 07-187900	DISREGARD PE EARLIER REVI	RINTS BEARING SION DATES	REVISION DATES (PRELIMINARY STAGE ONLY)	SHEET OF 2

DEREK HIGA

SIGN OFF DATE

DESIGN OVERSIGHT ENGINEER

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DRAWN BY:

CHECKED BY:

EDELYNE MIGUEL

DAN JANKLY

D. JANKIY

FIELD INVESTIGATION BY

DATE: 3/02/09 - 3/06/09

ORIGINAL SCALE IN INCHES

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- 6) Plan view shown on Boring Location Plan sheet 2 of 2.

IST COUNTY LΑ 710 3/18/10 GEOTECHNICAL ENGINEER DATE MAHESWARAN RAVEENDRA No. GE 2743 Exp. 06-30-11 PLANS APPROVAL DATE The State of California or its officers or agents QOTECHNICA . hall not be responsible for the accuracy or completeness of electronic copies of this plan sheet. CH2M HILL 6 HUTTON CENTRE DRIVE, SUITE 700 SANTA ANA, CA 92707

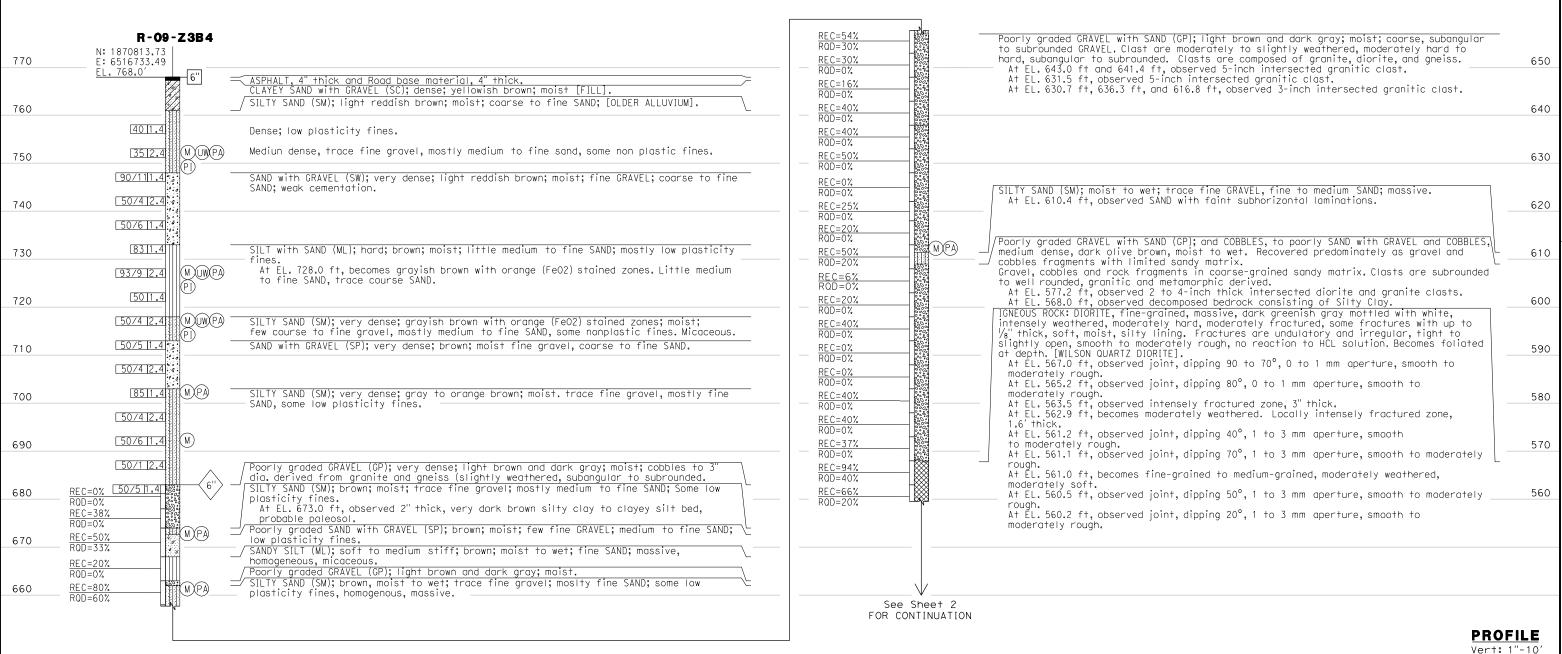
SR-710 TUNNEL TECHNICAL STUDY

LOG OF TEST BORING SHEET 1 OF 2

N/A

POST MILES

DISREGARD PRINTS BEARING EARLIER REVISION DATES



PREPARED FOR THE

MAHESWARAN RAVEENDRA PROJECT ENGINEER

EA 07-187900

STATE OF

CALIFORNIA

DEPARTMENT OF TRANSPORTATION

NGS Benchmark used. Pt MF36F: N1847799.06, E6512460.75, Elevation 407.29' MF 36F: A 3" Brass disk stamped "Metropolitan Water District of Southern California MF 36F 1989", set in the top of the bridge curb at the northwest corner of the Hellman Avenue overcrossing of the Long Beach Freeway (I-710). 68' west of the center of the south bound lanes of the freeway, 25' north of the centerline of Hellman Avenue, 13' east of the west end of the bridge, 1' north of the north curb face. NGS PID EW9453.

clay lining.

Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. Vertical control based on NAVD-88. Pt 0153 N1845410.50, E6509860.21, Pt 0617 N1858044.3, E6491094.23.

Units are in U.S. survey feet.

NOTES:

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- 3) An automatic trip hammer system consisting of a hammer weight of 140 lbs falling a distance of 30" was used to advance the drive samplers.
- 4) To convert from California ring sampler blow count to equivalent standard penetration test sampler blow count, for granular and cohesive soil, multiply by 0.67.
- 5) All bedding and other structural angles are measured from horizontal.

DIST COUNTY ROUTE TOTAL PROJECT SHEET TOTAL NO SHEETS

7 LA 710 N/A

GEOTECHNICAL ENGINEER

DATE

PLANS APPROVAL DATE

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CH2M HILL
6 HUTTON CENTRE DRIVE, SUITE 700
SANTA ANA, CA 92707

	6) Plan view shown o	on Boring Location Plan sheet 2 of 2.
R-09-Z3B	84	
See Sheet 1 FOR CONTINUATION		
560		
REC=60% ROD=20%	At EL. 557.5 ft, observed shear, dipping 50 to 5°, undulatory, irregular shear, dusky red, paperthin clay lining, striated along strike, parallels faint foliations 4" above and 4" below.  At EL. 556.9 ft, observed shear, dipping 40°, tight, moderately rough, 1/8" thick clay lining.  At EL. 556.3 ft, observed shear, dipping 45°, tight, moderately rough, 1/8" thick clay lining, intensely fractured below shear.  At EL. 556.2 ft, becomes intensely weathered, soft, intensely fractured.	At EL. 523.5 ft, observed shear, dipping 40 to 20°, three shears and numerous fractures from EL. 523.8 ft to 523.0 ft, with yellowish red and dark red lining.  At EL. 522.0 ft, observed shear, dipping 50 to 40°, three shears from EL. 522.2 ft EL. 521.5 ft, clay lined.
50 REC=86% ROD=52% REC=100%	At EL. 556.9 ft, observed shear, dipping 40°, tight, moderately rough, 1/8" thick clay lining.  At EL. 556.3 ft, observed shear, dipping 45°, tight, moderately rough, 1/8"	At EL. 521.5 ft, becomes soft to moderately soft from EL. 521.5 ft to 520.5 ft.  At EL. 520.5 ft, observed shear, dipping 65°, 2mm aperture, clay lined, striated downdip.
ROD=60% REC=73% REC=73% REC=73%	M UWEM  At EL. 556.3 ft, observed shear, dipping 45°, tight, moderately rough, 1/8"  thick clay lining, intensely fractured below shear.  At EL. 556.2 ft, becomes intensely weathered, soft, intensely fractured.  At EL. 552.2 ft, observed shear/fault zone, dipping 30°, 3" thick, highly sheared zone with dusky red clay lining along sub parallel shears, polished and striated, medium	At EL. 520.0 ft, becomes tine-grained, dark greenish gray to light greenish gray, slightly weathered, hard, intensely to moderately fractured.  At EL. 519.5 ft, observed joint, dipping 30°. Unit is weak.
REC=100% RQD=30%	with dusky red clay lining along sub parallel shears, polished and striated, medium plasticity.  At EL. 551.9 ft, becomes coarse-grained, moderately weathered, moderately soft,	EL. 521.5 ft, clay lined.  At EL. 521.5 ft, becomes soft to moderately soft from EL. 521.5 ft to 520.5 ft.  At EL. 520.5 ft, observed shear, dipping 65°, 2mm aperture, clay lined, striated downdip.  At EL. 520.0 ft, becomes fine-grained, dark greenish gray to light greenish gray,  slightly weathered, hard, intensely to moderately fractured.  At EL. 519.5 ft, observed joint, dipping 30°. Unit is weak.  At EL. 518.8 ft, observed joint, dipping 30 to 20°.  At EL. 518.1 ft, observed shear, dipping 60 to 40°, 2mm aperture, dusky red clay lining.  At EL. 516.8 ft, observed shear, dipping 30°, 2mm aperture, clay lined, moderately rough.  At EL. 508.5 ft, observed foliation joint, dipping 40°, faint.  At EL. 507.8 ft, observed numerous carbonate lined joint faces, scattered dusky red
7530 ROD=33% REC=100% REC=100% ROD=0%	intensely fractured.  At EL. 551.0 ft, becomes intensely to moderately fractured, joint, dipping 60 to 50°,  3 mm aperture, moderately rough.  At EL. 549.5 ft, observed faint clay liped discontinuity, possible spear, irregular and	At EL. 507.8 ft, observed numerous carbonate lined joint faces, scattered dusky red  lined joints and shears.  At EL. 505.7 ft, observed shear/fault zone, dipping 45°, roughly 1' thick gouge zone with
REC=100% H₩	at EL. 331. Tr, becomes midderdrefy fractured, joint, dipping 60 to 30 t	Ined joints and shears.  At EL. 505.7 ft, observed shear/fault zone, dipping 45°, roughly 1' thick gouge zone with highly polished fault plane at EL. 505.0 ft, striated down dip. Gouge consists of light greenish gray bentonitic clay with abundant diorite gravel to 4" dia. Gouge is moist, stiff, highly plastic.
ROD=0% REC=83% ROD=33%	very weak.  At EL. 546.0 ft, observed shear, dipping 60°, polished and striated.  At EL. 545.5 ft, observed shear, dipping 90 to 45°, tight, ½" to ½" thick dusky red clay lining, undulatory.	At EL.502.5 ft, observed faint, undulatory, subhorizontal foliations.  At EL.502.0 ft, observed intensely fractured, shear, dipping 60°, 2mm aperture, clay
520 ROD=0% REC=100% ROD=0% REC=83% ROD=33% S10 REC=75% ROD=0% REC=83% ROD=0% REC=00% ROD=20% REC=100% ROD=20% REC=88% ROD=25% ROD=25% ROD=0% REC=100% ROD=76%  03-06-09	At EL. 545.0 ft, becomes intensely weathered, soft.  At EL. 544.0 ft, observed shear, dipping 90 to 60°,/8" thick dusky red clay lining, undulatory.	lined, striated downdip.  At EL. 501.0 ft, becomes very intensely fractured, remnant dusky red lined shears.  At EL. 499.2 ft, observed shear, dipping 50°, dusky red clay and carbonate infill.  At FL. 497.5 ft, becomes moderately fractured, joint, dipping 50 to 20°, numerous joints
500 ROD=25% ROD=25% REC=88% RQD=0%	At EL. 541.5 ft, becomes moderately weathered, moderately hard. At EL. 540.2 ft and 539.6 ft, observed joint, dipping 60°, 2mm aperture, moderately rough. At EL. 535.7 ft, observed fault, dipping 50°, paperthin clay lining, 1-2mm thick, smooth. M.UWWUC) Below fault, unit is very dark gray, slightly weathered, hard, moderately to intensely	lined, striated downdip.  At EL. 501.0 ft, becomes very intensely fractured, remnant dusky red lined shears.  At EL. 499.2 ft, observed shear, dipping 50°, dusky red clay and carbonate infill.  At EL. 497.5 ft, becomes moderately fractured, joint, dipping 50 to 20°, numerous joints to El. 496.0 ft bgs, generally 1 mm aperture, slightly to moderately rough. Unit is weak.  At EL. 497.0 ft, observed foliation joint, dipping 40 to 30°, faint.  At EL. 496.0 ft, observed shear, dipping 60°, tight, dusky red, 16" thick clay lined shear, truncates foliations.  At EL. 495.5 ft, observed shear, dipping 30°, tight, dusky red, 16" thick clay lined shear.
ROD=0% REC=100% ROD=76%	Muwuc Below fault, unit is very dark gray, slightly weathered, hard, moderately to intensely fractured.  At EL. 534.0 ft, observed shear, dipping 40°, paperthin dusky red clay lining, striated	shear, truncates foliations. At EL. 495.5 ft, observed shear, dipping 30°, tight, dusky red, ½" thick clay lined shear. Unit is weak. At EL. 495.2 ft. becomes locally very intensely fractured.
03-06-09 Terminated at EL. 492.0' ER:= 70%	at At FL 533 2 ft observed spear dipping 25° paperthin dusky red clay lining stricted	At EL. 495.0 ft, observed foliation joint, dipping 40°, faint.  At EL. 494.0 ft, observed shear, dipping 50°, very tight, dusky red, $\frac{1}{16}$ " thick clay lined shear, truncates foliations.
480		shedr. Unit 1s wedk.  At EL. 495.2 ft, becomes locally very intensely fractured.  At EL. 495.0 ft, observed foliation joint, dipping 40°, faint.  At EL. 494.0 ft, observed shear, dipping 50°, very tight, dusky red, ½6" thick clay lined shear, truncates foliations.  At EL. 493.5 ft, contains foliation joint, dipping 45°, faint foliation.  At EL. 493.0 ft, observed shear, dipping 50°, tight, dusky red, ½6" thick clay lined shear, truncates foliations above, massive below.
470	At EL. 532.7 ft, becomes moderately to slightly weathered, hard, intensely to moderately fractured.  At EL. 532.4 ft, observed fault, dipping 70°, 3/8" reverse offset, tight, striated roughly downdip, offsets 1/8" thick carbonate lined vein.  At EL. 531.6 ft, observed shear, dipping 50 to 45°, two shears with paperthin dusky red clay lining, Unit is medium strong.  At EL. 530.9 ft, observed shear, dipping 70 to 25°, undulatory, dusky red clay lining, striated roughly parallel to strike.  At EL. 530.5 ft, observed shear, dipping 65 to 25°, three shears with paperthin, dusky red clay lining.	
	At EL. 531.6 tt, observed snear, dipping 50 to 45 , two snears with paper filli dusky red clay lining. Unit is medium strong. At EL. 530.9 ft, observed shear, dipping 70 to 25°, undulatory, dusky red clay lining, striated roughly narallel to strike	
460	At EL. 530.5 ft, observed shear, dipping 65 to 25°, three shears with paperthin, dusky red clay lining.  At EL. 529.2 ft, becomes very intensely fractured.  At EL. 528.3 ft, observed shear, dipping 75 to 65°, 3-4mm aperture with dusky red clay	
450	film, slightly to moderately rough.  At FL. 527.3 ft. observed shear dipping 70°.2-3mm aperture striated with paperthin	
	dusky red clay lining. At EL. 526.0 ft, observed shear, dipping 90 to 70°, 2-3mm aperture, paperthin dusky red	

PROFILE Vert: 1"-10'

PREPARED FOR THE STATE OF DEREK HIGA
DESIGN OVERSIGHT ENGINEER D. JANKLY SR-710 TUNNEL TECHNICAL STUDY DRAWN BY: EDELYNE MIGUEL N/A MAHESWARAN RAVEENDRA PROJECT ENGINEER FIELD INVESTIGATION BY CALIFORNIA POST MILES CHECKED BY: DAN JANKLY DATE: 3/02/09 - 3/06/09 LOG OF TEST BORING SHEET 2 OF 2 DEPARTMENT OF TRANSPORTATION N/A SIGN OFF DATE ORIGINAL SCALE IN INCHES FOR REDUCED PLANS CU EA 07-187900 DISREGARD PRINTS BEARING EARLIER REVISION DATES OGS CIVIL LOG OF TEST BORINGS SHEET

clay lining. At EL. 525.2 ft, observed shear, dipping 90 to 80°, 4mm aperture,  $\frac{1}{8}$ " thick dusky red

UNCTIONAL SUPERVISOR

AME: SHIVA KARIMI

OGS CIVIL LOG OF TEST BORINGS SHEET

NGS Benchmark used. Pt MF36F: N1847799.06, E6512460.75, Elevation 407.29' MF 36F: A 3" Brass disk stamped "Metropolitan Water District of Southern California MF 36F 1989", set in the top of the bridge curb at the northwest corner of the Hellman Avenue overcrossing of the Long Beach Freeway (I-710). 68' west of the center of the south bound lanes of the freeway, 25' north of the centerline of Hellman Avenue. 13' east of the west end of the bridge, 1'north of the north curb face. NGS PID EW9453.

DRAWN BY: E. MIGUEL

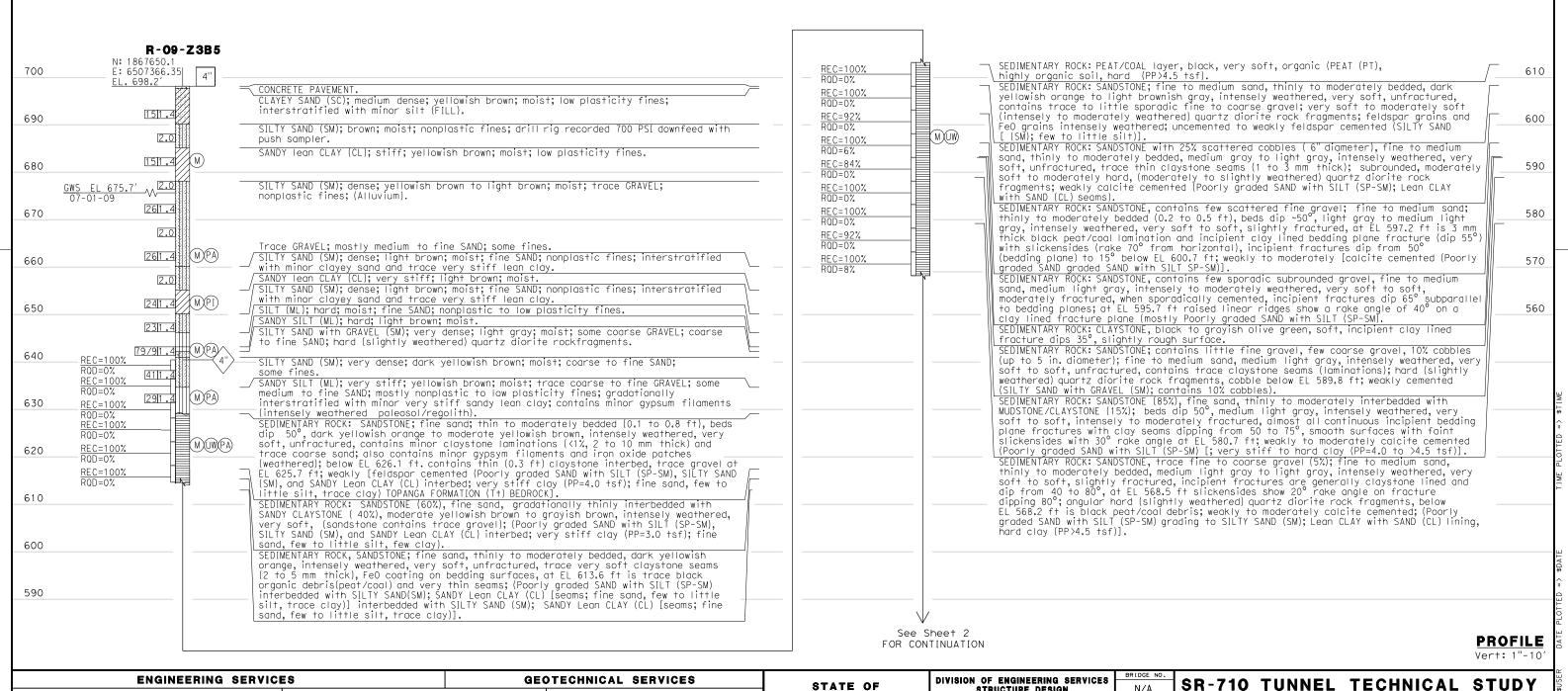
CHECKED BY: K. BARKER

Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. Vertical control based on NAVD-88. P+ 0153 N1845410.50, E6509860.21, Pt 0617 N1858044.3, E6491094.23.

Units are in U.S. survey feet.

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- 6) Plan view shown on Boring Location Plan sheet 2 of 2.

COUNTY LΑ 710 D. Pratt CERTIFIED ENGINEERING GEOLOGIST DATE // JOSEPH S. No. CEG 2141 Exp. 5/31/2011 PLANS APPROVAL DATE CERTIFIED ENGINEERING GEOLOGIST the State of California or its officers or agents hall not be responsible for the accuracy or completeness of electronic copies of this plan sheet



STATE OF

**CALIFORNIA** 

DEPARTMENT OF TRANSPORTATION

FIELD INVESTIGATION BY: K. LAI/J. PRATT

ORIGINAL SCALE IN INCHES FOR REDUCED PLANS

DATE: 4/15/09 - 5/1/09

N/A

POST MILES

DISREGARD PRINTS BEARING EARLIER REVISION DATES

LOG OF TEST BORING SHEET 1 OF 5

STRUCTURE DESIGN

**DESIGN BRANCH** 

EA 07-187900 FILE => \$REQUEST

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DIST COUNTY ROUTE POST MILES SHEET TOTAL SHEETS

7 LA 710 N/A

CERTIFIED ENGINEERING GEOLOGIST

PLANS APPROVAL DATE

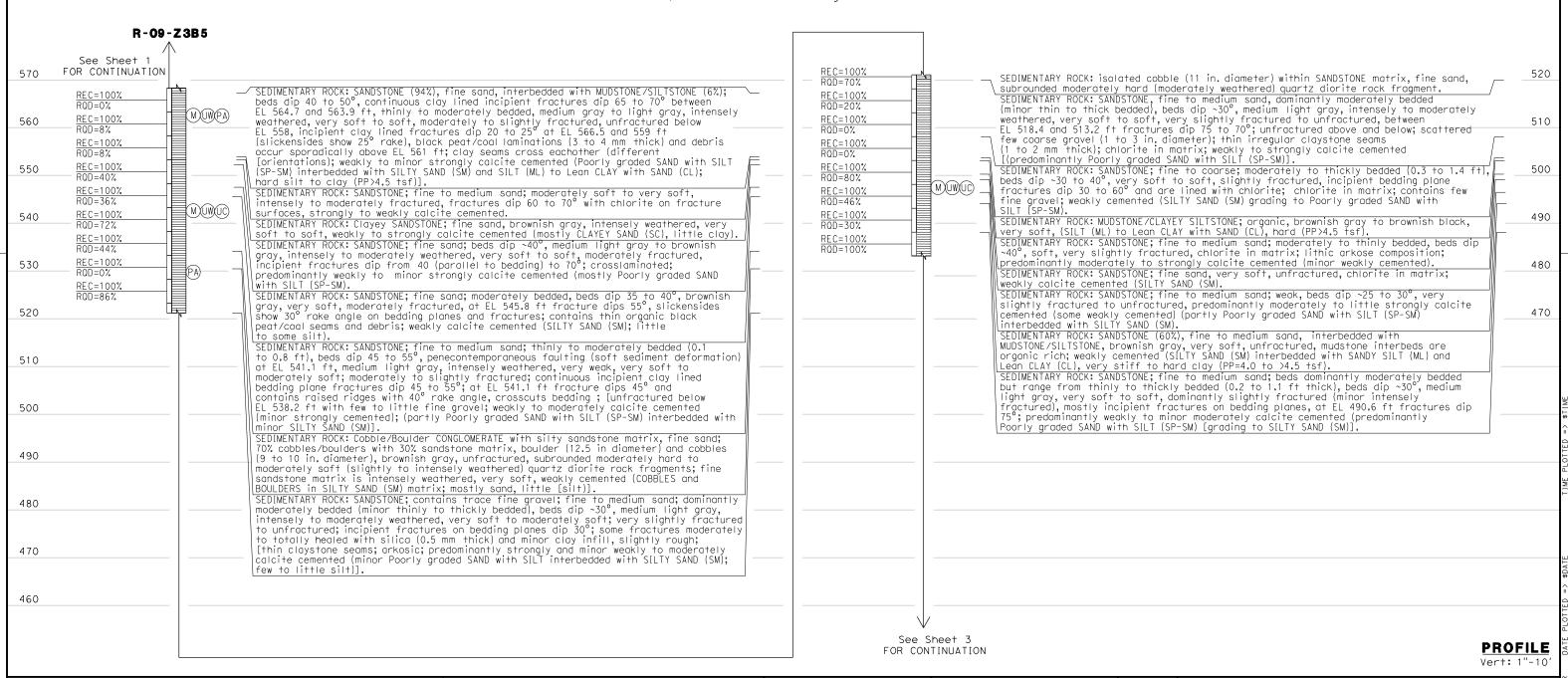
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SHEET TOTAL NO SHEETS

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DATE

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**ENGINEERING SERVICES GEOTECHNICAL SERVICES** DIVISION OF ENGINEERING SERVICES SR-710 TUNNEL TECHNICAL STUDY STATE OF N/A STRUCTURE DESIGN UNCTIONAL SUPERVISOR DRAWN BY: F. MIGUEL FIELD INVESTIGATION BY: K. LAI/J.PRATT **CALIFORNIA** POST MILES LOG OF TEST BORING SHEET 2 OF 5 AAME: SHIVA KARIMI DATE: 4/15/09 - 5/1/09 DEPARTMENT OF TRANSPORTATION **DESIGN BRANCH** CHECKED BY: K. BARKER ORIGINAL SCALE IN INCHES FOR REDUCED PLANS DISREGARD PRINTS BEARING EARLIER REVISION DATES OGS CIVIL LOG OF TEST BORINGS SHEET EA 07-187900 FILE => \$REQUEST

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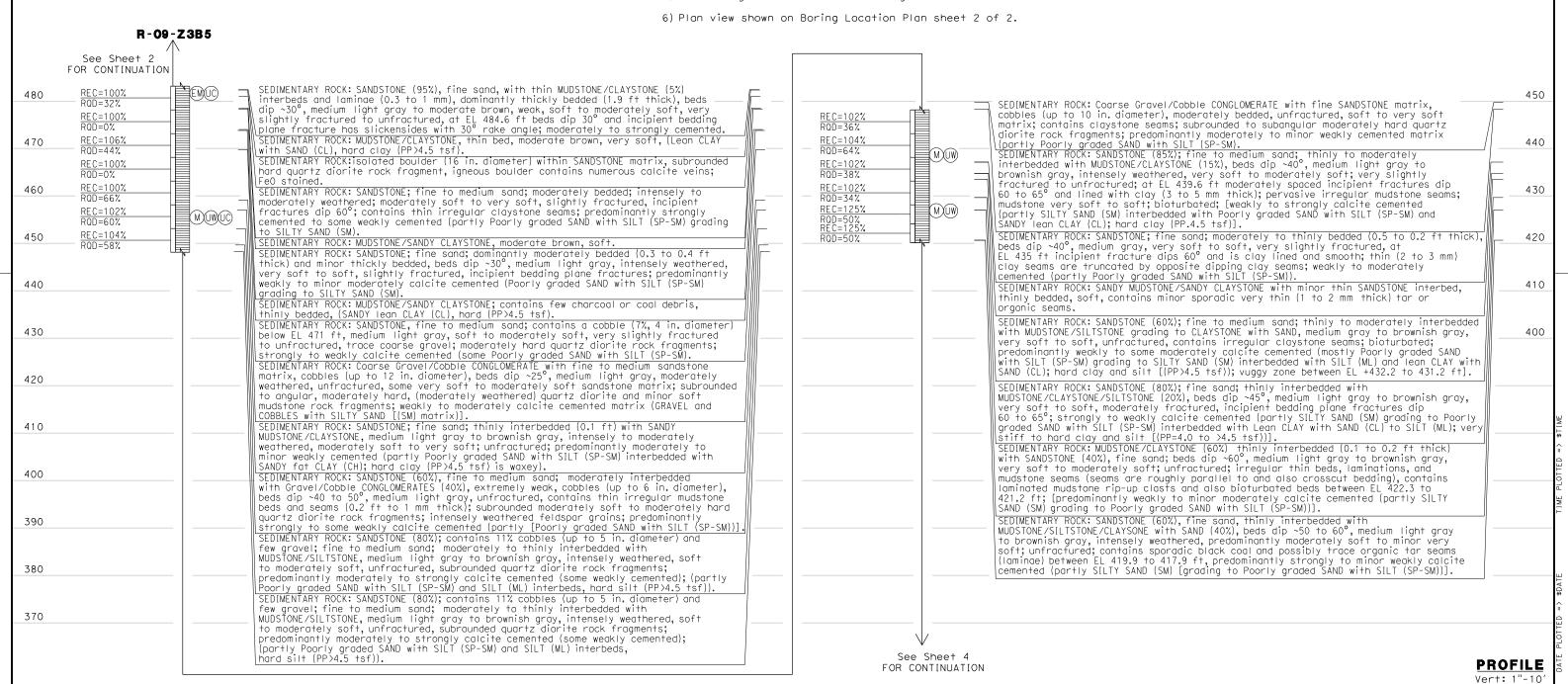
DIST COUNTY ROUTE TOTAL PROJECT NO SHEET TOTAL PROJECT NO SHEETS

7 LA 710 N/A

CERTIFIED ENGINEERING GEOLOGIST 3/18/10

PLANS APPROVAL DATE

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**ENGINEERING SERVICES GEOTECHNICAL SERVICES** DIVISION OF ENGINEERING SERVICES SR-710 TUNNEL TECHNICAL STUDY STATE OF N/A STRUCTURE DESIGN UNCTIONAL SUPERVISOR DRAWN BY: F. MIGUEL FIELD INVESTIGATION BY: K. LAI/J. PRATT **CALIFORNIA** POST MILES LOG OF TEST BORING SHEET 3 OF 5 AME: SHIVA KARIMI DEPARTMENT OF TRANSPORTATION **DESIGN BRANCH** CHECKED BY: K. BARKER DATE: 4/15/09 - 5/1/09 ORIGINAL SCALE IN INCHES FOR REDUCED PLANS DISREGARD PRINTS BEARING EARLIER REVISION DATES OGS CIVIL LOG OF TEST BORINGS SHEET EA 07-187900

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DIST COUNTY ROUTE POST MILES SHEET TOTAL PROJECT NO SHEETS

7 LA 710 N/A

CERTIFIED ENGINEERING GEOLOGIST

PLANS APPROVAL DATE

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SHEET TOTAL NO SHEETS

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DATE

SOURCE 210

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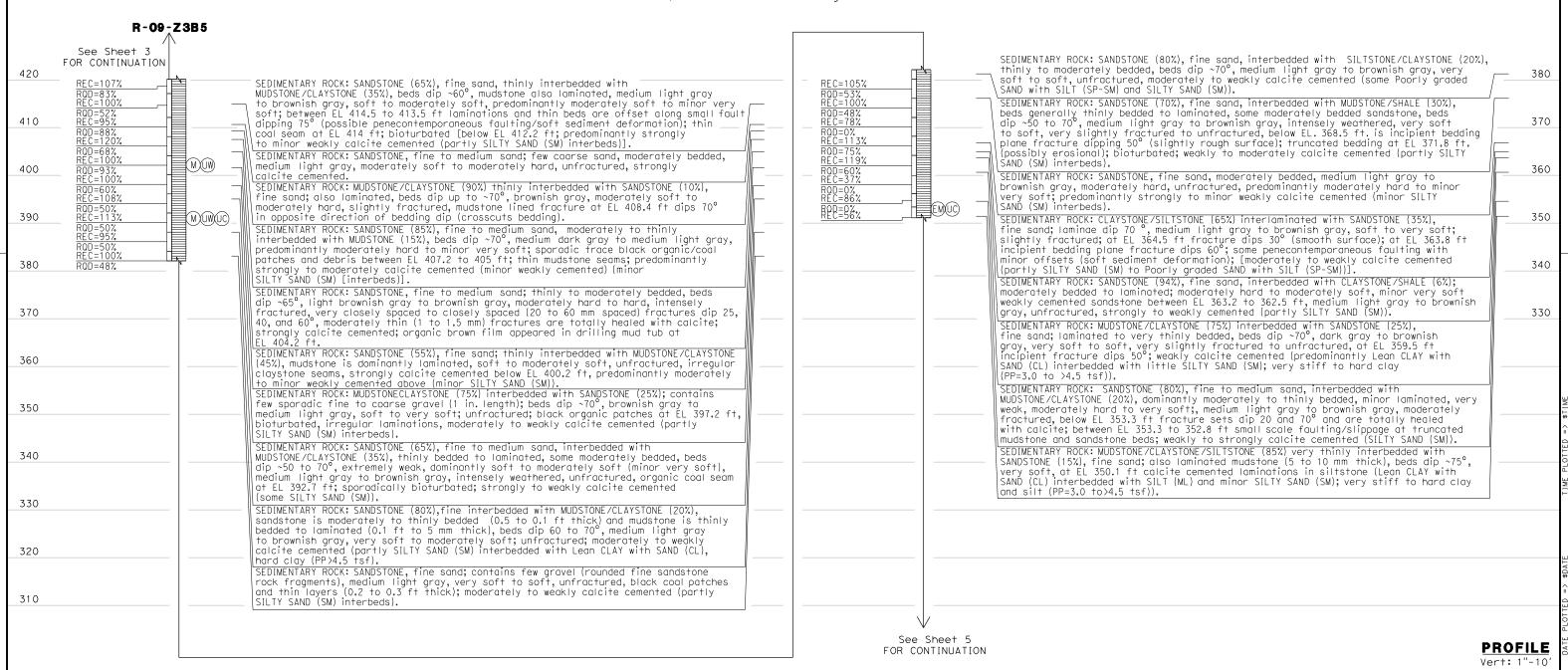
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**ENGINEERING SERVICES GEOTECHNICAL SERVICES** DIVISION OF ENGINEERING SERVICES SR-710 TUNNEL TECHNICAL STUDY STATE OF N/A STRUCTURE DESIGN UNCTIONAL SUPERVISOR DRAWN BY: F. MIGUEL FIELD INVESTIGATION BY: K. LAI/J. PRATT **CALIFORNIA** POST MILES AME: SHIVA KARIMI LOG OF TEST BORING SHEET 4 OF 5 DATE: 4/15/09 - 5/1/09 DEPARTMENT OF TRANSPORTATION **DESIGN BRANCH** CHECKED BY: K. BARKER ORIGINAL SCALE IN INCHES FOR REDUCED PLANS DISREGARD PRINTS BEARING EARLIER REVISION DATES OGS CIVIL LOG OF TEST BORINGS SHEET EA 07-187900 FILE => \$REQUEST

240

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SILT (SP-SM))]

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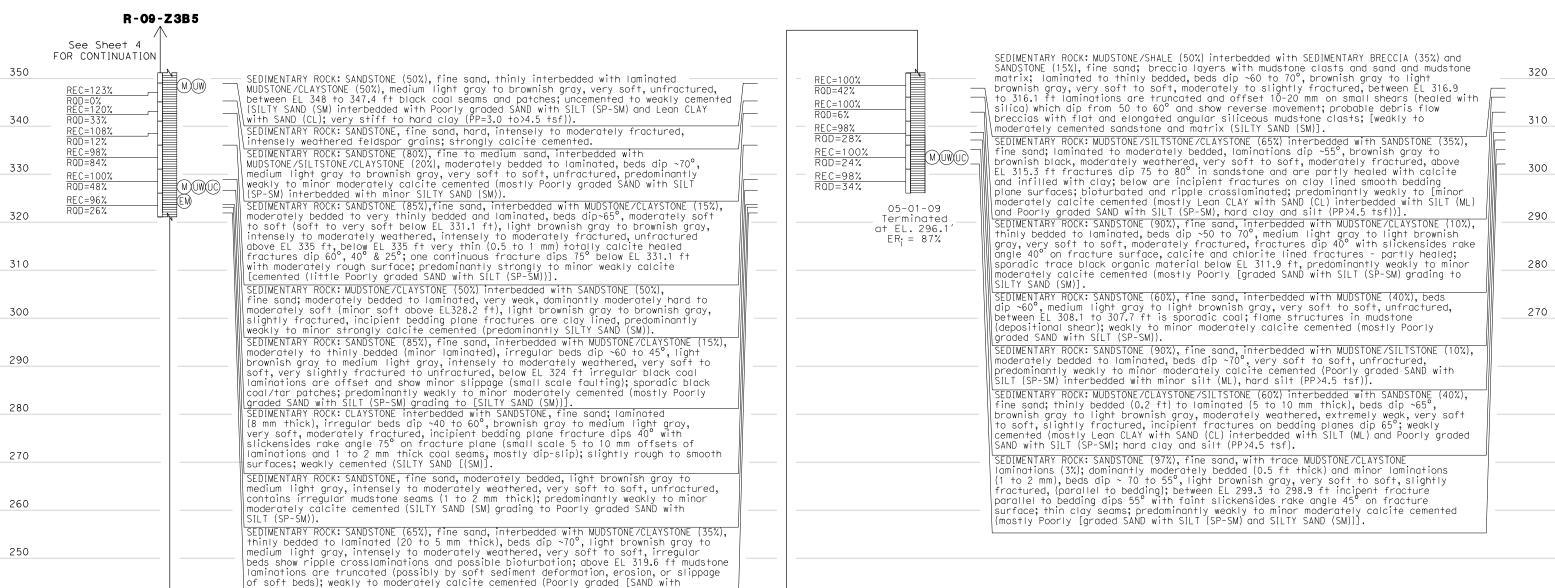
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COUNTY ROUTE IST LΑ 710 J. Fratt Joseph CERTIFIED ENGINEERING GEOLOGIST 3/18/10
DATE // JOSEPH S. PRATT No. CEG 2141 Exp. 5/31/2011 PLANS APPROVAL DATE CERTIFIED ENGINEERING GEOLOGIST The State of California or its officers or agent: shall not be responsible for the accuracy or OF CALIFO completeness of electronic copies of this plan sheet

PROFILE Vert: 1"-10'



ENCINEEDIN	NG SERVICES	GEOTECHNICAL SERVICES		DIVISION OF ENGINEERING SERVICES					
	NG SERVICES	GEOTECHNICAL SERVICES	STATE OF	STRUCTURE DESIGN	N/A	SR-710	TUNNEL 1	TECHNICAL	STUDY
FUNCTIONAL SUPERVISOR	DRAWN BY: E. MIGUEL	FIELD INVESTIGATION BY: K. LAI/J. PRATT	CALIFORNIA	SIROCIONE DESIGN	POST MILES				
NAME: SHIVA KARIMI	V DADVED	4 (4 F (00		DESIGN BRANCH	POST MILES	LOG OF	TEST BOR	RING SHEET	5 OF 5
5.1.1.1.	CHECKED BY: K. BARKER	DATE: 4/15/09 - 5/1/09	DEPARTMENT OF TRANSPORTATION	DESIGN BRANCH	N/A	Loa oi	ILOI DOI	III OIILLI	0 01 0
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OGS CIVIL LOG OF TEST BORINGS SHEET		ORIGINAL SCALE IN INCHES FOR REDUCED PLANS	0 1 2 3	EA 07-187900	EARLIER REVIS	SION DATES	-		5   6

DEREK HIGA

SIGN OFF DATE

DESIGN OVERSIGHT ENGINEER

OGS CIVIL LOG OF TEST BORINGS SHEET

NGS Benchmark used. Pt MF36F: N1847799.06, E6512460.75, Elevation 407.29' MF 36F: A 3" Brass disk stamped "Metropolitan Water District of Southern California MF 36F 1989", set in the top of the bridge curb at the northwest corner of the Hellman Avenue overcrossing of the Long Beach Freeway (I-710). 68' west of the center of the south bound lanes of the freeway, 25' north of the centerline of Hellman Avenue, 13' east of the west end of the bridge, 1' north of the north curb face. NGS PID EW9453.

DRAWN BY:

CHECKED BY:

EDELYNE MIGUEL

DAN JANKLY

M. TORSIELLO

FIELD INVESTIGATION BY

DATE: 2/17/09 - 3/11/09

ORIGINAL SCALE IN INCHES FOR REDUCED PLANS

Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. Vertical control based on NAVD-88. Pt 0153 N1845410.50, E6509860.21, Pt 0617 N1858044.3, E6491094.23.

Units are in U.S. survey feet.

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- 5) All bedding and other structural angles are measured from horizontal.
- 6) Plan view shown on Boring Location Plan sheet 2 of 2.

IST COUNTY LΑ 710 3/18/10 GEOTECHNICAL ENGINEER DATE MAHESWARAN RAVEENDRA No. GE 2743 Exp. 06-30-11 PLANS APPROVAL DATE The State of California or its officers or agents & OTECHNICAL shall not be responsible for the accuracy or completeness of electronic copies of this plan sheet. CH2M HILL 6 HUTTON CENTRE DRIVE, SUITE 700 SANTA ANA, CA 92707

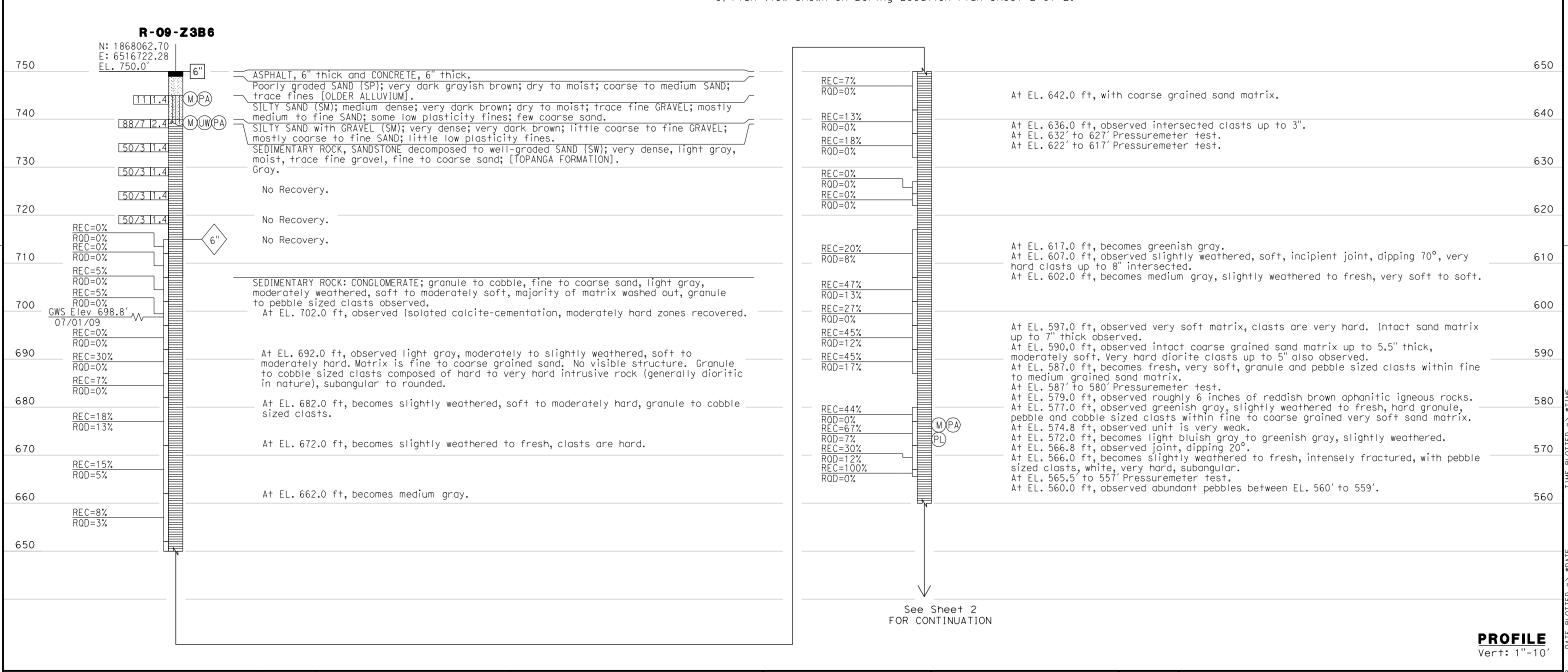
SR-710 TUNNEL TECHNICAL STUDY

LOG OF TEST BORING SHEET 1 OF 2

N/A

POST MILES

DISREGARD PRINTS BEARING EARLIER REVISION DATES



PREPARED FOR THE

STATE OF

CALIFORNIA

DEPARTMENT OF TRANSPORTATION

MAHESWARAN RAVEENDRA PROJECT ENGINEER

EA 07-187900

DEREK HIGA DESIGN OVERSIGHT ENGINEER

OGS CIVIL LOG OF TEST BORINGS SHEET

SIGN OFF DATE

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DRAWN BY:

CHECKED BY:

EDELYNE MIGUEL

DAN JANKLY

M. TORSIELLO

FIELD INVESTIGATION BY

DATE: 2/17/09 - 3/11/09

ORIGINAL SCALE IN INCHES FOR REDUCED PLANS

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DIST COUNTY ROUTE POST MILES SHEET TOTAL PROJECT NO N/A

TOTAL PROJECT NO N/A

3/18/10

GEOTECHNICAL ENGINEER

AMARESMRAN COMMAN AMARESMRAN COMMAN CO

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### 1.50 ft covered of themselved control of 20 covered by the cov	FOR CONTIN	eet 1 NUATION	area a laboratoria.	REC=59%		At EL. 497.5 ft, observed joint, dipping 65°, slickensided. At EL. 497.0 ft, observed calcite veins. At EL 495.7 ft, observed init dipping 70° slickensided. Unit is weak	
# EL 9540 ft, observed poor y injurited enginerate, Aft EL 952 ft, observed inicipient of in the joint get, if their course grained sandations with a property of the course grained sandations and the property of the course grained sandations with a property of the course grained sandations with a property of the property of the course grained sandations with a property of the property of	REC=100% ROD=29%	P	At EL. 557.0 ft, observed SANDSTONE bed, roughly 1'thick, medium-grained to granule, medium gray, slightly weathered, moderately fractured, bedding plane separation, dipping 30°, seven joints observed between EL. 557' to 555', dips range from 10 to 20°. At EL. 556.0 ft, observed intensely fractured.	- REC=50% RQD=32%	M (W)SD UCPLEMPTS -	DV COICITE.	
A 1 (1, 550, 0 ft, becomes maske, greenis gray, clasts up to "Triensected."  A 1 (1, 550, 0 ft, becomes maske, greenis gray, clasts up to "Triensected."  A 1 (1, 550, 0 ft, becomes maske, greenis gray, clasts up to "Triensected between the control of the property of the		EMPT T	At EL. 554.0 ft, observed poorly indurated conglomerate.  At EL. 552.5 ft, observed incipient joint, dipping 60°, 1' thick coarse grained sandstone	REC=32% RQD=10%		At EL. 483.0 ft, becomes slightly weathered, Clasts are ½" to 1" dia., diorite pebbles, very hard, some slickensided surfaces. At EL. 482.0 ft, observed joint, dipping 70°, slickensided. At EL. 481.9 ft, observed Intersected 6" clast.	
dipping 30°. At EL. 591.0 ft, observed joint, dipping 40 to 30°, two joints observed.  At EL. 591.0 ft, observed some ancertariety softs, foliat, dipping 35°, two joints observed.  At EL. 591.0 ft, observed some ancertariety softs, joint, dipping 35°, slickensided.  At EL. 591.0 ft, observed some ancertariety softs, joint, dipping 35°, slickensided.  At EL. 591.0 ft, observed some ancertariety softs, joint, dipping 45°.  At EL. 591.0 ft, observed some ancertariety softs, joint, dipping 45°.  At EL. 591.0 ft, becomes very thinkly beddeed, bedding joint, dipping 45°.  At EL. 591.0 ft, becomes very thinkly beddeed, bedding joint, dipping 45°.  At EL. 591.0 ft, becomes very thinkly beddeed, bedding joint, dipping 45°.  At EL. 591.0 ft, becomes very weak, moderately soft, bedding joint, dipping 40°.  At EL. 591.0 ft, becomes very weak, moderately soft, bedding joint, dipping 40°.  At EL. 591.0 ft, becomes very weak, moderately soft, bedding joint, dipping 40°.  At EL. 591.0 ft, becomes very weak, moderately soft, bedding joint, dipping 40°.  At EL. 591.0 ft, becomes very weak, moderately soft, bedding joint, dipping 40°.  At EL. 591.0 ft, becomes very weak, moderately soft, bedding joint, dipping 40°.  At EL. 591.0 ft, becomes very weak, moderately soft, bedding joint, dipping 40°.  At EL. 591.0 ft, becomes very weak, moderately soft, bedding joint, dipping 40°.  At EL. 591.0 ft, becomes very weak, moderately soft, bedding joint, dipping 40°.  At EL. 591.0 ft, becomes very weak, moderately soft, bedding joint, dipping 40°.  At EL. 591.0 ft, becomes very weak, moderately soft, bedding joint, dipping 40°.  At EL. 591.0 ft, becomes very weak, moderately soft, bedding joint, dipping 40°.  At EL. 591.0 ft, becomes very soft and fricible bedraced.  At EL. 591.0 ft, becomes very soft and fricible, bedding joint, dipping 70°, slickensided.  At EL. 591.0 ft, becomes very slickensided with the very weak.  At EL. 591.0 ft, becomes very slickensided with the very weak, moderately should be a very weak.  At EL. 591.0 ft, becomes	REC=57% RQD=13% REC=67% RQD=32%	WOWO EMPL	At ÉL. 552.0 ft, becomes massive, greenish gray, clasts up to 7" intersected.  Unit is very weak.  At EL. 551.0 ft, observed three rough to moderately rough joints observed between	REC=38% RQD=0% REC=45% RQD=19%		subangular to angular, poorly cemented.	
Af EL. 58.3. ff, observed joint, dipping 40 to 30°, two joints observed.  At EL. 58.1. ff, observed bedding joint, dipping 35°, unit is week.  At EL. 58.1. ff, observed bedding joint, dipping 35°, unit is week.  At EL. 58.1. ff, becomes very thinkly bedded, bedded, slightly weathered, light gray, coarse angular sand matrix.  At EL. 58.1. ff, becomes very whickly bedded, slightly weathered, light gray, coarse angular sand matrix.  At EL. 58.1. ff, becomes very week, moerately soft, bedding joint, dipping 40°.  At EL. 58.1. ff, becomes very week, moerately soft, bedding joint, dipping 40°.  At EL. 58.1. ff, becomes very week, moerately soft, bedding joint, dipping 40°.  At EL. 58.2. ff, becomes very week, moerately soft, bedding joint, dipping 40°.  At EL. 58.2. ff, becomes very week, moerately soft, bedding joint, dipping 40°.  At EL. 58.2. ff, becomes very week, moerately soft, bedding joint, dipping 40°.  At EL. 58.2. ff, becomes very week, moerately soft, bedding joint, dipping 40°.  At EL. 58.2. ff, becomes very week, moerately soft, bedding joint, dipping 40°.  At EL. 58.2. ff, becomes very week, moerately soft, bedding joint, dipping 40°.  At EL. 58.2. ff, becomes very week, moerately soft, bedding joint, dipping 40°.  At EL. 58.2. ff, becomes very week, moerately soft, bedding joint, dipping 70°, sandstone, white with black minerals, appears decomposed and friable bedrock.  At EL. 58.2. ff, observed bedding joint, dipping 30°, sandstone, while with black minerals, appears decomposed and friable bedrock.  At EL. 58.2. ff, observed bedding joint, dipping 70°, sandstone, while with black minerals, appears decomposed and friable bedrock.  At EL. 58.2. ff, observed bedding joint, dipping 70°, sandstone, while with black minerals, appears decomposed and friable bedrock.  At EL. 46.3. ff, observed bedding joint, dipping 70°, sandstone, while with black minerals, appears decomposed of the first fine for bedding joint, dipping 70°, sandstone, while with black minerals, appears decomposed of the first fine for bedd	REC=83% ROD=83% REC=63% RQD=22%	MUNUO 1	At EL. 545.0 ft, becomes very fnickly bedded, moderately fractured, bedding joint, dipping 30°.	REC=15% RQD=0%			
At EL. 531.0 ft, boserved 6' thick lens with medium grained sand matrix.  At EL. 528.5 ft, becomes very weak, moderately soft, bedding joint, dipping 40°.  At EL. 527.7 ft, but it is very weak to cobble sized clasts.  At EL. 527.7 ft, but it is very weak to expend giont, dipping 35°, unit is very weak to weak.  At EL. 523.0 ft, observed bedding joint, dipping 35°, unit is very weak to weak.  At EL. 522.0 ft, observed sandstone lens, lightly reachered sandstone lens, groy, fine to medium grained.  At EL. 520.5 ft, observed sandstone lens, groy, fine to medium grained.  At EL. 519.5 ft, observed pebble to gravel sized clasts, subangular to angular, local slickensided surfaces.  SEDIMENTARY ROCK: SANDSTONE, medium to coarse grained, angular, friable, possibly sheared.  SEDIMENTARY ROCK: CONGLOMERATE, slightly weathered to fresh, moderately hard, pebble sized clasts within well indurated sandstone matrix.  At EL. 437.0 ft, becomes very soft and friable, pebbles within sandstone are decomposed and easily richely, numerous slickensided surfaces.  At EL. 437.0 ft, becomes moderately to slightly weathered, soft, sandstone to siltstone, numerous ner vertical slickensided surfaces.  At EL. 437.0 ft, becomes moderately to slightly weathered, soft, sandstone are decomposed and easily clickensided surfaces.  At EL. 437.0 ft, becomes moderately to slightly weathered, soft, sandstone to siltstone, numerous ner vertical slickensided surfaces.  At EL. 438.0 ft, observed bedding joint, dipping 25°, local ½° thick beds of sandstone and siltstone, numerous ner vertical slickensided surfaces.  At EL. 437.0 ft, becomes moderately to slightly weathered, soft, sandstone to siltstone, numerous numerous numerous decomposed diorite, very dark gray. Numerous gray, and slickensided surfaces which are friable, pebbles within sandstone and slickensided surfaces.  At EL. 437.0 ft, becomes weak moderately soft, sandstone to siltstone, numerous slickensided surfaces which are friable, pebbles within sandstone and slickensided surfaces.  At EL.	R0D=0% <u>REC=100%</u> R0D=0% REC=13%		At EL. 543.3 ft, observed joint, dipping 40 to 30°, two joints observed.  At EL. 541.0 ft, observed bedding joint, dipping 35°, unit is weak.  At EL. 540.0 ft to 527.0 ft, observed numerous joints, dipping 0°.  At EL. 535.6 ft, becomes very thinly bedded, bedding joint, dipping 45°.  At Fl. 535.0 ft. becomes very thickly bedded. slightly weathered. light gray, coarse	RQD=24% REC=100% RQD=87%		At EL. 453. to El. 424.  At EL. 452.8 ft, becomes massive, bluish gray, moderately to slightly weathered, extremely weak, very soft to soft, mostly fine to medium sand. Little fine to coarse, angular grayel.	
SEDIMENTARY ROCK: CONGLOMERATE, slightly weathered to fresh, moderately hard, pebble sized  SEDIMENTARY ROCK: CONGLOMERATE, slightly weathered to fresh, moderately hard, pebble sized  Clasts within well indurated sandstone matrix.  At EL. 437.0 ft, becomes moderately to slightly weathered, soft, sandstone to siltstone, numerous near vertical slickensided surfaces 80 to 90° dip.  Clasts within well indurated sandstone matrix.  At EL. 435.0 ft, observed bedding joint, dipping 70°, dark gray slickensided surfaces.  At EL. 434.8 ft, observed possibly decomposed diorite, very dark gray. Numerous gray, clay lined slickensided surfaces which are friable.  At EL. 432.0 ft, observed numerous joints, likely continuation of shear zone from El. 453'.  At EL. 432.0 ft, observed Sandstone with some pebbles, gray to dark gray, friable, numerous slickensided surfaces.  At EL. 432.0 ft, becomes moderately soft.	RQD=0%		At EL. 531.8 ft, Unit is weak. At EL. 531.0 ft, observed 6" thick lens with medium grained sand matrix.	RQD=48% REC=100% RQD=72%		At EL. 450.0 ft, observed sheared and friable bedrock. At EL. 446.3 ft, observed bedding joint, dipping 20°, Sandstone, white with black minerals, appears decomposed due to shearing.	
SEDIMENTARY ROCK: SANDSTONE, medium to coarse grained, angular, friable, possibly sheared.  SEDIMENTARY ROCK: CONGLOMERATE, slightly weathered to fresh, moderately hard, pebble sized numerous near vertical slickensided surfaces 80 to 90° dip.  Clasts within well indurated sandstone matrix.  At EL. 506.0 ft, observed random fracture, dipping 70°, dark gray slickensided surfaces.  At EL. 431.0 ft, observed possibly decomposed diorite, very dark gray. Numerous gray, clay lined slickensided surfaces which are friable.  At EL. 432.0 ft, observed numerous joints, likely continuation of shear zone from El. 453'.  At EL. 432.0 ft, observed Sandstone with some pebbles, gray to dark gray, friable, numerous slickensided surfaces.  At EL. 431.0 ft, becomes moderately soft.			At EL. 521.0 ft, observed granule to cobble sized clasts.  At EL. 523.0 ft, observed bedding joint, dipping 35°, unit is very weak to weak.  At EL. 522.0 ft, observed Sandstone lens, light gray, fine to course grained, friable.  At EL. 520.5 ft, observed Sandstone lens, gray, fine to medium grained.  At EL. 519.5 ft, observed pebble to gravel sized clasts, subangular to angular, local slickensided surfaces.	Teri	minated at	within joints. At EL. 439.0 ft, observed bedding joint, dipping 25°, Unit is sheared, continuing from El. 453′.	
clay lined slickensided surfaces which are friable.  At EL. 433.0 ft, observed numerous joints, likely continuation of shear zone from EI. 453'. At EL. 432.0 ft, observed Sandstone with some pebbles, gray to dark gray, friable, numerous slickensided surfaces. At EL. 431.0 ft, becomes moderately soft.			SEDIMENTARY ROCK: SANDSTONE, medium to coarse grained, angular, friable, possibly sheared.  SEDIMENTARY ROCK: CONGLOMERATE, slightly weathered to fresh, moderately hard, pebble sized clasts within well indurated sandstone matrix.	E	ER <sub>i</sub> = 75%	and easily friable, numerous slickensided surfaces.  At EL. 437.0 ft, becomes moderately to slightly weathered, soft, sandstone to siltstone,  numerous near vertical slickensided surfaces 80 to 90° dip.  At EL. 435.5 ft, observed bedding joint, dipping 25°, Local %" thick beds of sandstone and siltstone.  At EL. 434.8 ft, observed possibly decomposed diorite, very dark gray. Numerous gray,	
						clay lined slickensided surfaces which are friable.  At EL. 433.0 ft, observed numerous joints, likely continuation of shear zone from EI. 453'.  At EL. 432.0 ft, observed Sandstone with some pebbles, gray to dark gray, friable, numerous slickensided surfaces.	

MAHESWARAN RAVEENDRA
PROJECT ENGINEER

N/A

SR-710 TUNNEL TECHNICAL STUDY

POST MILES
N/A

LOG OF TEST BORING SHEET 2 OF 2

CU
EA 07-187900

DISREGARD PRINTS BEARING
EARLIER REVISION DATES

SHEET OF
EARLIER REVISION DATES

2 2

FILE => \$REQUEST

PREPARED FOR THE STATE OF

CALIFORNIA

DEPARTMENT OF TRANSPORTATION

UNCTIONAL SUPERVISOR

AME: SHIVA KARIMI

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DRAWN BY: F. MIGUEL

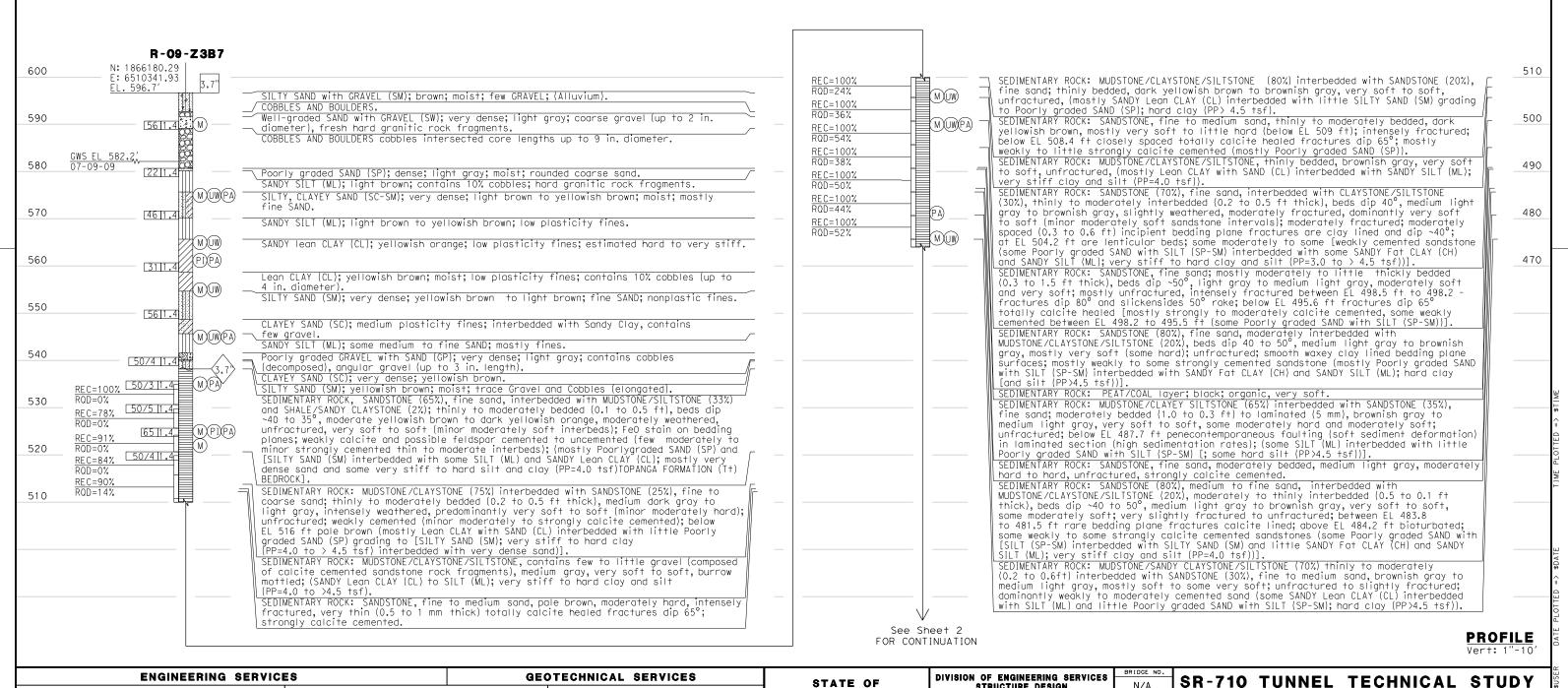
CHECKED BY: K. BARKER

Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. Vertical control based on NAVD-88. P+ 0153 N1845410.50, E6509860.21, P+ 0617 N1858044.3, E6491094.23.

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COUNTY LΑ 710 1. Pratt CERTIFIED ENGINEERING GEOLOGIST DATE JOSEPH S. No. CEG 2141 Exp. 5/31/2011 PLANS APPROVAL DATE CERTIFIED ENGINEERING GEOLOGIST the State of California or its officers or agents hall not be responsible for the accuracy or completeness of electronic copies of this plan sheet



STATE OF

**CALIFORNIA** 

DEPARTMENT OF TRANSPORTATION

FIELD INVESTIGATION BY: J. PRATT/A. TSEGIE

ORIGINAL SCALE IN INCHES FOR REDUCED PLANS

DATE: 1/29/09 - 2/20/09

N/A

POST MILES

DISREGARD PRINTS BEARING EARLIER REVISION DATES

LOG OF TEST BORING SHEET 1 OF 5

STRUCTURE DESIGN

**DESIGN BRANCH** 

EA 07-187900 FILE => \$REQUEST

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**ENGINEERING SERVICES** 

DRAWN BY: F. MIGUEL

CHECKED BY: K. BARKER

UNCTIONAL SUPERVISOR

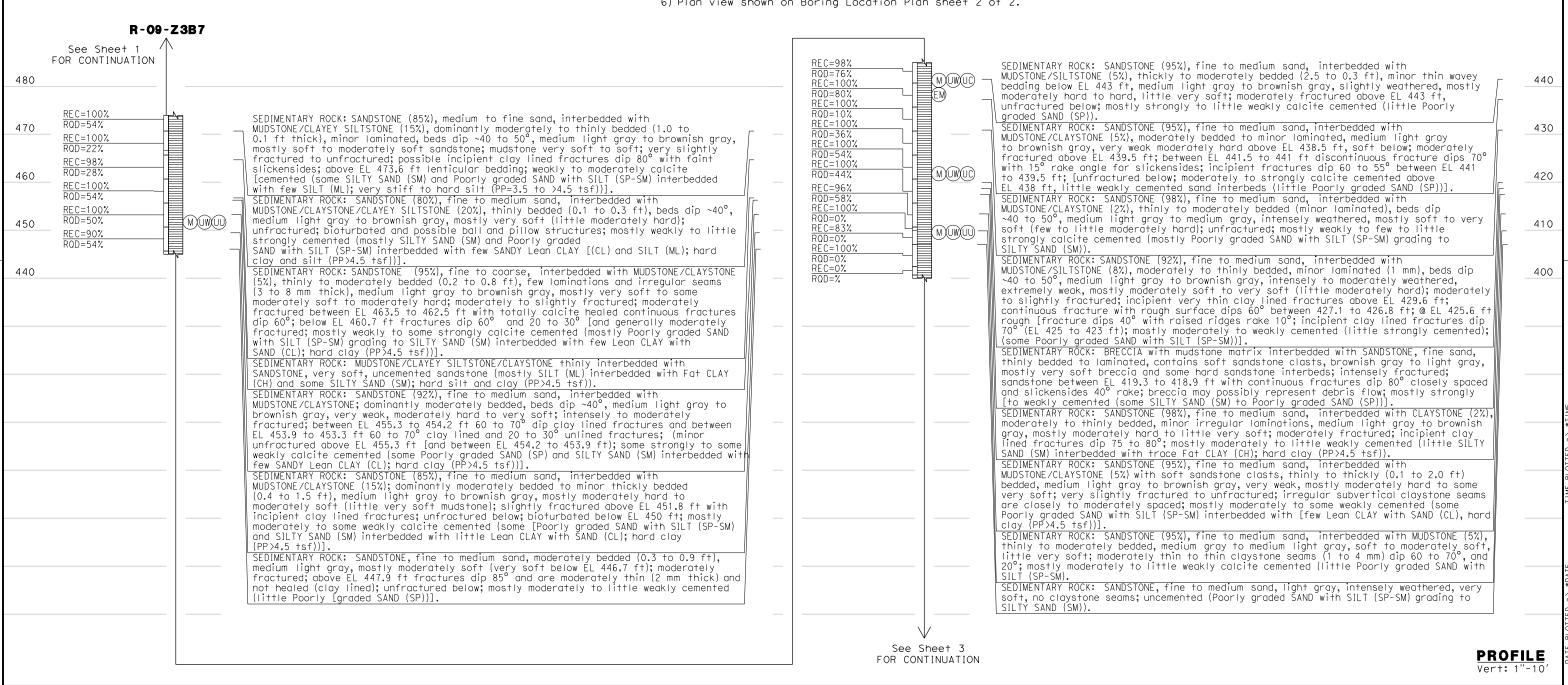
AME: SHIVA KARIMI

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STATE OF

**CALIFORNIA** 

**GEOTECHNICAL SERVICES** 

DATE: 1/29/09 - 2/20/09

FIELD INVESTIGATION BY: J. PRATT/A. TSEGIE

ORIGINAL SCALE IN INCHES FOR REDUCED PLANS

N/A

POST MILES

SR-710 TUNNEL TECHNICAL STUDY

DIVISION OF ENGINEERING SERVICES

STRUCTURE DESIGN

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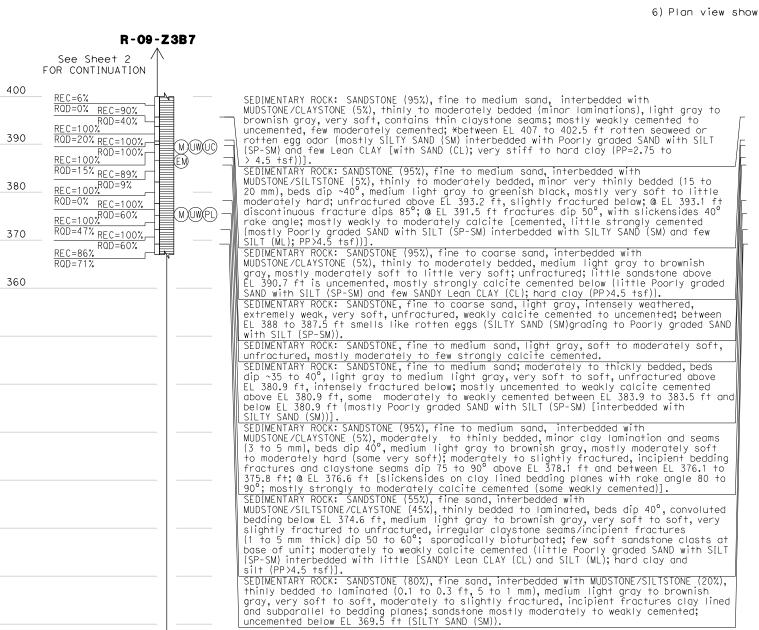
See Sheet 4

FOR CONTINUATION

- 5) All bedding and other structural angles are measured from horizontal.
- 6) Plan view shown on Boring Location Plan sheet 2 of 2.

COUNTY ROUTE LΑ 710 A. Pratt Joseph CERTIFIED ENGINEERING GEOLOGIST JOSEPH S. PRATT No. CEG 2141 Exp. 5/31/2011 PLANS APPROVAL DATE CERTIFIED ENGINEERING GEOLOGIST The State of California or its officers or agent: shall not be responsible for the accuracy or OF CALIF completeness of electronic copies of this plan sheet

**PROFILE** 



MUWUC SEDIMENTARY ROCK: SANDSTONE (90%), fine to coarse sand, interbedded with MUDSTONE/SILTSTONE/CLAYSTONE (10%), thinly to moderately bedded, minor laminations, beds MUDSIONE/SELISIONE/CLAYSIONE (10%), fining to moderately bedded, minor laminations, beds dip ~40°, medium light gray to brownish gray, intensely weathered, extremely weak soft to very soft above EL 365.7 ft, moderately hard to moderately soft below; moderately to slightly fractured, incipient fractures (claystone seams) dip 40 to 80° and increase below EL 363.5 ft; mostly moderately to little weakly cemented above [EL 365.7 ft, moderately to strongly calcite cemented below (little SILTY SAND (SM) interbedded with few SILT (ML) and Lean CLAY with SAND (CL); hard silt and clay (PP>4.5 tsf))].

SEDIMENTARY ROCK: SANDSTONE (95%), fine to coarse sand, interbedded with MUDSTONE/CLAYSTONE (5%), moderately to thinly bedded (1 to 0.1 ft), minor laminated, beds dip ~40° medium light gray to brownish gray intensely weathered mostly moderately REC=100% RQD=68% EM) 360 M)UW)UC) REC=100% EM) 350 RQD=% REC=97% RQD=47% dip ~40°, medium light gray to brownish gray, intensely weathered, mostly moderately hard, few claystone laminations very soft; very slightly fractured to unfractured, incipient fractures (claystone seams) dip 60°; contains a cobble (6 in. diameter) below M)UWUU) RQD=40% 340 EL 361.7 ft (subrounded hard granitic rock fragment); [mostly moderately to strongly calcite cemented, some weakly cemented (some Poorly graded SAND with SILT (SP-SM) interbedded with few Lean CLAY with SAND (CL), hard clay)]. SEDIMENTARY ROCK: SANDSTONE (90%), fine to coarse sand, interbedded with MUDSTONE/CLAYSTONE (10%), thinly bedded (50 to 60 mm), beds dip ~40°, contains a cobble (10 in diameter) and coarse gravel below EL 354.3 ft, light gray to brownish 330 gray, very weak, moderately soft to moderately hard, slightly fractured to unfractured; @ EL 356.9 ft incipient bedding plane fracture dips 40°, slickenside rake 30°; @ EL 357.2 ft penecontemporaneous faulting (soft sediment deformation) with minor offset [subrounded hard quartz diorite rock fragments; mostly moderately to some strongly calcite cemented].

SEDIMENTARY ROCK: SANDSTONE (90%), fine to medium, interbedded with MUDSTONE/CLAYSTONE (10%), thinly to moderately bedded, minor laminated, beds dip ~50°, light gray to brownish gray, intensely weathered, mostly moderately soft to little moderately hard above EL 350.1 ft, very soft to moderately soft below; very slightly fractured to unfractured; incipient fractures dip 60 to 70°; penecontemporaneous faulting (soft sediment deformation) between EL 350.7 to [350.1 ft, moderately to strongly calcite cemented above EL 350.1 ft, moderately to weakly cemented below (below EL 350.1 ft, some SILTY SAND (SM) interbedded with Lean CLAY with SAND (CL); hard clay (PP>4.5 +sf)]. Clay (PP)4.5 tsf))].

SEDIMENTARY ROCK: SANDSTONE (90%), fine to medium sand, interbedded with MUDSTONE/SILTSTONE (10%), contains a boulder (35%, 14 in. diameter) below EL 345.7 ft, thinly to moderately bedded, light gray to brownish gray, very weak mostly very soft to some moderately hard; unfractured; subrounded very soft decomposed quartz diorite rock fragment (weathered to clays, medium dark gray); mostly weakly to some strongly calcite cemented (mostly Poorly graded SAND with SILT (SP-SM) [interbedded with SILT (ML))].

SEDIMENTARY ROCK: SANDSTONE (90%), fine to medium sand, interbedded with MUDSTONE/SILTSTONE (10%), thinly to moderately bedded, medium gray to brownish gray, mostly very soft above EL 342.2 ft, moderately hard below; unfractured; minor oil/tar @ EL 341.1 ft; mostly weakly calcite cemented above EL 342.2 ft, strongly to moderately calcite cemented below (above EL 342.2 ft mostly Poorly graded SAND with SILT [(SP-SM) grading to SILTY SAND (SM) interbedded with few SILT (ML); hard silt (PP>4.5 tsf))]. SEDIMENTARY ROCK: SANDSTONE (92%), fine to coarse sand, interbedded with MUDSTONE/SILITSTONE (8%); moderately to thinly bedded 0.2 to 1.0 ft), minor laminated; beds dip ~50°, light brownish gray to medium light gray, mostly very soft to some moderately hard; unfractured; weakly to moderately calcite cemented (little strongly cemented); (mostly Well graded SAND with SILT (SW-SM) interbedded with few SILT (ML); hard silt (PP>4.5 tsf)).

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	DRAWN BY: E. MIGUEL		FIELD INVESTIGATION BY: J. PRATT/A. TSEGIE	CALIFORNIA		POST MILES						
NAME: SHIVA KARIMI	CHECKED BY: K. BARKER		DATE: 1/29/09 - 2/20/09	DEPARTMENT OF TRANSPORTATION	DESIGN BRANCH	N/A	LOG OF	TEST B	ORING SHE	£T 3	, OF	5
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#### COUNTY **BENCHMARK:** LΑ 710 NGS Benchmark used. Pt MF36F: N1847799.06, E6512460.75, Elevation 407.29' 1. Pratt 1) This LOTB sheet was prepared in accordance with the Caltrans Soil and CERTIFIED ENGINEERING GEOLOGIST 3/18/10 DATE MF 36F: A 3" Brass disk stamped "Metropolitan Water District of Southern California Rock Logging, Classification and Presentation Manual (June 2007) MF 36F 1989", set in the top of the bridge curb at the northwest corner of the Hellman except as noted in Appendix A.1 of the Final Geotechnical Summary Report Avenue overcrossing of the Long Beach Freeway (I-710). 68' west of the center of the south bound lanes of the freeway, 25' north of the centerline of Hellman Avenue, 13' JOSEPH S. SR-710 Tunnel Technical Study Los Angeles County, California dated (April, 2010). No. CEG 2141 east of the west end of the bridge, 1'north of the north curb face. NGS PID EW9453. Exp. 5/31/2011 PLANS APPROVAL DATE CERTIFIED ENGINEERING GEOLOGIST 2) California ring, standard penetration test, and HQ&PQ core samplers Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. were used to obtain soil and rock samples. the State of California or its officers or agents hall not be responsible for the accuracy or Vertical control based on NAVD-88. completeness of electronic copies of this plan sheet P+ 0153 N1845410.50, E6509860.21, 3) An automatic trip hammer system consisting of a hammer weight of 140 lbs P+ 0617 N1858044.3, E6491094.23. falling a distance of 30" was used to advance the drive samplers. Units are in U.S. survey feet. 4) To convert from California ring sampler blow count to equivalent standard penetration test sampler blow count, for granular and cohesive soil, multiply 5) All bedding and other structural angles are measured from horizontal. 6) Plan view shown on Boring Location Plan sheet 2 of 2. R-09-Z3B7 See Sheet 3 FOR CONTINUATION 310 SEDIMENTARY ROCK: Boulder CONGLOMERATE, boulder 22 in. diameter, (may be just singular boulder isolated in sandstone above and below), subrounded soft (intensely weathered) REC=100% RQD=38% SEDIMENTARY ROCK: SANDSTONE (92%), fine to coarse sand, interbedded with MUDSTONE/SILTSTONE (8%), moderately to thinly bedded, minor laminated (2 mm), beds quartz diorite rock fragment. M (UW)(UC) (M)(W)(UU) REC=100% SEDIMENTARY ROCK: SANDSTONE (96%), fine to coarse sand, interbedded wit PTS(EM/PL) RQD=50% $dip \sim 40^{\circ}$ , light brownish gray to medium light gray, mostly moderately hard to hard, MUDSTONE/CLAYSTONE (4%), moderately to thinly bedded, beds dip ~40 to 55°, medium 300 330 MUDSIONE/CLAYSIONE (4%), moderately to thinly bedded, beds dip ~40 to 55°, medium light gray to brownish gray, very weak, mostly moderately soft to some very soft, little moderately hard; intensely to moderately fractured; incipient clay lined fractures (mudstone seams) dip from 40 to 75° and cross eachother; mostly moderately to some weakly calcite cemented, little [strongly cemented (some Well graded SAND with SILT (SW-SM) interbedded with trace Lean CLAY with SAND (CL); hard clay (PP>4.5 tsf))]. SEDIMENTARY ROCK: Coarse Gravel and Cobble CONGLOMERATES (50%) thinly to moderately interbedded with SANDSTONE (50%), cobbles (4 in. diameter) and gravel (2 in. diameter), little very soft; very slightly fractured to unfractured; @ 331.3 ft bioturbated beds; REC=98% RQD=32% REC=90% mostly strongly to moderately calcite cemented, little weakly cemented (little Well graded SAND with SILT (SW-SM) interbedded with few [SILT (ML); hard silt (PP>4.5 tsf))]. SEDIMENTARY ROCK: SANDSTONE (92%), fine to medium sand, interbedded with MUDSTONE/SILTSTONE (8%), moderately to thinly bedded (0.7 to 0.1 ft), beds dip ~40°, light gray to brownish gray, very soft to moderately soft interbeds; unfractured; weakly to moderately cemented (mostly SILTY SAND (SM) grading to Poorly graded SAND with SILT RQD=32% 320 290 **₩WW** fine sand, medium light gray, very soft to moderately soft sandstone; cobbles and gravel are composed of subrounded moderately hard (intensely weathered) quartz diorite rock SP-SM) interbedded with few SILT (ML)). SEDIMENTARY ROCK: SANDSTONE (92%), medium to coarse sand, interbedded with 310 280 MUDSTONE/SILTSTONE (8%), thinly to moderately bedded, light gray to brownish gray, very weak, very soft, unfractured, bioturbated below EL 324.7 ft; mostly weakly to little fragments; weakly to moderately cemented (some cobbles and gravel interbedded with few Poorly graded SAND with [SILT (SP-SM))]. SEDIMENTARY ROCK: SANDSTONE (85%), fine to coarse sand, interbedded with moderately cemented (mostly Poorly graded SAND with SILT (SP-SM) interbedded with few SILT (ML); hard silt (PP>4.5 tsf)). MUDSTONE/CLAYSTONE (7%) and Coarse Gravel CONGLOMERATE (8%, below EL 296.3 ft), thinly to SEDIMENTARY ROCK: SANDSTONE (96%), fine to medium sand, interbedded with MUDSTONE/SILTSTONE (4%); moderately bedded (0.3 to 0.5 ft), minor laminations and irregular seams, beds dip 30 to 40°, medium light gray to brownish gray, moderately soft to very soft; very slightly fractured to unfractured; between EL 324.1 to 323.8 ft incipient clay lined fractures (irregular seams) dip 70 to 80°; mostly moderately to weakly calcite cemented (some Poorly graded SAND with SILT (SP-SM) [interbedded with SILT (MI); bord sit (PP) 5 tef)] moderately bedded, medium light gray to brownish gray, intensely weathered, soft to very soft; unfractured; coarse gravel composed of moderately hard quartz diorité rock fragments; trace thin shelled pelecypod (clam) debris (composed of high Mg calcite or aragonite-fizzes vigorously with HCL) is scattered between EL 298.9 to [296.3 ft; mostly moderately to weakly cemented, little strongly cemented (Well graded SAND with SILT (SW-SM) interbedded with few Poorly graded Gravel (GP) and few Lean CLAY with SAND (CL); (ML); hard silt (PP>4.5 tsf))]. SEDIMENTARY ROCK: Boulder/Cobbie CONGLOMERATE; contains 14 to 10 in. diameter boulders SEDIMENTARY ROCK: SANDSTONE (92%), fine sand, interbedded with MUDSTONE/SILTSTONE (8%); and cobbles, medium gray, unfractured, subrounded moderately soft to moderately hard thinly bedded to laminated, medium light gray to brownish gray, very soft to soft, intensely ťo moderaťely weathered) quartz diorite rock fragments; no matrix apparent. unfractured, contains irregular thin siltstone laminations; mostly weakly to some moderately cemented (mostly Poorly graded SAND with SILT (SP-SM) interbedded with few SILT (ML), very stiff to hard silt (PP=3.75 to >4.5 tsf)). SEDIMENTARY ROCK: SANDSTONE, medium sand, thinly to moderately bedded, medium light gray, vuggy, mostly moderately hard to little very soft; unfractured; mostly strongly to little weakly calcite cemented, pelecypod shell debris below EL 292.7 ft (little Poorly graded SAND with SILT (SP-SM)). SEDIMENTARY ROCK: SANDSTONE (95%), medium sand, interbedded with MUDSTONE/SILTSTONE (5%), thin siltstone seams, medium gray to brownish gray, soft, unfractured, mostly moderately to little strongly cemented. SEDIMENTARY ROCK: Gravel CONGLOMERATE with mudstone matrix, medium gray to brownish gray, soft, unfractured, rounded soft intensely weathered medium sandstone clasts in a gray, sorr, unractured, rounded sorr intensery weather ed medium sandstone class in a soft mudstone/siltstone matrix, probable depositional conglomerate. SEDIMENTARY ROCK: SANDSTONE (95%), medium to fine sand, interbedded with MUDSTONE/SILTSTONE (5%), contains a thin (0.2 ft thick) Gravel CONGLOMERATE layer below EL 315.9 ft, thinly to moderately bedded, beds dip ~35 to 50°, medium gray to brownish gray, intensely weathered, extremely weak, mostly moderately soft to moderately hard, little very soft; unfractured; gravel conglomerate has a medium sandstone matrix; contains few laminations; mostly moderately to little strongly calcite cemented, little uncemented to weakly cemented (little [Poorly graded SAND with SILT (SP-SM) interbedded with few SILT (ML); hard silt (PP)4.5 tsf))]. SEDIMENTARY ROCK: Cobble CONGLOMERATE, contains 7 in. diameter cobbles, no matrix apparent, very light gray, contains subangular to subrounded hard (slightly weathered) to moderately hard (intensely weathered) quartz diorite rock fragments; unfractured; upper cobble is slightly foliated; uncemented (Cobbles). SEDIMENTARY ROCK: SANDSTONE (96%), fine to medium sand, interbedded with MUDSTONE/CLAYSTONE (4%), dominantly thinly to moderately bedded (30 to 110 mm), minor laminated (1 to 7 mm), beds dip ~30 to 50°, very light gray to brownish gray, very weak, moderately soft, intensely to moderately fractured, closely to moderately spaced incipient fractures (claystone seams) cross eachother, moderately to weakly calcite cemented (some Poorly graded SAND with SILT (SP-SM) interbedded with trace Lean CLAY with SAND (CL); hard clay (PP>4.5 tsf)). See Sheet 5 **PROFILE** FOR CONTINUATION **ENGINEERING SERVICES GEOTECHNICAL SERVICES** DIVISION OF ENGINEERING SERVICES SR-710 TUNNEL TECHNICAL STUDY STATE OF N/A STRUCTURE DESIGN UNCTIONAL SUPERVISOR FIELD INVESTIGATION BY: J. PRATT/A. TSEGIE DRAWN BY: F. MIGUEL CALIFORNIA POST MILES AME: SHIVA KARIMI LOG OF TEST BORING SHEET 4 OF 5 DATE: 1/29/09 - 2/20/09 DEPARTMENT OF TRANSPORTATION **DESIGN BRANCH** CHECKED BY: K. BARKER ORIGINAL SCALE IN INCHES FOR REDUCED PLANS DISREGARD PRINTS BEARING EARLIER REVISION DATES OGS CIVIL LOG OF TEST BORINGS SHEET EA 07-187900

#### COUNTY **BENCHMARK:** LΑ 710 NGS Benchmark used. Pt MF36F: N1847799.06, E6512460.75, Elevation 407.29' 1) This LOTB sheet was prepared in accordance with the Caltrans Soil and 1. Pratt CERTIFIED ENGINEERING GEOLOGIST DATE MF 36F: A 3" Brass disk stamped "Metropolitan Water District of Southern California Rock Logging, Classification and Presentation Manual (June 2007) MF 36F 1989", set in the top of the bridge curb at the northwest corner of the Hellman except as noted in Appendix A.1 of the Final Geotechnical Summary Report Avenue overcrossing of the Long Beach Freeway (I-710). 68' west of the center of the south bound lanes of the freeway, 25' north of the centerline of Hellman Avenue, 13' east of the west end of the bridge, 1' north of the north curb face. NGS PID EW9453. JOSEPH S. SR-710 Tunnel Technical Study Los Angeles County, California dated (April, 2010). No. CEG 2141 Exp. 5/31/2011 PLANS APPROVAL DATE 2) California ring, standard penetration test, and HO&PO core samplers CERTIFIED ENGINEERING GEOLOGIST The State of California or its officers or agent: Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. were used to obtain soil and rock samples. shall not be responsible for the accuracy or Vertical control based on NAVD-88. completeness of electronic copies of this plan sheet P+ 0153 N1845410.50, E6509860.21, 3) An automatic trip hammer system consisting of a hammer weight of 140 lbs P+ 0617 N1858044.3, E6491094.23. falling a distance of 30" was used to advance the drive samplers. Units are in U.S. survey feet. 4) To convert from California ring sampler blow count to equivalent standard penetration test sampler blow count, for granular and cohesive soil, multiply 5) All bedding and other structural angles are measured from horizontal. 6) Plan view shown on Boring Location Plan sheet 2 of 2. R-09-Z3B7 See Sheet 4 FOR CONTINUATION SEDIMENTARY ROCK: SANDSTONE (96%), fine to medium sand, interbedded with MUDSTONE/CLAYSTONE (4%), moderately to thinly bedded, beds dip 45 to 55°, medium light gray to brownish gray, vuggy, very soft to soft; unfractured; pelecypod debris @ EL 288.8 ft; weakly to moderately calcite cemented (mostly Poorly graded SAND with SILT (SP-SM) interbedded with trace Lean CLAY with SAND (CL); hard clay (PP>4.5 tsf)). RQD=36% SEDIMENTARY ROCK: SANDSTONE (96%), medium sand, interbedded with MUDSTONE/SILTSTONE (4%), moderately to thinly bedded, beds dip ~45 to 60°, medium light gray to brownish gray, mostly moderately soft to little very soft; mostly moderately to little strongly 280 REC=98% RQD=38% calcite cemented, little weakly cemented; calcite nodules just above basal contact (little Poorly graded SAND with SILT (SP-SM) interbedded with trace SILT (ML)). SEDIMENTARY ROCK: Gravel BRECCIA with sandstone and mudstone matrix; contains coarse M UWUC LEEM) 270 gravel and trace cobbles (5 in length), thinly to moderately bedded, brownish gray to medium light gray, vuggy, very soft to soft intensely weathered matrix; unfractured; angular hard sandstone rock fragments (dominantly coarse gravel to trace cobble size); may represent cataclastic breccia with gouge; weakly to moderately cemented (Poorly graded GRAVEL with SILT [and SAND and COBBLES (GP-GM) grading to Poorly graded gravel with CLAY and SAND (GP-GC))]. SEDIMENTARY ROCK: SANDSTONE (64%), fine to medium sand, interbedded with Cobble-Gravel CONGLOMERATE (33%) and MUDSTONE/SILTSTONE (3%); cobbles (30%, up to 6 in. diameter) and trace coarse gravel, thinly to moderately bedded, medium light gray to brownish gray, slightly weathered, some moderately hard to some very soft, very soft below EL 279.9 ft; unfractured; contains sporadic subrounded hard (slightly weathered) quartz diorite rock fragments; mostly moderately to weakly calcite cemented, some uncemented conglomerate; very hard [drilling (mostly Poorly graded SAND with SILT (SP-SM) interbedded with some cobbles and gravel and trace SILT (ML))]. SEDIMENTARY ROCK: SANDSTONE, medium sand; moderately to thinly bedded (0.3 to 0.6 ft), light gray to medium light gray, moderately hard to hard, very intensely to intensely fractured, very closely spaced (10 to 30 mm) moderately thin (2 to 3 mm) discontinuous totally calcite healed fractures dip 70 to 75°; strongly calcite cemented. SEDIMENTARY ROCK: SANDSTONE (96%), fine to medium sand, interbedded with MUDSTONE/SILTSTONE (4%), thinly to moderately bedded, minor laminated (2 mm), light gray to brownish gray, mostly very soft to some moderately hard interbeds; contains a thin calcite layer @ EL 277.7 ft; mostly weakly to some strongly calcite cemented (mostly Poorly graded SAND with SILT (SP-SM) interbedded with trace SILT (ML); very stiff SEDIMENTARY ROCK: SANDSTONE (96%), medium sand, interbedded with MUDSTONE/CLAYSTONE (4%), thinly to thickly bedded, beds dip ~40 to 55°, medium gray to light gray, weak, mostly hard to little very soft; moderately to slightly fractured (0.7 to 1.9 ft), clay lined fractures dip 60 to 75°; crosslaminated between EL 274.5 to 273.3 ft; minor fossil debris; clean; clay seams cross eachother; mostly strongly to little weakly [calcite cemented (little Poorly graded SAND with SILT (SP-SM) interbedded with trace Lean CLAY with SAND (CL); hard clay (PP>4.5 tsf))]. **PROFILE ENGINEERING SERVICES GEOTECHNICAL SERVICES** DIVISION OF ENGINEERING SERVICES SR-710 TUNNEL TECHNICAL STUDY STATE OF N/A STRUCTURE DESIGN UNCTIONAL SUPERVISOR DRAWN BY: F. MIGUEL FIELD INVESTIGATION BY: K. BARKER **CALIFORNIA** POST MILES AME: SHIVA KARIMI LOG OF TEST BORING SHEET 5 OF 5 CHECKED BY: K. BARKER DATE: 2/12/09 - 2/27/09 DEPARTMENT OF TRANSPORTATION **DESIGN BRANCH** ORIGINAL SCALE IN INCHES FOR REDUCED PLANS DISREGARD PRINTS BEARING EARLIER REVISION DATES

EA 07-187900 FILE => \$REQUEST

OGS CIVIL LOG OF TEST BORINGS SHEET

NGS Benchmark used. Pt MF36F: N1847799.06, E6512460.75, Elevation 407.29' MF 36F: A 3" Brass disk stamped "Metropolitan Water District of Southern California MF 36F 1989", set in the top of the bridge curb at the northwest corner of the Hellman Avenue overcrossing of the Long Beach Freeway (I-710). 68' west of the center of the south bound lanes of the freeway, 25' north of the centerline of Hellman Avenue, 13' east of the west end of the bridge, 1' north of the north curb face. NGS PID EW9453.

Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. Vertical control based on NAVD-88. Pt 0153 N1845410.50, E6509860.21, Pt 0617 N1858044.3, E6491094.23.

Units are in U.S. survey feet.

NOTES:

- This LOTB sheet was prepared in accordance with the Caltrans Soil and Rock Logging, Classification and Presentation Manual (June 2007) except as noted in Appendix A.1 of the Final Geotechnical Summary Report SR-710 Tunnel Technical Study Los Angeles County, California dated (April, 2010).
- 2) California ring, standard penetration test, and HO&PO core samplers were used to obtain soil and rock samples.
- 3) An automatic trip hammer system consisting of a hammer weight of 140 lbs falling a distance of 30" was used to advance the drive samplers.
- 4) To convert from California ring sampler blow count to equivalent standard penetration test sampler blow count, for granular and cohesive soil, multiply by 0.67.
- 5) All bedding and other structural angles are measured from horizontal.
- 6) Plan view shown on Boring Location Plan sheet 2 of 2.

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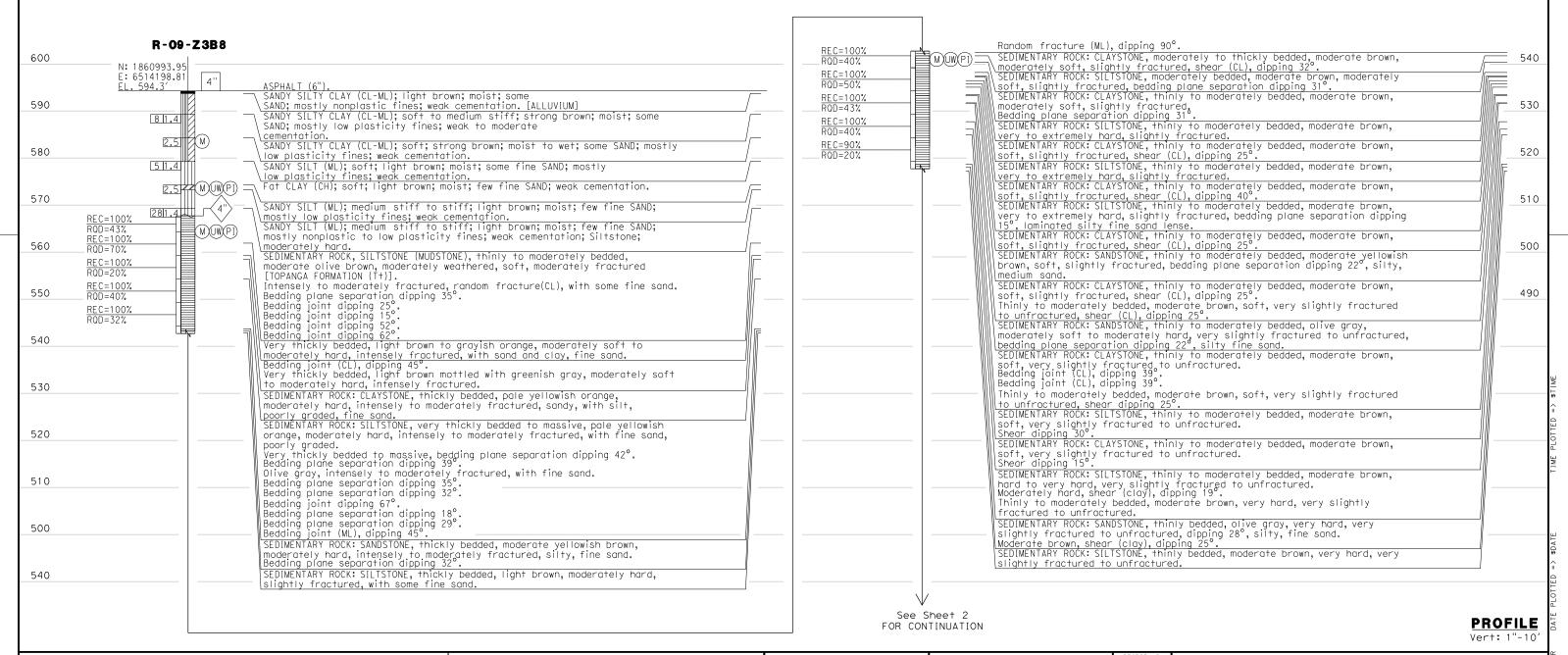
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3/18/10

DATE

PLANS APPROVAL DATE

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**ENGINEERING SERVICES GEOTECHNICAL SERVICES** DIVISION OF ENGINEERING SERVICES SR-710 TUNNEL TECHNICAL STUDY STATE OF N/A STRUCTURE DESIGN FUNCTIONAL SUPERVISOR FIELD INVESTIGATION BY: M. SALISBURY DRAWN BY: K. REYES **CALIFORNIA** POST MILES LOG OF TEST BORING SHEET 1 OF 5 AME: SHIVA KARIMI DEPARTMENT OF TRANSPORTATION **DESIGN BRANCH** J. PRATT DATE: 3/11/09 - 3/25/09 CHECKED BY: ORIGINAL SCALE IN INCHES FOR REDUCED PLANS DISREGARD PRINTS BEARING EARLIER REVISION DATES -OGS CIVIL LOG OF TEST BORINGS SHEET EA 07-187900

NGS Benchmark used. Pt MF36F: N1847799.06, E6512460.75, Elevation 407.29 MF 36F: A 3" Brass disk stamped "Metropolitan Water District of Southern California MF 36F 1989", set in the top of the bridge curb at the northwest corner of the Hellman Avenue overcrossing of the Long Beach Freeway (I-710). 68' west of the center of the south bound lanes of the freeway, 25' north of the centerline of Hellman Avenue, 13' east of the west end of the bridge, 1' north of the north curb face. NGS PID EW9453.

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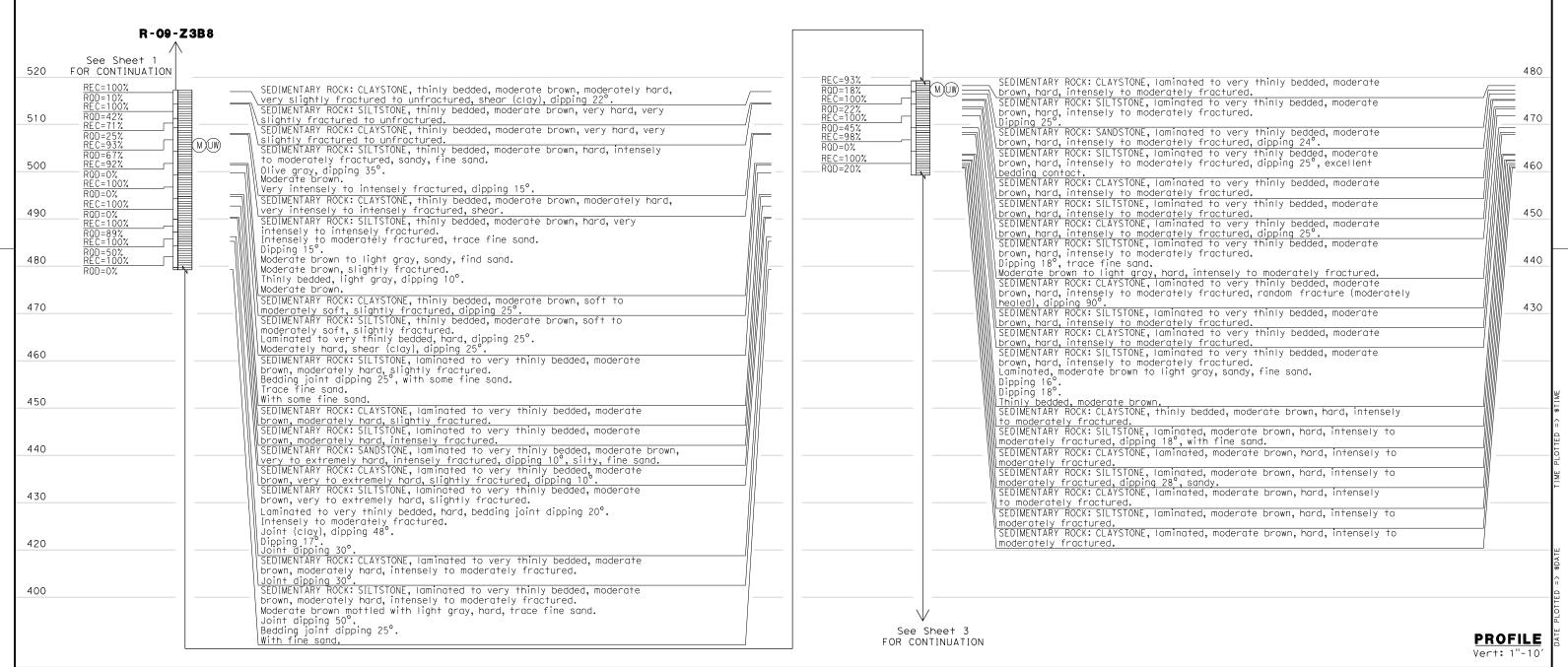
**ENGINEERING SERVICES** 

Units are in U.S. survey feet.

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- 3) An automatic trip hammer system consisting of a hammer weight of 140 lbs falling a distance of 30" was used to advance the drive samplers.
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- 5) All bedding and other structural angles are measured from horizontal.
- 6) Plan view shown on Boring Location Plan sheet 2 of 2.

DIST COUNTY 710 ΙΔ MICHAEL A. SALISBURY No. CEG 2462 EXP. 2/28/2011 CERTIFIED ENGINEERING GEOLOGIST PLANS APPROVAL DATE The State of California or its officers or agents completeness of electronic copies of this plan sheet

SR-710 TUNNEL TECHNICAL STUDY



STATE OF

DIVISION OF ENGINEERING SERVICES

**GEOTECHNICAL SERVICES** 

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Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. Vertical control basedon American Vertical Datum 1988. Pt 0153 N1845410.50, E6509860.21, Pt 0617 N1858044.3, E6491094.23.

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- 5) All bedding and other structural angles are measured from horizontal.
- 6) Plan view shown on Boring Location Plan sheet 2 of 2.

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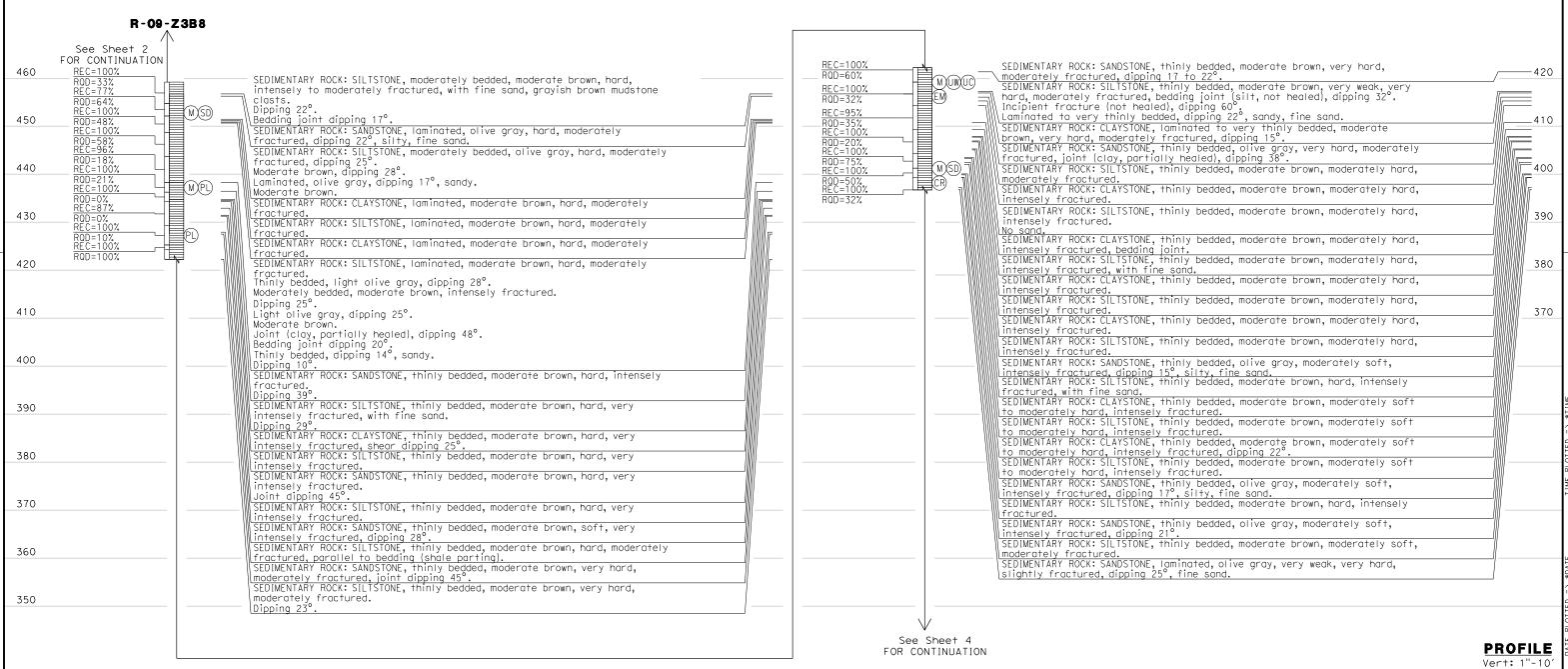
POST MILES SHEET TOTAL NO N/A

3/18/10

DATE

A SALISBURY NO. CEG 2462

EXP. 2/28/2011



**ENGINEERING SERVICES GEOTECHNICAL SERVICES** DIVISION OF ENGINEERING SERVICES SR-710 TUNNEL TECHNICAL STUDY STATE OF N/A STRUCTURE DESIGN UNCTIONAL SUPERVISOR FIELD INVESTIGATION BY: M. SALISBURY DRAWN BY: K. REYES **CALIFORNIA** POST MILES LOG OF TEST BORING SHEET 3 OF 5 AME: SHIVA KARIMI J. PRATT DEPARTMENT OF TRANSPORTATION **DESIGN BRANCH** DATE: 3/11/09 - 3/25/09 CHECKED BY: N/A ORIGINAL SCALE IN INCHES FOR REDUCED PLANS DISREGARD PRINTS BEARING EARLIER REVISION DATES OGS CIVIL LOG OF TEST BORINGS SHEET EA 07-187900

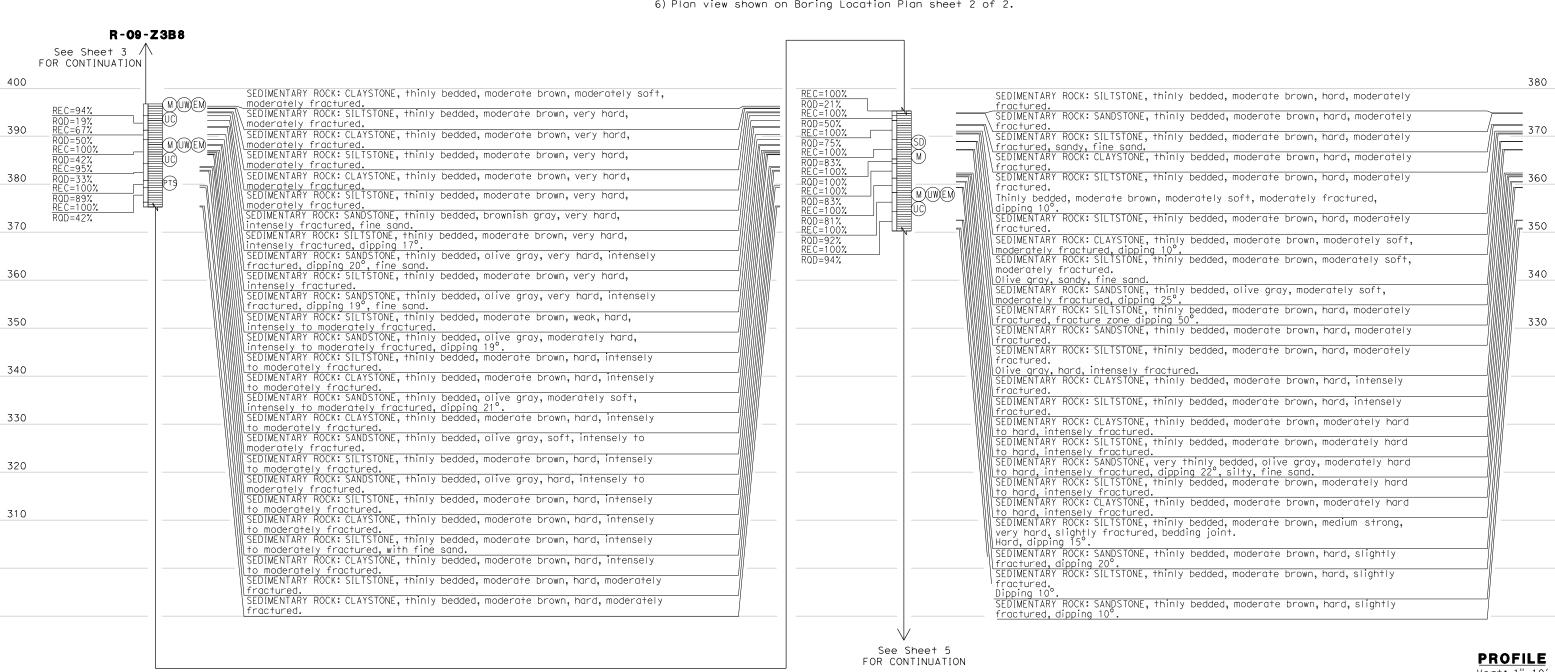
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Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. Vertical control basedon American Vertical Datum 1988. P+ 0153 N1845410.50, E6509860.21, Pt 0617 N1858044.3, E6491094.23.

Units are in U.S. survey feet.

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- 5) All bedding and other structural angles are measured from horizontal.
- 6) Plan view shown on Boring Location Plan sheet 2 of 2.

DIST COUNTY 710 ΙΔ MICHAEL A. SALISBURY No. CEG 2462 EXP. 2/28/2011 CERTIFIED ENGINEERING GEOLOGIST PLANS APPROVAL DATE The State of California or its officers or agents\ hall not be responsible for the accuracy or completeness of electronic copies of this plan sheet



**ENGINEERING SERVICES GEOTECHNICAL SERVICES** DIVISION OF ENGINEERING SERVICES SR-710 TUNNEL TECHNICAL STUDY STATE OF N/A STRUCTURE DESIGN UNCTIONAL SUPERVISOR FIELD INVESTIGATION BY: M. SALISBURY DRAWN BY: K. REYES CALIFORNIA POST MILES LOG OF TEST BORING SHEET 4 OF 5 AME: SHIVA KARIMI DATE: 3/11/09 - 3/25/09 DEPARTMENT OF TRANSPORTATION **DESIGN BRANCH** J. PRATT CHECKED BY: N/A ORIGINAL SCALE IN INCHES FOR REDUCED PLANS DISREGARD PRINTS BEARING EARLIER REVISION DATES OGS CIVIL LOG OF TEST BORINGS SHEET E A 07-187900 FILE => \$REQUEST

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DIST COUNTY ROUTE POST MILES SHEET TOTAL PROJECT NO SHEETS

7 LA 710 N/A

CERTIFIED ENGINEERING GEOLOGIST DATE

PLANS APPROVAL DATE

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TOTAL PROJECT NO SHEET TOTAL SHEET TOTAL SHEETS

NO SHEET SHEETS

MICHAEL A. SAL ISBURY
NO. CEG 2462

EXP. 2/28/2011

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ENGINEERING SHEET TOTAL

SAL ISBURY
NO. CEG 2462

EXP. 2/28/2011

CCRTIFIED

ENGINEERING SHEET TOTAL

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NO. CEG 2462

FOR COLORS

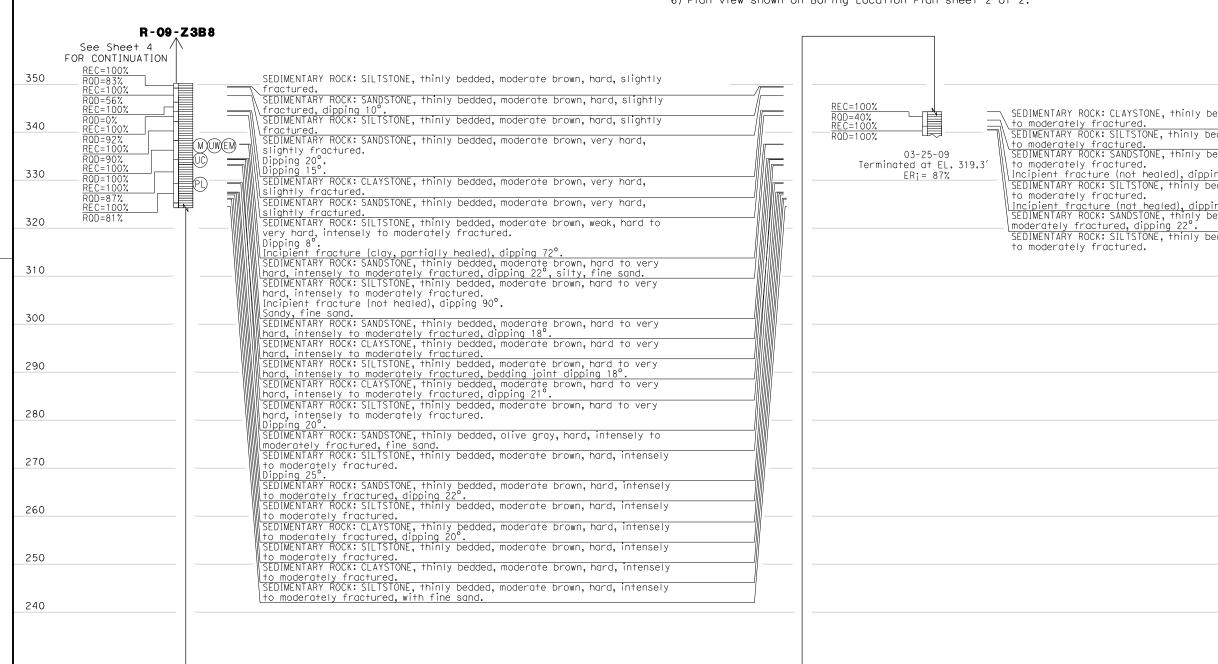
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TOTAL PROJECT SHEET SHEETS

TOTAL PROJECT SHEET SHEETS

TOTAL PROJECT SHEET



SEDIMENTARY ROCK: CLAYSTONE, thinly bedded, moderate brown, hard, intensely to moderately fractured.

SEDIMENTARY ROCK: SILTSTONE, thinly bedded, moderate brown, hard, intensely to moderately fractured.

SEDIMENTARY ROCK: SANDSTONE, thinly bedded, moderate brown, hard, intensely to moderately fractured.

SEDIMENTARY ROCK: SANDSTONE, thinly bedded, moderate brown, hard, intensely to moderately fractured.

Incipient fracture (not healed), dipping 90°.

SEDIMENTARY ROCK: SANDSTONE, thinly bedded, moderate brown, hard, intensely to moderately fractured, dipping 20°.

SEDIMENTARY ROCK: SANDSTONE, thinly bedded, olive gray, hard, intensely to moderately fractured, dipping 22°.

SEDIMENTARY ROCK: SILTSTONE, thinly bedded, moderate brown, hard, intensely to moderately fractured.

PROFILE
Vert: 1"-10'

280

ENGINEERING SERVIC	ES	GEOTECHNICAL SERVICES	STATE OF	DIVISION OF ENGINEERING SERVICES	BRIDGE NO.	SR-710	TUNNE	L TECHNICAL	STUDY	USE
FUNCTIONAL SUPERVISOR	DRAWN BY: K. REYES	FIELD INVESTIGATION BY: M. SALISBURY	CALIFORNIA	STRUCTURE DESIGN	N/A POST MILES					—————————————————————————————————————
NAME: SHIVA KARIMI	CHECKED BY: J. PRATT	DATE: 3/11/09 - 3/25/09	DEPARTMENT OF TRANSPORTATION	DESIGN BRANCH	N/A	LOG OF	TEST I	BORING SHEET	5 OF 5	AME
OGS CIVIL LOG OF TEST BORINGS SHEET		ORIGINAL SCALE IN INCHES FOR REDUCED PLANS	0 1 2 3	CU EA 07-187900	DISREGARD PR EARLIER REVIS	INTS BEARING SION DATES	-	REVISION DATES	5 5	USE RN.

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FILE => \$REQUEST

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DIST COUNTY ROUTE POST MILES SHEET TOTAL PROJECT

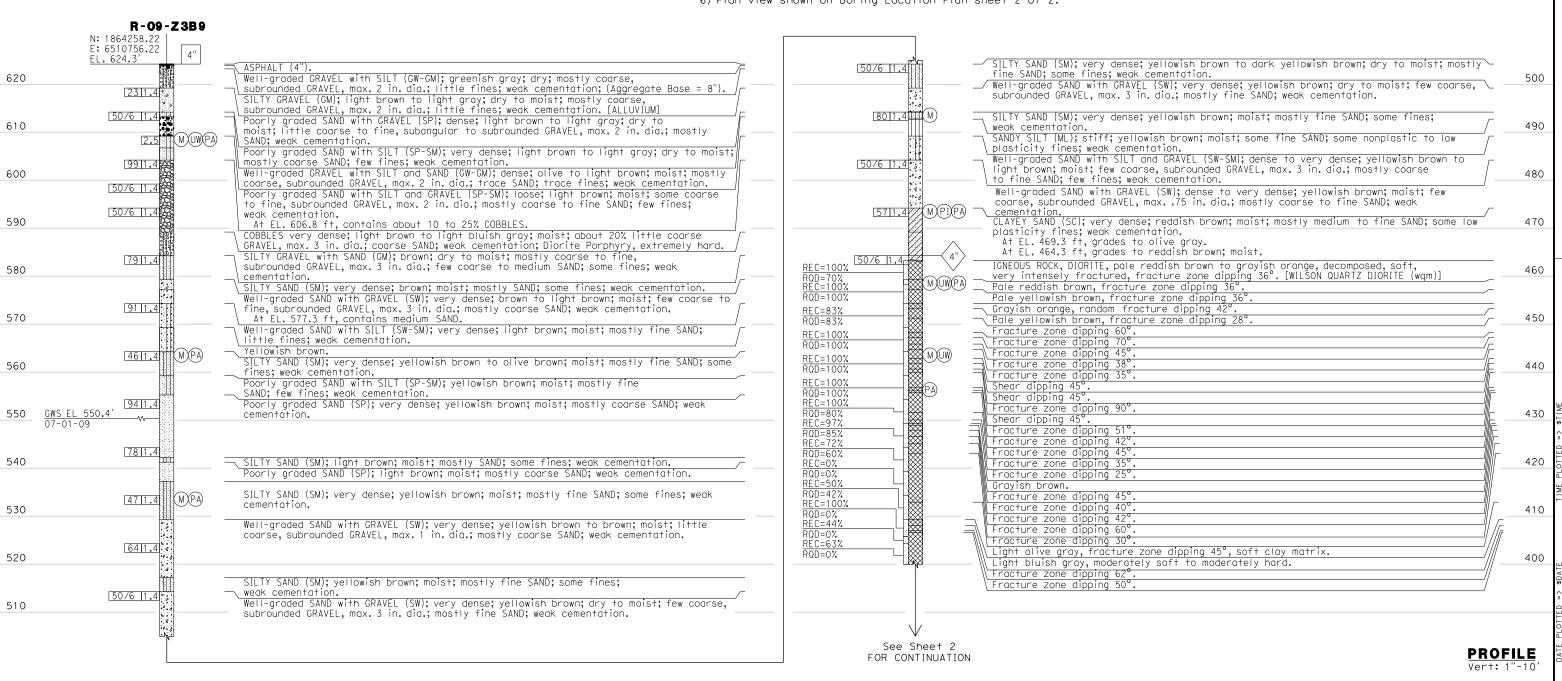
7 LA 710 N/A

3/18/10

CERTIFIED ENGINEERING GEOLOGIST DATE

PLANS APPROVAL DATE

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**ENGINEERING SERVICES GEOTECHNICAL SERVICES** DIVISION OF ENGINEERING SERVICES SR-710 TUNNEL TECHNICAL STUDY STATE OF N/A STRUCTURE DESIGN UNCTIONAL SUPERVISOR DRAWN BY: E. MIGUEL FIELD INVESTIGATION BY: M. SALISBURY **CALIFORNIA** POST MILES NAME: SHIVA KARIMI LOG OF TEST BORING SHEET 1 OF 2 CHECKED BY: J. PRATT DATE: 3/26/09 - 4/10/09 DEPARTMENT OF TRANSPORTATION **DESIGN BRANCH** ORIGINAL SCALE IN INCHES FOR REDUCED PLANS DISREGARD PRINTS BEARING EARLIER REVISION DATES OGS CIVIL LOG OF TEST BORINGS SHEET EA 07-187900

#### IST COUNTY **BENCHMARK:** LΑ 710 NGS Benchmark used. Pt MF36F: N1847799.06, E6512460.75, Elevation 407.29' 1) This LOTB sheet was prepared in accordance with the Caltrans Soil and CERTIFIED ENGINEERING GEOLOGIST DATE MF 36F: A 3" Brass disk stamped "Metropolitan Water District of Southern California Rock Logging, Classification and Presentation Manual (June 2007) MF 36F 1989", set in the top of the bridge curb at the northwest corner of the Hellman except as noted in Appendix A.1 of the Final Geotechnical Summary Report Avenue overcrossing of the Long Beach Freeway (I-710). 68' west of the center of the south bound lanes of the freeway, 25' north of the centerline of Hellman Avenue, 13' MICHAEL A. SALISBURY SR-710 Tunnel Technical Study Los Angeles County, California dated (April, 2010). No. CEG 2462 east of the west end of the bridge, 1'north of the north curb face. NGS PID EW9453. Exp. 2/28/2011 PLANS APPROVAL DATE 2) California ring, standard penetration test, and HQ&PQ core samplers CERTIFIED ENGINEERING GEOLOGIST The State of California or its officers or agents Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. were used to obtain soil and rock samples. shall not be responsible for the accuracy or Vertical control based on NAVD-88. completeness of electronic copies of this plan sheet Pt 0153 N1845410.50, E6509860.21, 3) An automatic trip hammer system consisting of a hammer weight of 140 lbs P+ 0617 N1858044.3, E6491094.23. falling a distance of 30" was used to advance the drive samplers. Units are in U.S. survey feet. 4) To convert from California ring sampler blow count to equivalent standard penetration test sampler blow count, for granular and cohesive soil, multiply 5) All bedding and other structural angles are measured from horizontal. 6) Plan view shown on Boring Location Plan sheet 2 of 2. R-09-Z3B9 See Sheet 1 FOR CONTINUATION 400 Fracture zone dipping 50°. Hard to very hard. Extremely hard. Very hard. 390 390 Fracture zone dipping 45°. Fracture zone dipping 45°. 380 380 REC=90% RQD=0% With soft clay matrix. 370 370 Fracture zone dipping 51 Fracture zone dipping 45°. 360 360 Fracture zone dipping 60° Fracture zone dipping 60° M)(UW)(PL) Fracture zone dipping 60°. 350 350 Fracture zone dipping 43°. Some vertical fractures. 340 340 Tracture zone dipping 45°. Moderately fractured. Fracture zone dipping 45 REC=100% ROD=0% REC=100% RQD=0% 330 Fracture zone dipping 90°. 330 X M (UW)(PL) Moderately hard. 04-10-09 Terminated at EL. 324.3' ER: = 87% 320 320 **PROFILE** DIVISION OF ENGINEERING SERVICES STRUCTURE DESIGN **ENGINEERING SERVICES GEOTECHNICAL SERVICES** SR-710 TUNNEL TECHNICAL STUDY STATE OF N/A FUNCTIONAL SUPERVISOR DRAWN BY: F. MIGUEL FIELD INVESTIGATION BY: M. SALISBURY **CALIFORNIA** POST MILES LOG OF TEST BORING SHEET 2 OF 2 NAME: SHIVA KARIMI CHECKED BY: J. PRATT DATE: 3/26/09 - 4/10/09 DEPARTMENT OF TRANSPORTATION **DESIGN BRANCH**

ORIGINAL SCALE IN INCHES FOR REDUCED PLANS

OGS CIVIL LOG OF TEST BORINGS SHEET

EA 07-187900 | FILE => \$REQUEST DISREGARD PRINTS BEARING EARLIER REVISION DATES

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**ENGINEERING SERVICES** 

DRAWN BY: E. MIGUEL

CHECKED BY:

J. PRATT

UNCTIONAL SUPERVISOR

AME: SHIVA KARIMI

OGS CIVIL LOG OF TEST BORINGS SHEET

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DIVISION OF ENGINEERING SERVICES STRUCTURE DESIGN

**DESIGN BRANCH** 

EA 07-187900

N/A

POST MILES

DISREGARD PRINTS BEARING EARLIER REVISION DATES

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DIST COUNTY ROUTE POST MILES SHEET TOTAL PROJECT NO N/A

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A 710 N/A

3/18/10

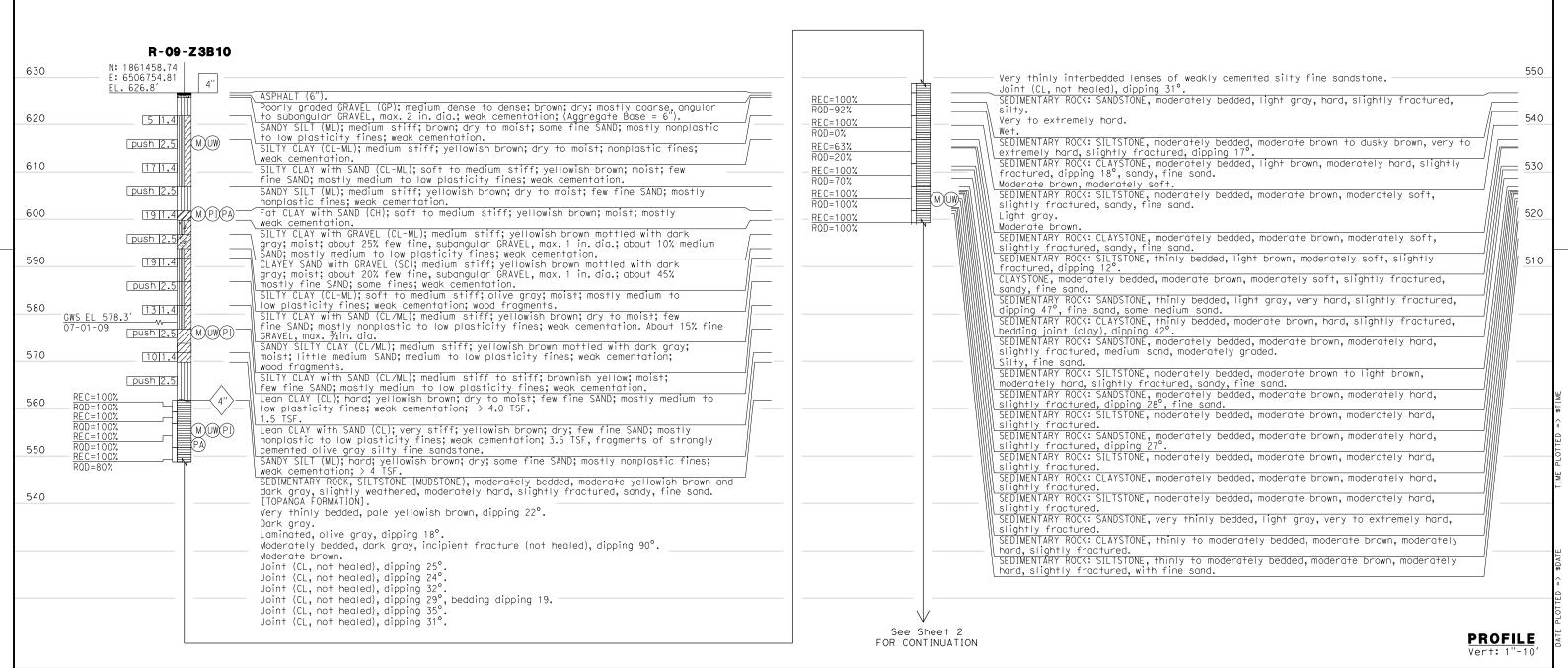
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SR-710 TUNNEL TECHNICAL STUDY

LOG OF TEST BORING SHEET 1 OF 6



STATE OF

CALIFORNIA

DEPARTMENT OF TRANSPORTATION

**GEOTECHNICAL SERVICES** 

DATE: 2/23/09 - 3/11/09

FIELD INVESTIGATION BY: M. SALISBURY

ORIGINAL SCALE IN INCHES FOR REDUCED PLANS

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**ENGINEERING SERVICES** 

DRAWN BY: F. MIGUEL

CHECKED BY:

J. PRATT

UNCTIONAL SUPERVISOR

AME: SHIVA KARIMI

OGS CIVIL LOG OF TEST BORINGS SHEET

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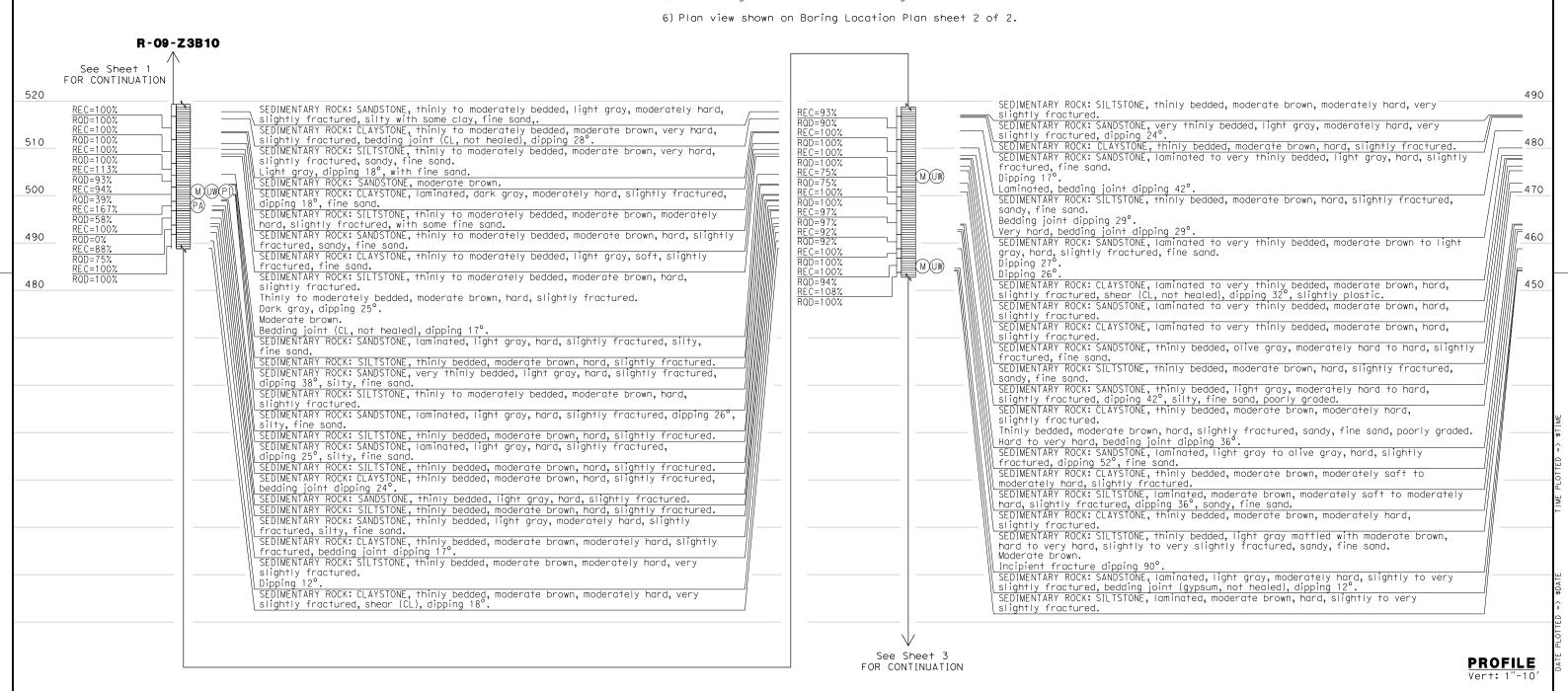
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MICHAEL A. SALISBURY
PLANS APPROVAL DATE

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SR-710 TUNNEL TECHNICAL STUDY

LOG OF TEST BORING SHEET 2 OF 6



STATE OF

CALIFORNIA

DEPARTMENT OF TRANSPORTATION

**GEOTECHNICAL SERVICES** 

DATE: 2/23/09 - 3/11/09

FIELD INVESTIGATION BY: M. SALISBURY

ORIGINAL SCALE IN INCHES FOR REDUCED PLANS

DIVISION OF ENGINEERING SERVICES

STRUCTURE DESIGN

**DESIGN BRANCH** 

N/A

POST MILES

DISREGARD PRINTS BEARING EARLIER REVISION DATES

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See Sheet 4

FOR CONTINUATION

FILE => \$REQUEST

- 5) All bedding and other structural angles are measured from horizontal.
- 6) Plan view shown on Boring Location Plan sheet 2 of 2.

PROFILE



very slightly fractured, with some fine sand.  RDD=26%; REC=114%; RDD=33%; REC=125%; REC=125%; REC=133%; REC=133%; REC=100%; ROD=53%; REC=100%; ROD=100%; RO		430
REC=133% ROD=53% REC=100% ROD=100%		120
Joint (CL, not healed), dipping 45°.  Joint (CL, not healed), dipping 42°.  SEDIMENTARY ROCK: CLAYSTONE, thinly bedded, moderate brown, moderately hard, intensely fractured.	///////////////////////////////////////	420
	//////////////////////////////////	44.0
SEDIMENTARY ROCK: SILTSTONE, thinly bedded, moderate brown, moderately hard, intensely fractured.  SEDIMENTARY ROCK: CLAYSTONE, thinly bedded, moderate brown, moderately hard,		410
intensely fractured.   SEDIMENTARY ROCK: SANDSTONE, thinly bedded, moderate brown to dark gray, moderatel hard, intensely fractured, dipping 27°, silty, fine sand.	///////////////////////////////////////	
SEDIMENTARY RÖCK: SILTSTONE, thinly bedded, moderate brown, hard, intensely fracture sandy, fine sand.  SEDIMENTARY ROCK: CLAYSTONE, thinly bedded, moderate brown, moderately hard, intensely fractured.		
SEDIMENTARY ROCK: SILTSTONE, thinly bedded, moderate brown, moderately hard, intensely fractured.  SEDIMENTARY ROCK: CLAYSTONE, thinly bedded, moderate brown, moderately hard,		
intensely fractured.  SEDIMENTARY ROCK: SILTSTONE, thinly bedded, moderate brown, moderately hard, intensely fractured.  SEDIMENTARY ROCK: CLAYSTONE, thinly bedded, moderate brown, moderately hard,		MF
		=> \$TIME
Dipping 60°.  SEDIMENTARY ROCK: CLAYSTONE, laminated, moderate brown, moderately hard, intensely fractured.  SEDIMENTARY ROCK: SILTSTONE, laminated, moderate brown, moderately hard,		TIME PLOTTED
intensely fractured.  SEDIMENTARY ROCK: CLAYSTONE, laminated, moderate brown, moderately hard, intensely fractured.		TIME
SEDIMENTARY ROCK: SILTSTONE, laminated, moderate brown, moderately hard, intensely fractured.  SEDIMENTARY ROCK: SANDSTONE, laminated, alive gray, moderately hard, intensely fractured, silty, fine sand.		
SEDIMENTARY ROCK: CLAYSTONE, laminated, moderate brown, moderately hard, intensely fractured.		* \$DATE
		PLOTTED => \$DATE

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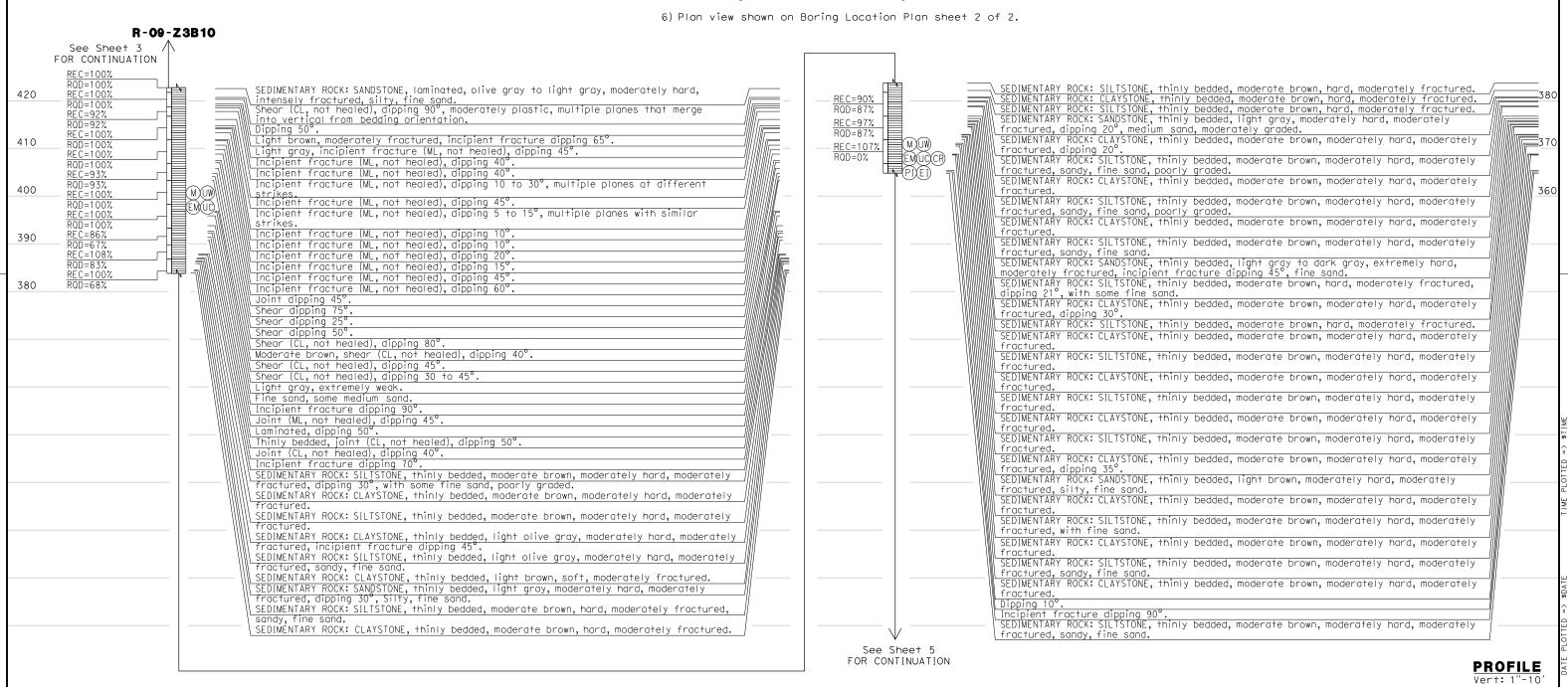
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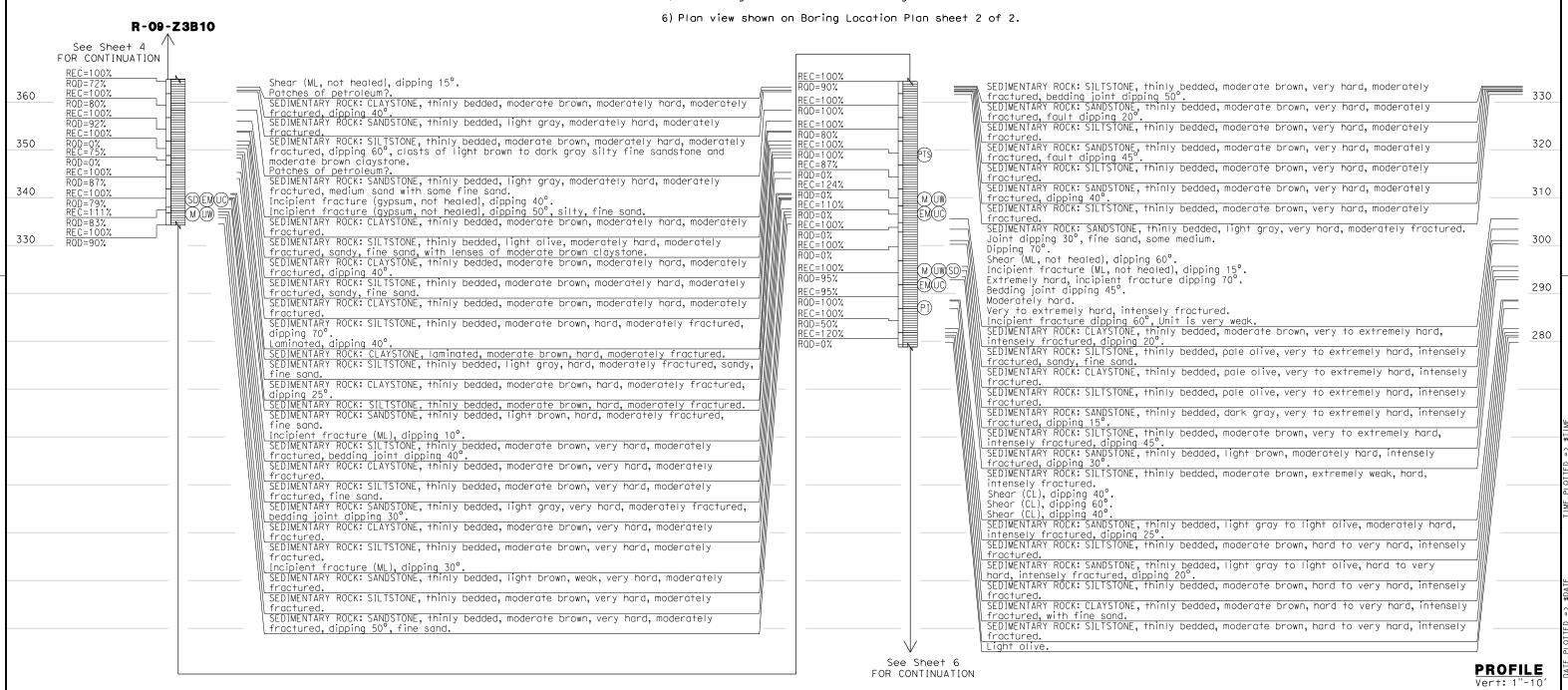
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**ENGINEERING SERVICES GEOTECHNICAL SERVICES** DIVISION OF ENGINEERING SERVICES SR-710 TUNNEL TECHNICAL STUDY STATE OF N/A STRUCTURE DESIGN UNCTIONAL SUPERVISOR DRAWN BY: F. MIGUEL FIELD INVESTIGATION BY: M. SALISBURY CALIFORNIA POST MILES LOG OF TEST BORING SHEET 5 OF 6 AME: SHIVA KARIMI DATE: 2/28/09 - 3/11/09 DEPARTMENT OF TRANSPORTATION **DESIGN BRANCH** J. PRATT CHECKED BY: ORIGINAL SCALE IN INCHES FOR REDUCED PLANS DISREGARD PRINTS BEARING EARLIER REVISION DATES OGS CIVIL LOG OF TEST BORINGS SHEET EA 07-187900

#### **BENCHMARK:** LΑ 710 NGS Benchmark used. Pt MF36F: N1847799.06, E6512460.75, Elevation 407.29' 1) This LOTB sheet was prepared in accordance with the Caltrans Soil and MF 36F: A 3" Brass disk stamped "Metropolitan Water District of Southern California Rock Logging, Classification and Presentation Manual (June 2007) CERTIFIED ENGINEERING GEOLOGIST MF 36F 1989", set in the top of the bridge curb at the northwest corner of the Hellman except as noted in Appendix A.1 of the Final Geotechnical Summary Report Avenue overcrossing of the Long Beach Freeway (I-710). 68' west of the center of the south bound lanes of the freeway, 25' north of the centerline of Hellman Avenue, 13' east of the west end of the bridge, 1' north of the north curb face. NGS PID EW9453. SR-710 Tunnel Technical Study Los Angeles County, California MICHAEL A. SALISBURY dated (April, 2010). No. CEG 2462 Exp. 2/28/2011 PLANS APPROVAL DATE 2) California ring, standard penetration test, and HQ&PQ core samplers CERTIFIED ENGINEERING GEOLOGIST The State of California or its officers or agents Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. were used to obtain soil and rock samples. shall not be responsible for the accuracy or Vertical control based on NAVD-88. completeness of electronic copies of this plan sheet Pt 0153 N1845410.50, E6509860.21, 3) An automatic trip hammer system consisting of a hammer weight of 140 lbs falling a distance of 30" was used to advance the drive samplers. P+ 0617 N1858044.3, E6491094.23. Units are in U.S. survey feet. 4) To convert from California ring sampler blow count to equivalent standard penetration test sampler blow count, for granular and cohesive soil, multiply 5) All bedding and other structural angles are measured from horizontal. 6) Plan view shown on Boring Location Plan sheet 2 of 2. R-09-Z3B10 See Sheet 5 FOR CONTINUATION 280 SEDIMENTARY ROCK: CLAYSTONE, thinly bedded, light olive mottled with dark greenish gray, hard to very hard, intensely fractured, dipping 12°, with fine sand. SEDIMENTARY ROCK: SILTSTONE, thinly bedded, light olive mottled with dark greenish gray, 270 hard to very hard, intensely fractured. (M)(W) Light olive. Moderately soft to moderately hard, shear (CL), dipping 50°. EMUC) Hard. Very hard. 260 Sandy, medium sand with some fine sand. Light gray. SEDIMENTARY ROCK: SANDSTONE, thinly bedded, light gray, very weak, extremely hard, intensely fractured, silty, medium sand with some coarse sand. Moderately hard. SEDIMENTARY ROCK: CLAYSTONE, thinly bedded, moderate brown, hard, intensely fractured, dipping 15°, with fine sand. SEDIMENTARY ROCK: SANDSTONE, thinly bedded, grayish brown, very hard, intensely fractured, bedding joint dipping 50°, with silt, fine sand. 250 03-11-09 Terminated at EL. 251.8 ER;= 87% Light gray, moderately hard. SEDIMENTARY ROCK: SILTSTONE, thinly bedded, moderate brown, moderately hard, intensely 240 fractured. Incipient fracture dipping 50°. Joint (CL, not healed), dipping 45°. SEDIMENTARY ROCK: SANDSTONE, thinly bedded, light gray, extremely hard, intensely fractured, dipping 20°, fine sand. SEDIMENTARY ROCK: SILTSTONE, thinly bedded, moderate brown to light olive, extremely hard, intensely fractured, dipping 40°, sandy, fine sand. Dipping 25° Dipping 25°. Joint (CL, not healed), dipping 40°. Dipping 20°. **PROFILE ENGINEERING SERVICES** DIVISION OF ENGINEERING SERVICES STRUCTURE DESIGN **GEOTECHNICAL SERVICES** SR-710 TUNNEL TECHNICAL STUDY STATE OF N/A FUNCTIONAL SUPERVISOR DRAWN BY: F. MIGUEL FIELD INVESTIGATION BY: M. SALISBURY **CALIFORNIA** POST MILES LOG OF TEST BORING SHEET 6 OF 6 AME: SHIVA KARIMI J. PRATT DATE: 2/28/09 - 3/11/09 DEPARTMENT OF TRANSPORTATION **DESIGN BRANCH** CHECKED BY:

ORIGINAL SCALE IN INCHES FOR REDUCED PLANS

OGS CIVIL LOG OF TEST BORINGS SHEET

IST

DISREGARD PRINTS BEARING EARLIER REVISION DATES

EA 07-187900

COUNTY

NGS Benchmark used. Pt MF36F: N1847799.06, E6512460.75, Elevation 407.29' MF 36F: A 3" Brass disk stamped "Metropolitan Water District of Southern California MF 36F 1989", set in the top of the bridge curb at the northwest corner of the Hellman Avenue overcrossing of the Long Beach Freeway (I-710). 68' west of the center of the south bound lanes of the freeway, 25' north of the centerline of Hellman Avenue, 13' east of the west end of the bridge, 1' north of the north curb face. NGS PID EW9453.

Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. Vertical control based on NAVD-88. Pt 0153 N1845410.50, E6509860.21, Pt 0617 N1858044.3, E6491094.23.

Units are in U.S. survey feet.

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5) All bedding and other structural angles are measured from horizontal.

DIST COUNTY ROUTE POST MILES SHEET NO TOTAL PROJECT NO SHEETS

7 LA 710 N/A

CERTIFIED ENGINEERING GEOLOGIST

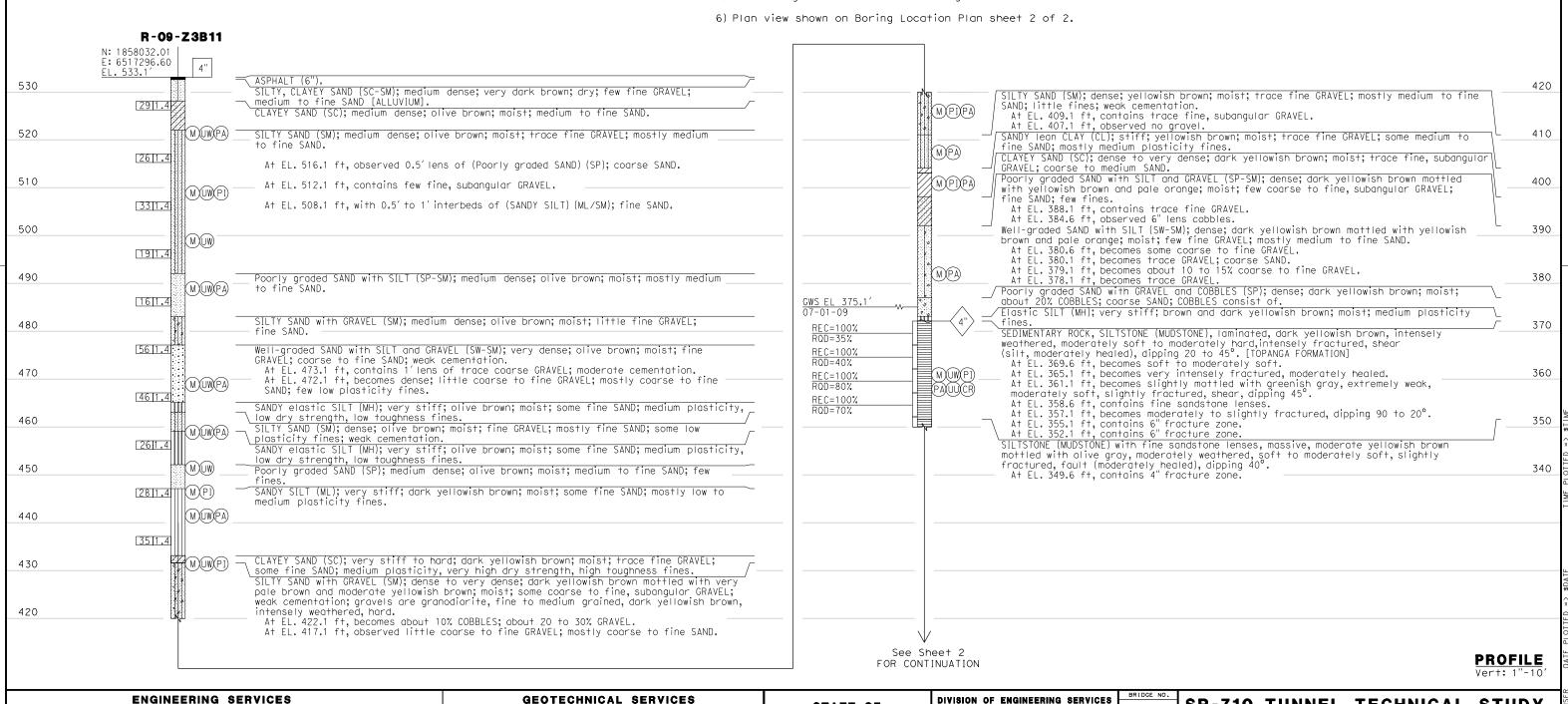
DATE

PLANS APPROVAL DATE

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**GEOTECHNICAL SERVICES** DIVISION OF ENGINEERING SERVICES SR-710 TUNNEL TECHNICAL STUDY STATE OF N/A STRUCTURE DESIGN UNCTIONAL SUPERVISOR FIELD INVESTIGATION BY: K. BARKER DRAWN BY: F. MIGUEL CALIFORNIA POST MILES NAME: SHIVA KARIMI LOG OF TEST BORING SHEET 1 OF 2 DATE: 1/13/09 - 1/21/09 DEPARTMENT OF TRANSPORTATION **DESIGN BRANCH** CHECKED BY: M. SALISBURY ORIGINAL SCALE IN INCHES FOR REDUCED PLANS DISREGARD PRINTS BEARING EARLIER REVISION DATES OGS CIVIL LOG OF TEST BORINGS SHEET EA 07-187900

# BENCHMARK: NGS Benchmark used. MF 36F: A 3" Brass d

NGS Benchmark used. Pt MF36F: N1847799.06, E6512460.75, Elevation 407.29' MF 36F: A 3" Brass disk stamped "Metropolitan Water District of Southern California MF 36F 1989", set in the top of the bridge curb at the northwest corner of the Hellman Avenue overcrossing of the Long Beach Freeway (I-710). 68' west of the center of the south bound lanes of the freeway, 25' north of the centerline of Hellman Avenue, 13' east of the west end of the bridge, 1' north of the north curb face. NGS PID EW9453.

Horizontal coordinates are CCS NAD-83, Zone 5, 1991.35 epoch. Vertical control based on NAVD-88. Pt 0153 N1845410.50, E6509860.21, Pt 0617 N1858044.3, E6491094.23.

Units are in U.S. survey feet.

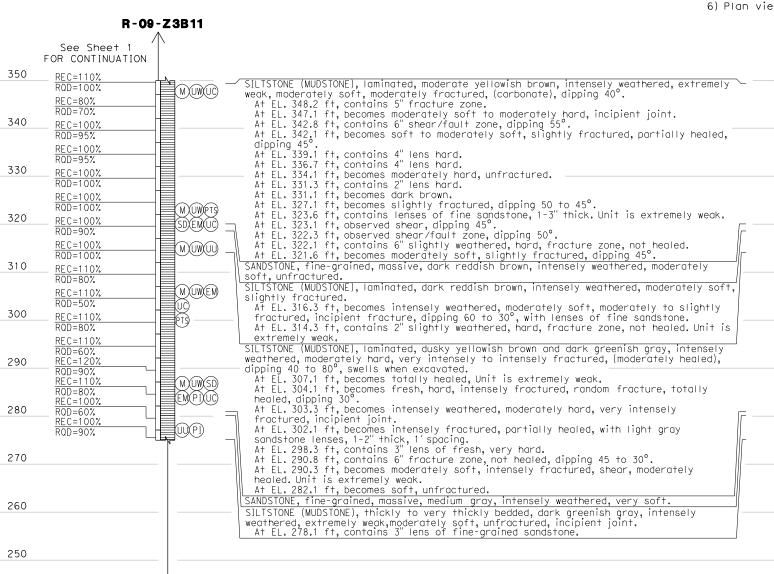
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- 6) Plan view shown on Boring Location Plan sheet 2 of 2.

COUNTY POST MILES IST ROUTE LΑ 710 N/A Barker Kristoph 3/18/10 CERTIFIED ENGINEERING GEOLOGIST DATE KRISTOPHER P. No. CEG 2383 Exp. 8/31/2011 CERTIFIED ENGINEERING GEOLOGIST PLANS APPROVAL DATE The State of California or its officers or agents shall not be responsible for the accuracy or OF CALIFOR completeness of electronic copies of this plan sheet.



SANDSTONE, fine-grained, massive, light gray, slightly weathered, hard, unfractured. SILTSTONE (MUDSTONE), dark greenish gray, extremely weak, intensely weathered, 270 moderately soft. M)(UW)(UC) RQD=80% SANDSTONE, fine-grained, light gray, fresh, very hard. **€**M) SILTSTONE (MUDSTONE), laminated, dark greenish gray, intensely weathered, extremely weak, moderately soft, very intensely fractured, shear (totally healed), with fine-grained REC=120% 260 REC=138% RQD=100% sandstone laminátions. At EL. 272.1 ft, contains 3" lens of fracture zone. At EL. 271.9 ft, becomes extremely weak, soft, slightly fractured, moderately healed, dipping 45 to 0°.

At EL. 267.1 ft, becomes moderately soft, very intensely fractured, moderately healed, dipping 45°.

At EL. 262.6 ft, becomes moderately hard. 01-21-09 Terminated at EL. 258.1 ER; = 87% 250 At EL. 262.1 ft, becomes very intensely fractured, totally healed.

PROFILE
Vert: 1"-10'

ENGINEERING S	SERVICES	GEOTECHNICAL SERVICES	STATE OF	DIVISION OF ENGINEERING SERVICES	BRIDGE NO.	SR-710	TUNNEL TECHNICAL	STUDY
FUNCTIONAL SUPERVISOR	DRAWN BY: E. MIGUEL	FIELD INVESTIGATION BY: K. BARKER	CALIFORNIA	STRUCTURE DESIGN	POST MILES			
NAME: SHIVA KARIMI	CHECKED BY: M. SALISBURY	DATE: 1/13/09 - 1/21/09	DEPARTMENT OF TRANSPORTATION	DESIGN BRANCH	N/A	LOG OF	TEST BORING SHEET	2 OF 2
OGS CIVIL LOG OF TEST BORINGS SHEET		ORIGINAL SCALE IN INCHES FOR REDUCED PLANS	0 1 2 3	CU EA 07-187900	DISREGARD PRI EARLIER REVIS	NTS BEARING ION DATES	REVISION DATES	5HEET OF 2

DESIGN OVERSIGHT ENGINEER

OGS CIVIL LOG OF TEST BORINGS SHEET

SIGN OFF DATE

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MAHESWARAN RAVEENDRA PROJECT ENGINEER

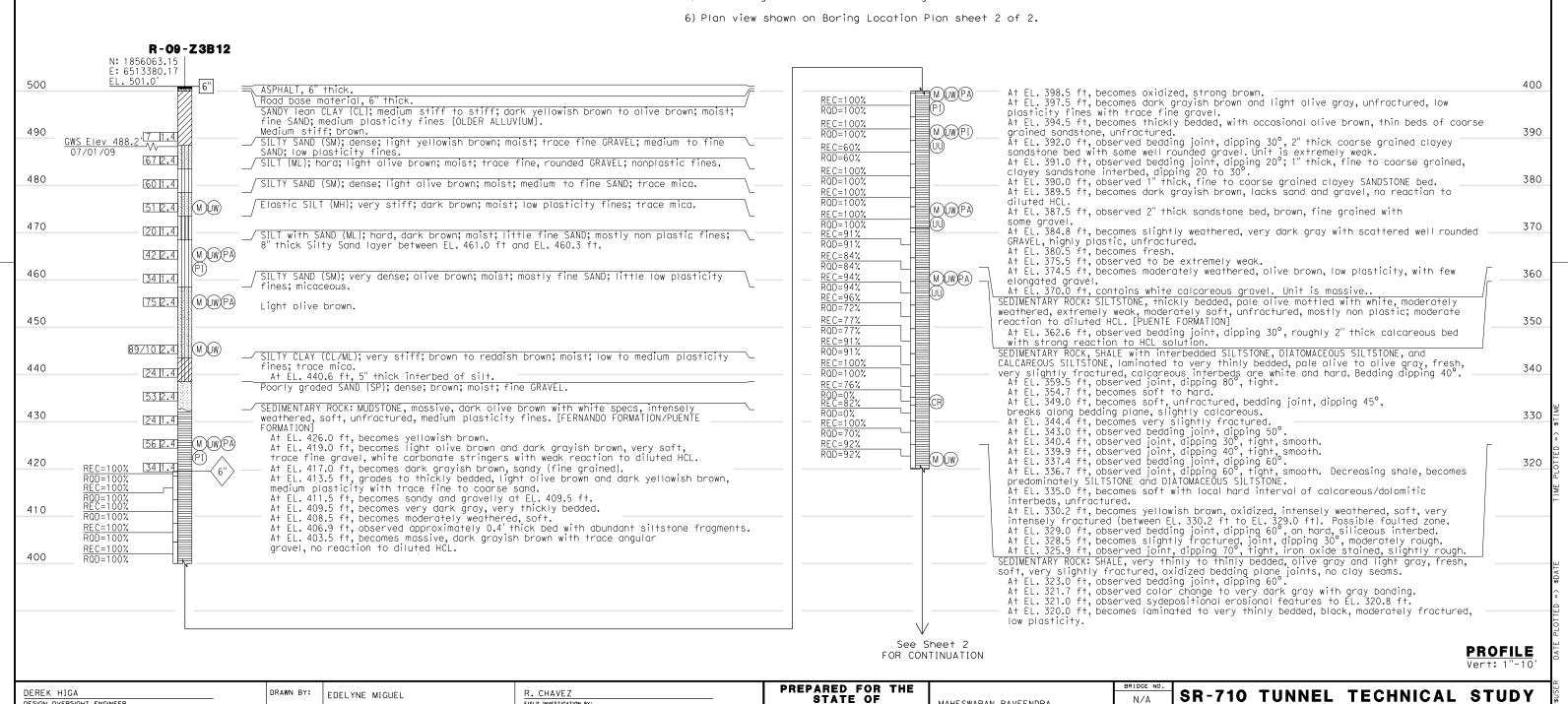
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DISREGARD PRINTS BEARING EARLIER REVISION DATES

LOG OF TEST BORING SHEET 1 OF 2

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COUNTY 710 ΙΑ 3/18/10 GEOTECHNICAL ENGINEER DATE MAHESWARAN RAVEENDRA No. GE 2743 Exp. 06-30-11 PLANS APPROVAL DATE he State of California or its officers or agents hall not be responsible for the accuracy or completeness of electronic copies of this plan sheet. CH2M HILL 6 HUTTON CENTRE DRIVE, SUITE 700 SANTA ANA, CA 92707



CALIFORNIA

DEPARTMENT OF TRANSPORTATION

FIELD INVESTIGATION BY

DATE: 1/06/09 - 1/13/09

ORIGINAL SCALE IN INCHES

DAN JANKLY

DEREK HIGA

SIGN OFF DATE

DESIGN OVERSIGHT ENGINEER

OGS CIVIL LOG OF TEST BORINGS SHEET

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DRAWN BY:

CHECKED BY:

EDELYNE MIGUEL

DAN JANKLY

R. CHAVE 7

FIELD INVESTIGATION BY

DATE: 1/06/09 - 1/13/09

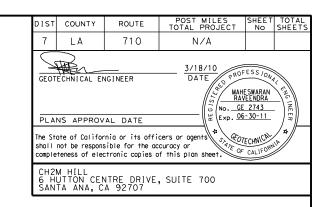
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SR-710 TUNNEL TECHNICAL STUDY

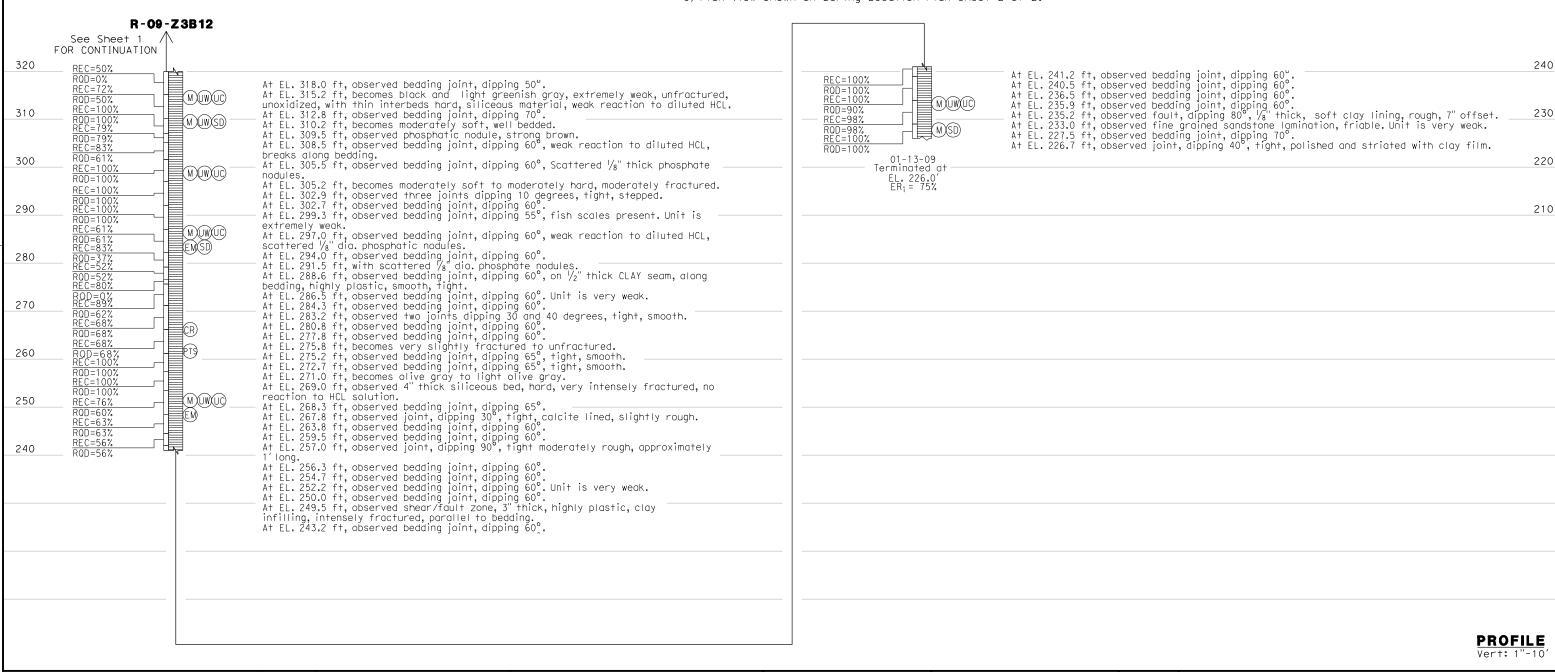
LOG OF TEST BORING SHEET 2 OF 2

N/A

POST MILES

N/A

DISREGARD PRINTS BEARING EARLIER REVISION DATES



PREPARED FOR THE

MAHESWARAN RAVEENDRA PROJECT ENGINEER

EA 07-187900

STATE OF

**CALIFORNIA** 

DEPARTMENT OF TRANSPORTATION

DESIGN OVERSIGHT ENGINEER

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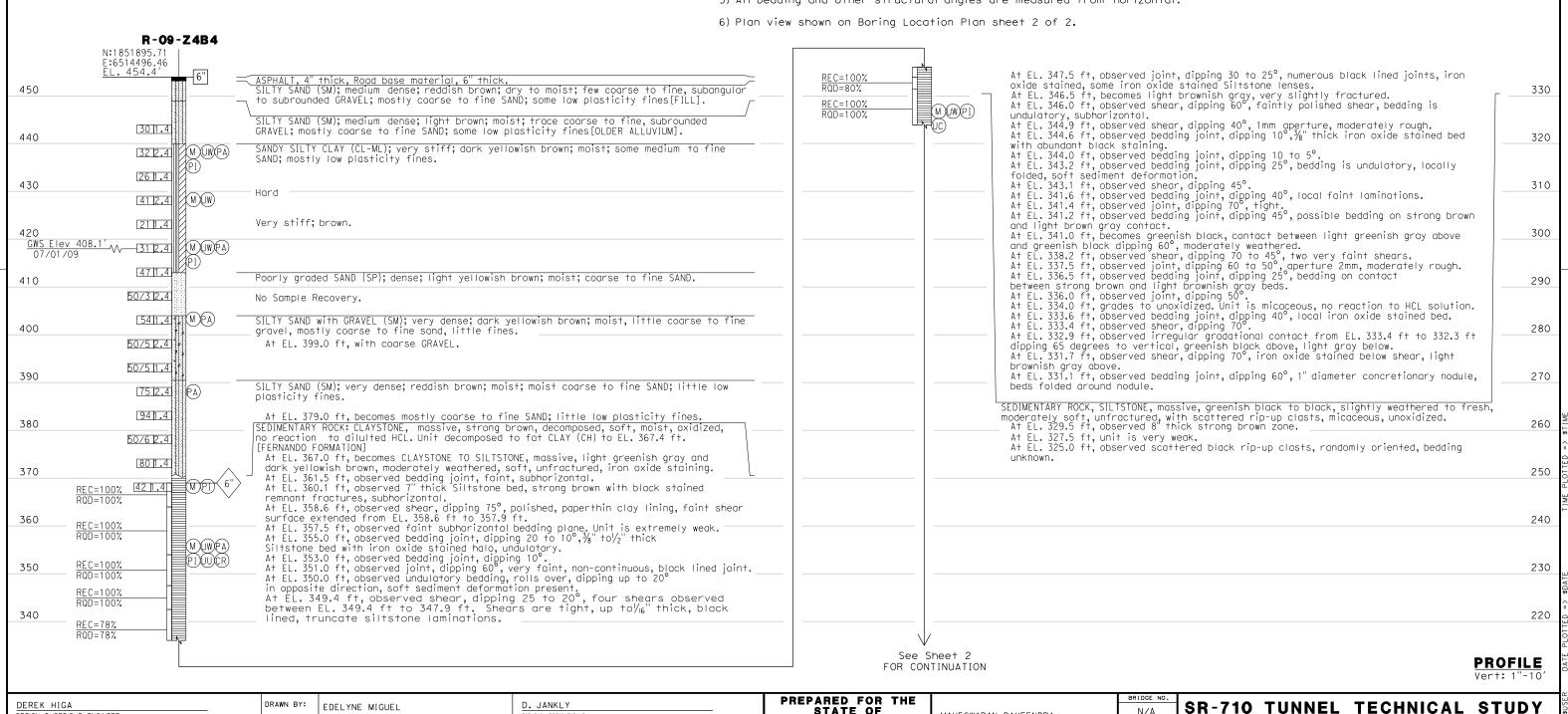
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COUNTY 710 ΙΑ 3/18/10 GEOTECHNICAL ENGINEER DATE MAHESWARAN RAVEENDRA No. GE 2743 Exp. 06-30-11 PLANS APPROVAL DATE The State of California or its officers or agents & OTECHNICAL hall not be responsible for the accuracy or completeness of electronic copies of this plan sheet. CH2M HILL 6 HUTTON CENTRE DRIVE, SUITE 700 SANTA ANA, CA 92707



STATE OF

CALIFORNIA

DEPARTMENT OF TRANSPORTATION

FIELD INVESTIGATION BY

DATE: 3/10/09 - 3/16/09

ORIGINAL SCALE IN INCHES

DAN JANKLY

MAHESWARAN RAVEENDRA PROJECT ENGINEER

EA 07-187900 FILE => \$REQUEST N/A

POST MILES

DISREGARD PRINTS BEARING EARLIER REVISION DATES

LOG OF TEST BORING SHEET 1 OF 2

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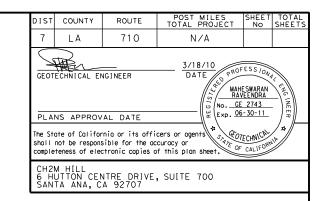
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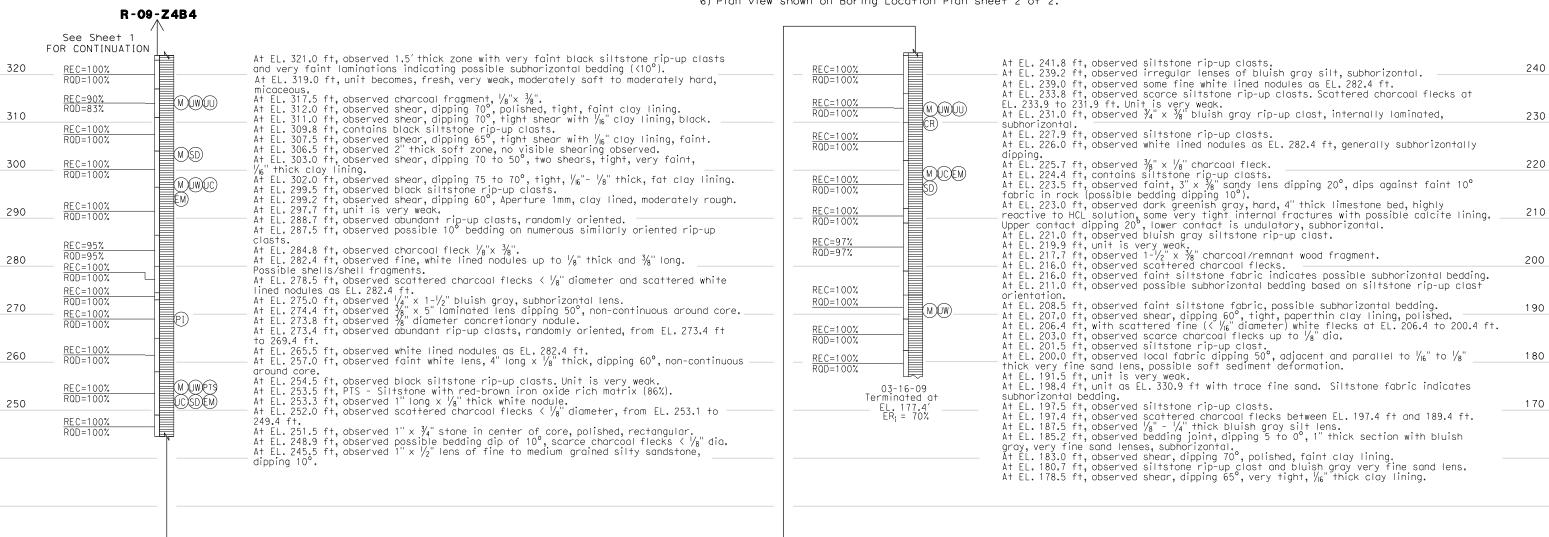
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**PROFILE** Vert: 1

DEREK HIGA DESIGN OVERSIGHT ENGINEER	DRAWN BY:	EDELYNE MIGUEL	D. JANKLY  FIELD INVESTIGATION BY:	PREPARED FOR THE STATE OF	MAHESWARAN RAVEENDRA	BRIDGE NO.	SR-710	TUNNEL TECHNICAL	STUDY
SIGN OFF DATE	CHECKED BY:	DAN JANKLY	DATE: 3/10/09 - 3/16/09	CALIFORNIA DEPARTMENT OF TRANSPORTATION	PROJECT ENGINEER	POST MILES	LOG OF	TEST BORING SHEET	2 OF 2
OGS CIVIL LOG OF TEST BORINGS SHEET			ORIGINAL SCALE IN INCHES FOR REDUCED PLANS	0   2 3	CU FA 07-187900	DISREGARD PR EARLIER REVI	INTS BEARING SION DATES	REVISION DATES	SHEET OF 2 2