### ALAMEDA CORRIDOR

### PRE-CONCEPT ESTIMATE

# SEPTEMBER 12, 1991

### PRESENTATION PACKET

# DMJM/M&N

HE 554 .A62 P33

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#### PRE-CONCEPT ESTIMATE

#### **BASIS OF ESTIMATE**

The quantity and cost estimates presented are based on:

- 1. 1'' = 100' scale aerial mapping, dated June 26, 1990
- 2. All new construction, including new railway equipment for tracks, ballasts, ties, etc.
- 3. All new roadway construction, including pavement, curbing, drainage, and sidewalks, etc.
- 4. Utility locations based on a data research of existing records.
- 5. Average unit prices in 1991 dollars published by Caltrans and from bid tabulations of local contract lettings.
- 6. Graphic layouts of roadway and railway alternative solutions, plotted on 1''=100' scale maps.
- 7. Preliminary geotechnical information available from historical records and some recent investigations as summarized in a report, dated February 22, 1991.

1

8. Conceptual level right-of-way pricing.

### AT-GRADE ALTERNATIVE

### COST COMPARISON

# (\$MILLION)

	1991 PRE-CONCEPT	1989 INITIAL COST
CONSTRUCTION, RIGHT-OF-WAY, AND UTILITY RELOCATION (INCLUDING CONTINGENCY)	\$891	\$430
MANAGEMENT, ADMINISTRATION,	126	72
ENGINEERING, CONSTRUCTION MANAGEMENT	· ·	
SUBTOTAL:	\$1,017	\$502
FINANCE AND LEGAL	61	·

 PROJECT RESERVE
 108
 - 

 SUBTOTAL:
 \$1,186
 \$502

ESCALATION	403	
TOTAL PROJECT:	\$1,589	NA 2

27609

### AT-GRADE ALTERNATIVE

FEB 2 8 2001

### **COST COMPARISON**

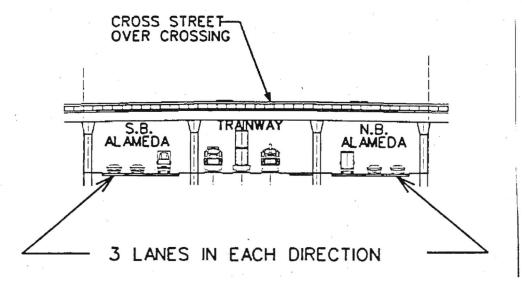
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(\$ MILLION)

	FEATURES	1991 PRE-CONCEPT	1989 INITIAL COST	FEATURES
	15 Mi. ROADWAY	\$81	\$60	12 Mi. ROADWAY
	29 GRADE SEPARATIONS (41 STRUCTURES HIGHWAY AND RAIL ESTIMATED)	\$391	\$250	16 GRADE SEPARATIONS (16 STRUCTURES ESTIMATED)
	23 Mi. MAINLINE TRACKAGE (INCLUDES REDONDO JUNCTION GRAD SEPARATION)	\$101 E	\$120	23 Mi. MAINLINE TRACKAGE (INCLUDES REDONDO JUNCTION GRADE SEPARATION)
	TOTAL CONSTRUCTION	l \$573	\$430	TOTAL CONSTRUCTION
	UTILITY RELOCATION	I \$58		UTILITY RELOCATION
	RIGHT-OF-WAY*	\$260 *	**	RIGHT-OF-WAY
5	TOTAL CONSTRUCTION, RIGHT-OF-WAY, AND UTILITY RELOCATION	\$891	\$430	TOTAL CONSTRUCTION AND RIGHT-OF-WAY

\* EXCLUDING RAILROAD R/W ALONG ALAMEDA ALL COSTS INCLUDE CONTINGENCY.

\*\* INCLUDED IN COST OF GRADE SEPARATIONS.



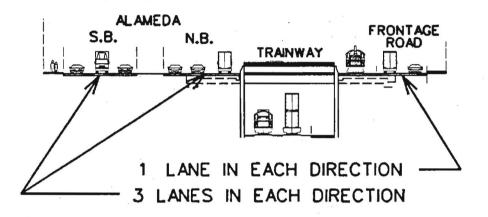
PROJECT COSTS (\$MILLION)

ALTERNATIVE 1 AT-GRADE TRAINWAY -6 LANE ALAMEDA

ALTERNATIVE 5 SAME TRAINWAY -4 LANE ALAMEDA \$1,589

\$1,580

### PROJECT COSTS (MILLION)

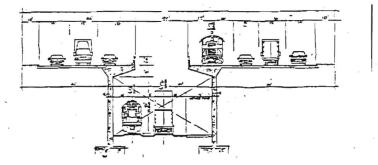


ALTERNATIVE 2.1A DEPRESSED TRAINWAY -6 LANE ALAMEDA

\$1,979

4A

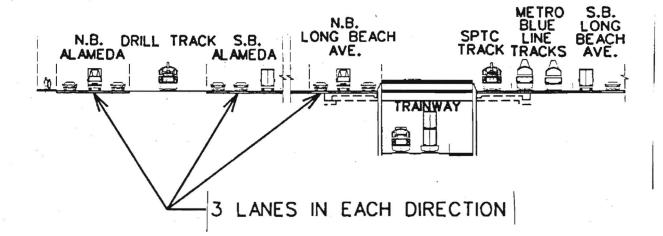
### PROJECT COSTS (\$ MILLION)



ALTERNATIVE 2.1 DEPRESSED TRAINWAY TERMINATING AT 91 FREEWAY -6 LANE ALAMEDA

ALTERNATIVE 6.1 SAME TRAINWAY -4 LANE ALAMEDA \$2,184

\$1,960



PROJECT COSTS (MILLION)

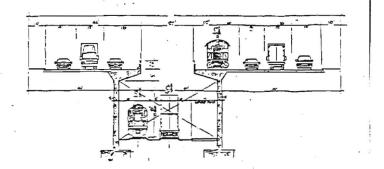
ALTERNATIVE 2.2 DEPRESSED TRAINWAY WILMINGTON DIVERSION THROUGH THE CITY OF VERNON – 6 LANE ALAMEDA

\$2,041

ALTERNATIVE 6.2 SAME TRAINWAY -4 LANE ALAMEDA

\$1,963

PROJECT COSTS (\$ MILLION)



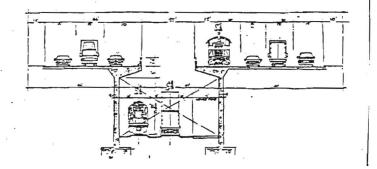
# ALTERNATIVE 2.3 DEPRESSED TRAINWAY TERMINATING AT N/O ROSECRANS – 6 LANE ALAMEDA

\$2,087

ALTERNATIVE 6.3 SAME TRAINWAY -4 LANE ALAMEDA

\$1,873





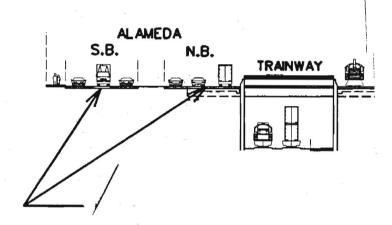
ALTERNATIVE 2.4 DEPRESSED TRAINWAY TERMINATING AT N/0 FIRESTONE -6 LANE ALAMEDA

\$1,791

ALTERNATIVE 6.4 SAME TRAINWAY -4 LANE ALAMEDA

\$1,643

### PROJECT COSTS (MILLION)



ALTERNATIVE 3.0 DEPRESSED TRAINWAY -EAST SIDE ALAMEDA 6 LANE ALAMEDA

\$2,582

6B

#### DEFINITIONS

**Project Cost** 

#### Contingency (20% Allowance)

Management, Administration, Engineering and Construction Management (20% Allowance)

Financing and Legal Costs (6% Allowance)

All inclusive costs for implementing the project projected to the midpoint of construction.

A percentage applied to the construction cost to allow for refining construction items as a result of further engineering. As engineering progresses, construction needs will be defined and details developed which will result in more complete quantity take-offs and construction costing and the contingency will be decreased.

The cost of managing and administering the project, performing the engineering, and managing the construction. These costs include:

- Management and administrative staff
- Project control and scheduling
- Engineering for preparation of plans, specifications, and estimates
- Geotechnical investigations
- Detail survey for final design
- Construction management
- Construction inspection

• Construction survey

Construction testing

The cost associated with obtaining the project funding and legal support during the life of the project. This item includes the following:

- Cost of obtaining project funds
- Contractual legal advice
- Special legal advice
- Legal support for EIR challenges
- General legal advice

#### Project Reserve (10% Allowance)

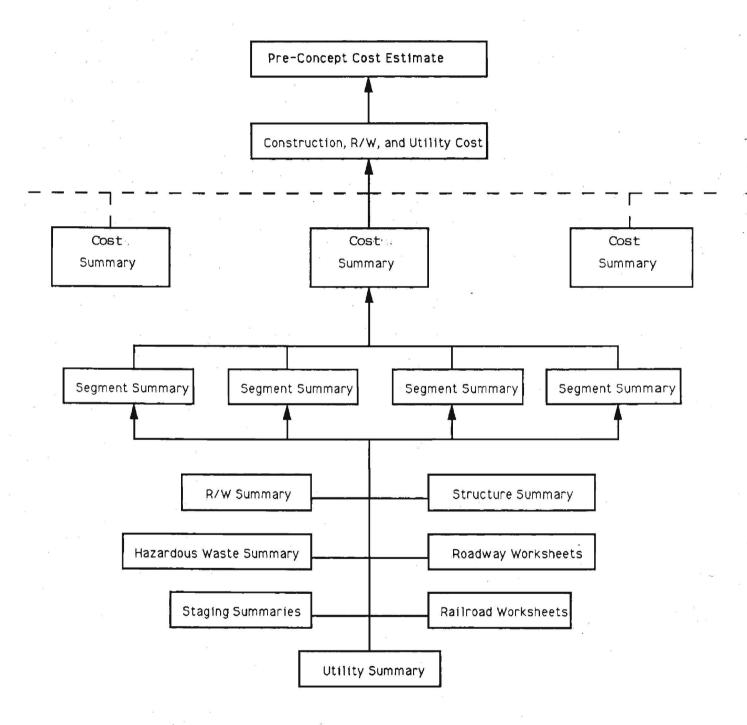
An allowance included for the cost of accommodating items required to implement the project. This item is intended to include the following:

- Permits and regulatory requirements
- Temporary construction
- Construction packaging
- Accelerating construction items
- Unknown construction requirements/cost of doing business
- Design/Construction modifications due to unknown/varying field conditions

The projection of the present day (1991) project costs to the midpoint of construction to account for inflation. The projected midpoint of construction considered was 1997 and the escalation rate used was 5 percent per year compounded annually.

Escalation (34% Factor)

# ALAMEDA CORRIDOR PRE-CONCEPT ESTIMATE Cost Estimate Model



DMJM / MN

6/06/91



### Alternate 7 Structure Estimate (Worksheet)

11 Total Alt.7 Structure Cost

Segment A Structures

\$289,371,540

Segment A Structures	\$0
Structures-East of 25th/Washington	\$64,611,000
Segment B Structures	\$56,749,780
Segment C Structures	\$71,446,860
Segment D Structures	\$96,563,900

		Location	Structure Type	Overhead Cost		Underpass Cost		Cost
			.,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		Use		Use	
	1	Redanda Junction (15 mph)	RR Grade Sep	ж. Э		\$39,800,000	1	\$39,800,000
	2	Redondo Junction (40 mph)	RR Grade Sep			\$48,300,000		
	3	Washington Blvd. (Widen)	Railroad			\$687,000	1	\$687,000
	- 4	Washington Blvd/ Sante Fe	Highway	\$13,832,000	1	\$12,034,000		\$13,832,000
	. 5	Soto St.	Railroad			\$2,150,000	1	\$2,150,000
	6	Perrino Place	Railroad			\$3,306,000	1	\$3,306,000
e.	7	Los Angeles River Bridge	Railroad			\$4,836,000	1	\$4,836,000
	- 8	N/B Alameda	Highway	Ť		\$7,048,600	1	\$7,048,600
	- 9	25th St.	Highway			\$4,134,000	1	\$4,134,000
I	- 10	Vernon Ave.	Highway	\$7,554,000		\$4,047,780	1	\$4,047,780
ł	- 11	38th/41st. St.	Highway			\$4,047,780	1	\$4,047,780
	- 12	Gage Avenue	Highway	\$6,808,200	1	\$5,182,000		\$6,808,200
1	- 13	Slauson Ave.	Highway	\$8,087,340	1	\$4,047,780		\$8,087,340
	14	Slauson Ave. (RR)**	Railroad			\$197,300		
7	15	Randolph**	Railroad			\$197,300		
	-16	Nadeau St.	Highway	\$3,712,800		\$3,944,640	1	\$3,944,640
i	~17	Florence Ave.	Highway	\$7,568,450		\$3,944,640	1	\$3,944,640
Į	-18	Northbound Cross Over	Highway			\$14,686,800	1	\$14,686,800
	- 19	Firestone	Highway	\$7,000,000		\$5,000,000	1	\$5,000,000
	- 20	Southern Ave.	Highway	\$6,810,600	1	\$6,060,600		\$6,810,600
ł	- 21	Alameda/Tweedy	Highway	\$14,416,200	1			\$14,416,200
	- 22	Imperial/Alameda	Highway			\$4,498,500	1	\$4,498,500
1								
	- 23	Weber	Highway	\$6,288,360	1	\$4,975,200		\$6,288,360
	- 24		Highway	\$5,727,360	1			\$5,727,360
1		Alondra	Highway	\$5,192,920		\$4,463,200 \$4,375,000	1	\$4,375,000
1		Compton/Ramps	Highway	\$12,370,380	1	\$4,575,000 \$4,520,100		\$12,370,380
1		Compton Creek Box Culvert	Highway/Rail	\$3,732,000	-	\$4,520,100		\$3,732,000
	27	Compton Greek Dox Current	ngnway/nan	\$3,732,000	3			\$3,732,000
\$	~ 28	Greenleaf	Highway	\$8,228,460	1			\$8,228,460
	-29	Alameda UP at Laurel Pk.	Highway			\$10,089,800	1	\$10,089,800
	30	Dominguez Channel (7A)	Railroad			\$1,255,000	1	\$1,255,000
	31	Dominguez Channel (7B)	Railroad			\$1,745,000	1	\$1,745,000
	32	Dominguez Channel (7C)	Railroad			\$775,000	1	\$775,000
	- 33	Sepulveda	Highway	\$14,575,000	1			\$14,575,000
	34	ACTA Railroad	Railroad	\$19,642,800	1			\$19,642,800 -
	35	UPRR		\$13,513,100	. 1			\$13,513,100
	- 36	Connector Road "A"	Highway	\$1,630,700	1			\$1,630,700
	~ 37	Connector Road *8*	Highway	\$1,036,800	1 -			\$1,036,800
		Connector Road *C*	Highway	\$120,000	1			\$120,000
		Henry Ford	Highway	\$1,233,000	1			\$1,233,000
	~ 40	TI Fwy On-Ramp	Highway	\$1,093,000	1			\$1,093,000
	- 41	TI Fwy Off-Ramp	Highway	\$1,081,000	1			\$1,081,000
	12	Marina RR Crossing	Railroad	\$3,690,000	1			\$3,690,000 <
	- 43	Anaheim (Reconstruct)	Highway	\$14,585,000	1			\$14,585,000
	- 44	Pacific Coast Highway	Highway	\$10,498,700	1			\$10,498,700
F				Total Struct	ures			\$289,371,540

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1-B		Segment Summary		
	ELEMENT	ITEM	ITEM COST	ELEMENT COST [\$Millions ]
	ROADWAY			\$26.8
	. Iono Minit			
		Curb and Gutter	\$1,079,650	
		Drainage System	\$481,600	
		Electroliers	\$416,000	
		Embankment	\$918,750	
		Local Circulation	\$3,195,150	
		Pavement	\$6,550,625	
		Removals	\$3,185,638	
		Sidewalk	\$818,685	
		Signal Systems	\$2,340,000	
		Signs, Stripes, Pavmt. Marking	\$464,700	
	*	Traffic Management	\$879,820	
		Mobilization	\$2,033,062	
		Contingency (20%)	\$4,472,736	
		-		
	STRUCTURES			\$162.0
		Str. East of 25/ Washington	\$64,611,000	
		Sound Walls	\$1,356,000	
		Alameda St. Structures	\$56,749,780	
			\$12,271,678	
		Contingency (20%)	\$26,997,692	
	TRAINWAY			\$39.1
	UNDING 1	Removals	\$402,700	
		Construct Subgrade	\$4,360,030	
		Construct Track	\$20,258,850	
		Turnouts (Included)	420,200,000	
		Crossovers (Included)		
		Crossing Frogs (Included)		181
		Road Crossings (Included)		
		Fence	\$950,400	
	· .	Signals	\$6,213,080	
		Operation Control	\$246,600	
	š	Security	\$123,300	
		Contingency (20%)	\$6,510,992	
	RIGHT-OF-W	AY	-	\$145.4
		No. of Parcels	173	
	÷	Land Value	\$65,266,000	
		Improvement Value	\$39,404,000	
		Demolition	\$5,233,500	
		Relocation Assistance Payments(RAP)	\$8,373,600	
		Contingency (5%)	\$5,913,855	
		Administration (10%)	\$12,419,096	8
		R/W Engr. (8% of Demo and Acquisition)	\$8,792,280	
		OCATION	· · · · · · · · · · · · · · · · · · ·	
	UTILITY RELO	Contingency (20%)		\$8.9

ALAMEDA CORRIDOR PRE-CONCEPT COST ESTIMATE

### **COST SUMMARY FOR ALTERNATE 1**

#### AT GRADE TRAINWAY ALONG ALAMEDA STREET

#### ALAMEDA-ONE WAY COUPLET (3 LANES EACH DIRECTION) FROM 25TH STREET TO FIRESTONE BLVD. ALAMEDA- 6 LANES WEST SIDE FROM FIRESTONE TO SR91

#### [COSTS IN \$ MILLIONS]

ITEM	SEGMENT A	SEGMENT B 25th Street to 85th Street	SEGMENT C 85th Street to SR91	SEGMENT D SR 91 to Terminal Island	ALTERNATE TOTALS
ROADWAY	\$2.6	\$26.8	\$33.0	\$19.0	\$81.5
STRUCTURES	\$0.0	\$162.0	\$101.6	\$127.5	\$391.0
TRAINWAY	\$0.0	\$39.1	\$25.5	\$36.3	\$100.8
RIGHT-OF-WAY	\$7.2	\$145.4	\$62.4	\$44.6	\$259.7
UTILITY RELOCATION	\$1.2	\$8.9	\$20.8	\$26.7	\$57.7
TOTAL CONST. & R/W COST	\$11.1	\$382.2	\$243.2	\$254.1	\$890.7

ALAMEDA CORRIDOR PRE-CONCEPT COST ESTIMATE

#### **COST SUMMARY FOR ALTERNATE 2.1A** TRENCH OVERHANGS REMOVED **DEPRESSED TRAINWAY - ALAMEDA 6 LANES** [ONE WAY COUPLETS - THREE LANES EACH DIRECTION EACH WAY FROM 25TH ST. TO COMPTON BLVD.] [COST IN \$ MILLIONS] ITEM SEGMENT A SEGMENT B SEGMENT C SEGMENT D ALTERNATE TOTALS I-10 to 25th Street 25th Street to 85th Street 85th Street to SR91 SR 91 to Terminal Island \$2.6 \$14.3 \$18.7 \$19.0 \$54.7 ROADWAY \$0.0 \$292.6 \$298.6 \$127.5 \$718.6 STRUCTURES \$0.0 \$40.9 \$28.2 \$36.3 \$105.4 TRAINWAY \$7.2 \$93.3 \$10.1 \$44.6 \$155.3 RIGHT-OF-WAY \$47.2 \$1.2 \$11.1 \$8.1 \$26.7 UTILITY RELOCATION \$452.2 \$363.8 \$254.1 \$1,081.2 TOTAL CONST. & R/W COST \$11.1

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# Construction, R/W, and Utility Cost

(\$ Millions)

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Alt.	Description	Roadway	Structures	Trainway	Subtotal Construction Cost*	Utility Relocation	R/W	Construction R/W and Utility Cost
1	At-Grade Trainway- Alamedz & Lanes [One Way Couplet- North of Firestone Bivd., & Lanes West Side South of Firestone Bivd.]	\$81.5	\$391.0	\$100.8	\$573.3	\$57.7	\$259.7	\$890.7
2.1	Depressed Trainway- Alameda & Lanes [One Way Couplet from 25th St. to Compton Blvd., & Lanes West Side to SR01 Freeway]	\$54.7	\$834.9	\$105.4	\$995.0	\$47.2	\$147.1	\$1,189.2
2.1A	Alternate 2.1-Railroad Trench Wall Overhangs Deleted	\$54.7	\$718.6	\$105.4	\$878.8	\$47.2	\$155.3	\$1,081.2
2.2	Alternate 2.1-Vernon Diversion-Alamada & Lanas [Trainway Via Wilmington Branch (SPTC) North of Randolph Street]	\$58.6	\$762.3	\$105.7	\$926.7	\$53.2	\$130.3	\$1,110.2
2.3	Alternate 2.1-Trainway al-grade at Rosecrans-Alameds & Lanes [Trainway transitions from depressed to al-grade north of Rosecrans]	\$54.6	\$768.1	\$104.0	\$926.8	\$47.2	\$167.2	\$1,141.2
2.4	Alternate 2.1-Trainway at-grade at Fireatone-Alameda 6 Lanea [Trainway transitions from depressed to at-grade north of Firestone]	\$56.6	\$581.5	\$102.6	\$740.7	\$47.2	\$200.8	\$988.7
3	Depressed Trainway-Alameda & Lanea [& Lanea West Side, East Side Local and Property Access on Structures from 25th St. to 92nd St Frontage Rd. Access from 92nd St. to SR 91]	\$54.3	\$1,019.4	\$105.4	\$1,179.0	\$53.5	\$173.3	\$1,405.9

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	Constr	uction, R/\ (\$ }	N, and Ut	ility Cost	(Continued)		ь 	
	Description	Roadway	Structures	Trainway	Subtotal Construction Cost*	Utility Relocation	R/W	Construction R/W and Utility Cost
4	Alternate 2.1-Alameda 4 and 8 Lanes plus Exclusive Truck Lanes [Lanes adjacent to Depressed Trainway designated Exclusive Truckway between 25th St. and Compton Bivd. Truckway grade-separated from Cross Streets at Selected Major Intersections]	\$54.6	\$985.3	\$105.4	\$1,145.3	\$47.2	\$147.1	\$1,339.6
5	At-Grade Trainway- Alameds 4 Lanes [One Way Couplet- North of Firestone Blvd., 4 Lanes West Side South of Firestone Blvd.]	\$79.0	\$391.0	\$100.8	\$570.8	\$57.7	\$257.1	\$885.6
8.1	Alternate 2.1 Depressed Trainway- Alameda 4 Lanes [One Way Couplet from 25th St. to Compton Blvd., 4 Lanes West Side to SR91 Freeway]	\$52.5	\$718.6	\$105.4	\$876.5	\$47.2	\$146.2	\$1,069.9
6.2	Alternate 2.2-Vernon Diversion-Alameda 4 Lanes [Trainway Via Wilmington Branch (SPTC) North of Randolph Streetj	\$56.3	\$723.7	\$105.7	\$885.7	\$53.2	\$129.5	\$1,068.4
4,3	Alternate 2.3-Trainway at-grade at Rosecrans-Alameda 4 Lanes [Trainway transitions from depressed to st-grade north of Rosecrans]	\$52.4	\$656.6	\$104.0	\$813.0	\$47.2	\$166.4	\$1,026.6
6.4	Alternate 2.4-Trainway al-grade at Firestone-Alameda 4 Lanes [Trainway transitions from depressed to at-grade north of Firestone]	\$61.0	\$498.6	\$102.5	\$662.3	\$47.2	\$200.0	\$909.4

· 1991 Dollars

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Sep 02, 1991

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	Pre-C	oncept Cost (\$ Millions)	Estimate	2			
Alt.	Description	Construction R/W and Utility Cost	Engineering Const. Management Administration [20%]*	Financing and Legal Costs [6%]	Project Reserve [10%]	Project Cost [1991-\$]	Project Cost [ Escalated ]**
1	At-Grade Trainway- Alameda 6 Lanes [One Way Couplet- North of Firestone Blvd., 6 Lanes West Side South of Firestone Blvd.]	\$890.7	\$126.2	\$61.0	\$107.8	\$1,185.7	\$1,588.0
2.1	Depressed Trainway- Alameda 6 Lanes [One Way Couplet from 23th St. to Compton Bivd., 6 Lanes West Side to SR91 Freeway]	\$1,189.2	\$208.4	\$83.9	\$148.2	\$1,629.7	\$2,183.8
2.1A	Atternate 2.1-Railroad Trench Wall Overhangs Delated	\$1,081.2	\$185.2	\$76.0	\$134.2	\$1,476.6	\$1,978.7
2.2	Alternate 2.1-Vernon Diversion [Trainway Via Wilmington Branch (SPTC) North of Randolph Street]	\$1,110.2	\$196.0	\$78.4	\$138.5	\$1,523.0	\$2,040.9
2.3	Alternate 2.1-Trainway al-grade at Rosecrans [Trainway surfaces transitions from depressed to at-grade north of Rosecrans]	\$1,141.2	\$194.8	\$80.2	\$141.6	\$1,557.8	\$2,087.4
2.4	Alternate 2.1-Trainway at-grade at Fireatone-Alameda 6 Lanes [Trainway transitions from depressed to at-grade north of Fireatone]	\$988.7	\$157.6	\$68.8	\$121.5	\$1,336.6	\$1,791.0
3	Depressed Trainway-Alameda & Lanes [6 Lanes West Side, East Side Local and Property Access on Structures from 25th St. to 92nd St Frontage Rd. Access from 92nd St. to SR 91]	\$1,405.9	\$246.5	\$99.1	\$175.2	\$1,926.7	\$2,581.8

	Pre-Concept Cost Estimate (Continued) (\$ Millions)												
Alt.	Description	Construction R/W and Utility Cost	Engineering Const. Management Administration	Financing and Legal Costs [6%]	Project Reserve [10%]	Project Cost [1991-\$]	Project Cost [Escalated]**						
4	Alternate 2.1-Alameda 4 Lanes plus Exclusive Truck Lanes [Lanes adjacent to Depressed Trainway designated Exclusive Truckway between 25th St. and Compton Blvd. Truckway grade-separated from Cross Streets at Selected Major Intersections]	\$1,339.6	\$238.5	\$94.7	\$167.3	\$1,840.0	\$2,465.6						
5	At-Grade Trainway- Alameda 4 Lanes [One Way Couplet- North of Firestone Blvd., 4 Lanes West Side South of Firestone Blvd.]	\$885.6	\$125.7	\$60.7	\$107.2	\$1,179.2	\$1,580.1						
6.1	Alternate 2.1 Depressed Trainway- Alameda 4 Lanes [One Way Couplet from 25th SI. to Compton Blvd., 4 Lanes West Side to SR91 Freeway]	\$1,069.9	\$184.7	\$75.3	\$133.0	\$1,462.9	\$1,960.3						
6.2	Alternate 2.2-Vernon Diversion-Alameda 4 Lanes [Trainway Via Wilmington Branch (SPTC) North of Randolph Street]	\$1,068.4	\$187.8	\$75.4	\$133.2	\$1,464.7	\$1,962.7						
6.3	Alternate 2.3-Trainway at-grade at Rosecrana-Alameda 4 Lanes [Trainway transitions from depressed to at-grade north of Rosecrans]	\$1,026.6	\$172.0	\$71.9	\$127.1	\$1,397.7	\$1,872.9						
6.4	Alternate 2.4-Trainway at-grade at Firestone-Alemeda 4 Lanes [Trainway transitions from depressed to at-grade north of Firestone] 4 Lanes West Side to SR91 Freeway]	\$909.4	\$141.9	\$63.1	\$111.4	\$1,225.8	\$1,642.6						

\*Engr., Const. Managmt. and Admin. Cost is 20% of Construction and Utility Relocation Costs

\*\*Escalation (5% per year to FY 97)-34%

Sep 12, 1991

18

DMJMMN

APPENDIX

#### SUPPLEMENTAL GRADE SEPARATION STRUCTURES

The analysis of traffic demands and local access and circulation needs indicated that four additional grade separation structures are warranted for this project. For informational purposes, the total project costs in 1991 dollars for these grade separation structures are presented below and are included in the alternative costs presented herein.

38th/41st St.	\$29.1 million
Nadeau St.	\$16.0 million
Weber St.	\$29.0 million
Greenleaf St.	\$32.8 million

A-1

Revised 09/02/91

#### HAZARDOUS WASTE REMOVAL

#### COST ESTIMATE

Our preliminary investigations have determined that known hazardous waste sites exist on private properties along or in close proximity to the Alameda Corridor. Certain portions of these sites must be acquired for this project. Under current regulations, these sites to be acquired must be dealt with to clean-up and dispose of or treat in place to stabilize the hazardous waste. Since the issue is related to right-of-way acquisition negotiations, the costs associated with the removal of the wastes <u>have not</u> been included in the alternative project cost estimates. The following estimates are offered as potential costs to the project for waste removal.

Alternative 1

\$35 million

Alternative 2.1

\$18 million

It is important to note that the estimates have been prepared on the basis of limited research and investigation. Once an alternative design is selected, further investigation and analysis will be required to better quantify the magnitude and costs of addressing the hazardous waste issues.



October 2, 1991

Gill V. Hicks ACTA General Manager 6550 Miles Avenue, Room 113 Huntington Park, CA 90255

RE: Alameda Corridor Updated Decision Matrix Package

Dear Gill:

Enclosed for your use and distribution to the Technical Working Group are forty copies of the noted package. This information should replace the individual memorandums that were presented on September 12, 1991.

The information transmitted is identified as follows:

- 1. Three sheet decision matrix
- 2. Technical Memorandums
  - a. Traffic No. 3, 4, 5
    - b. Safety/Security No. 1 and 2
    - c. Railroad No. 1
    - d. Environmental No. 2, 3, 6, 7
    - e. Economic No. 2, 4
  - f. Cost No. 1, 3
  - g. Construction No. 1

We are looking forward to meeting with the committee on October 10, 1991 to receive their review comments on the decision matrix.

Sincerely,

DMJM Vilue endall Project Director

1 • -

	ALAMEDA CORRID	OR PROJECT GOALS		MEASURED UNIT	ALT 1.0 4	U.T. 21A	ALT. 2.2 A					UT. 61	ALT. 47	ALT 63	NT. 44
]	RAFFIC GOALS	ASSIGNED WEIGHT: 17.0	*												
– GOALS – REDUCT VEHKLE DELAYSAT	- MEASUREMENT CRITERIA -	- COMMENT -	-TYPE OF GOAL -												
REDUCE VEHICLE DELAYS AT GRADE CROSSINGS ON ALL PORT ACCESS RAIL LINES	REDUCTION IN VEHICLE HOURS		ALTERNATIVE CONCEPT GOAL	REDUCTION IN											
				VEHICLE HOURS INDEX (MAX.) MILES PERHOUR	14,505	14,130 D.97	14,130	14,344	14,219	14,130	14,508	14,130 0.97	14,130	14,344	14,219
IMPROVE NORTH/SOUTH TRAVEL SPEEDS	AVERAGE TRAVEL SPEED ALONG ALAMEDA		ALTERNATIVE CONCEPT GOAL	MILES PERHOUR INDEX (MAJC)	22.6	22.1 0.98	22.1 0.95	22.4	22.4	22.1	222.0 0.97	21.5 0.95	21.4	21.8	0 98 21 6 0 95
IMPROVELEVEL-OF-SERVICE AT INTERSECTIONS	TIME DELAY		ALTERNATIVE CONCEPT GOAL	MINUTES OF DELAY	105	50	- 58	74.8	77.8	38	105	86	51	74 8	77 8
IMPROVE CONNECTIONS TO 1-105 AND I-10 PREEWAYS	TRAVEL TIME PROMALAMEDATO	·	ALTERNATIVE CONCEPT GOAL	INDEX (MIN.) MINUTES	0.55	1.00	1.00 6.7	0.78 6.7	0.75 8.7	1.00	0.66	1.00	1.00	0.78	075
TO PARALLEL PREEWAYS	INCREASE OF VEHICLE MILES TRAVELED - PORT TRUCKS ON		ALTERNATIVE CONCEPT GOAL	INDEX (MIN.) REDUCTION IN VEHICLE MILES BY FORTS TRUCKS	0.81	1.00	1.00	1.00	1.00	0.81	1.00 9.985	1.00 9,255	1.00 9,995	1.00	1 00 8 AS2
(EMPHASIZE TRUCKS)	ALAMEDA PLUS DECREASE ON FREEWAYS ADDITIVE EMERGENCY VEHICLE		ALTERNATIVE CONCEPT GOAL	INDEX (MAX.)	1.00	0.90	0.90	10,200 0.00 8.0	0.93	0.90	0.61	0.47 9.8	0.46	0.44	0.45
ACCESS	RESPONSETIME	THIS GOAL WEL NOT DIFFER	ALIERNAINE CONCEPT GOAL	INDEX(MIN.)	0.84	1.00	1.00	1.00	0.89	1.00	084	1.00	1.00	1.00	0.89
TO RAE.		THIS GOAL WELL NOT DIFFER BETWEEN NO PROJECT AND THE PROJECT OR AMONG THE ALTERNATIVES													
COORDINATE AND INTERFACE WITH PLANS AT CORRIDOR ENDS		THIS GOAL WILL NOT DIFFER BETWEEN NO PROJECT AND THE PROJECT OR AMONG THE ALTERNATIVES												1	
				TOTAL (INDEX) NORMALIZED TOTAL	6.20 0.59	5.85 1.00	695 1.00	5.62	5.54	5 00	4.5% 0.603	5.40 0.92	5.36 0.92	5.18 D.00	5 03 0 80
			WBIGHT	ED NORMALIZED SCORE	15.1	17.0	17.0	18.3	16.1	15.4	14.2	15.7	15.8	16.0	14.8
<u>S</u>	AFETY/SECURITY GOALS	ASSIGNED WEIGHT: 8.0	5												
- GOALI - IMPROVE VEHICULAR SAPETY	- MEASUREMENT CRITERIA -	- COMMENT -	-TYPE OF GOAL - ALTERNATIVE CONCEPT GOAL	1					1				T		
- RALROAD	TRAINS & AUT			REDUCTION IN TRAINS & ADT (MELLIONS)	25.7	25.7	25.7	25.7	26.7	26.7	26.7	26.7	25.7	257	28.7
IMPROVE SAFETY FOR				INDEX (MAX.) REDUCTION IN	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1 00	1.00
PEDESTRIANS	EXPOSURE AT GRADE CROSSINGS		ALTERNATIVE CONCEPT GOAL	TRAINS PER DAY & CROSSINGS	3,946	3,822	3,822	3.842	3,805	3,822	3,948	3,622	3,622	3,842	3,865
IMPROVE SAFETY FOR OPERATIONS		ACCOUNTED FOR IN GOALS 1 AND 2 ABOVE		INDEX (MAX.)	1.00	0.97	0.97	0.97	0.96	0.97	1.00	0.97	0.97	0.97	0.99
INAROVE SECURITY		NOT MEASUREABLE		-				·							
				TOTAL (INDEX) NORMALIZED TOTAL	2.00	1.97 0.98	1.97	1.97	1.5%	1 97 0 98	2.00 1.00	1.97 0.98	1.97 0.96	1 97 0 99	1.98 0.99
			WEIGHT	BD NORMALIZED SCORE	I I	7.9	7.8	7.9	7.9	7.9	۵.	7.9	7.9	7.9	79
R	AILROAD GOALS	ASSIGNED WEIGHT: 20.0	<u>15</u>												
- GOALS - IMPROVE RAILROAD OPERATING	- MEASUREMENT CRITERIA - ROUTE EFFICIENCY CRITERIA	- COMMENT -	ALTERNATIVE CONCEPT GOAL		<u>г</u>	· · · · · · · · · · · · · · · · · · ·	·						- 1		
PLEXIBILITY AND EPPICENCY				NUMERICAL RATING	6.00	5.41	5.14	5.61	5.81	5.41	6.00	6.41	6.14	5.61	5 61
IMPROVE RALLROAD SPEEDS	AVG. OPERATING MILES PER HOUR		ALTERNATIVE CONCEPT GOAL	INDEX (MAX.)	1.00	0.90	0.85	0.94	0.97	0.90	1.00	0.90	0.85	0.94	0.97
		<i>2</i>		AVERAGE MILES PER HOUR	32.9	32.9	31.5	32.9	32.9	32.9	32.0	32.9	31.5	32.9	32 9
I PROVIDE FAIR AND EQUAL ACCESS	à la companya de la c	ACCOMPLISHED WITH PROJECT		INDEX (MAX.)	1.00	1.00	0.96	1.00	1.00	1.00	1.00	1.00	0.98	1.00	00 ا
FOR ALL CARRIERS		IMPLEMENTATION													
L MAINTAIN SERVICE TO CUSTOMERS	·····	THIS GOAL WILL NOT DIFFER													
AND THE BRANCE IN COSTONERS		THIS GOAL WILL NOT DIFFER AMONG THE ALTER NATIVES			e -								÷		10
			·	TOTAL (INDEX) NORMALIZED TOTAL	2.00	1.90 0.95	1.811 Q.911	1.94 0.97	1.97 0.98	1.90 0.95	2:00 1:00	1.90 0.95	1.81 0.91	1.94 0.97	1 97 0 94
											1	1	I		1

							ALTER	NATIV	ES						
	VIRONMENTAL GOALS	ASSIGNED WEIGHT: 15.0	<u> </u>	MEASURED UNIT	ALT. 1.0	ALT. 21A	ALT. 22	ALT. 13	ALT. 14	ALT. J	ALT. S	ALT. 61	ALT. 6.2	ALT. CJ	ALT.
	The one of the office of the o		_												
- GOALS - IMPROVE OVERALL QUALITY OF LIPS	- MEASUREMENT CRITERIA -	- COMMENT - INCLUDED IN OTHER GO. LS	-TYPE OF GOAL-		2										1
۲.		POLLUTION, AESTHETICS, NORE AND VIBRATION, ACCESS													
MINIMIZE PROJECTED AN FOLLUTION	TONSIONY OF CRITERIA POLLURANTS		ALTERNATIVE CONCEPT GOAL	TONS MELDAY	407.8	405.3	408.5	401.5	408.5	406.3	417.3	418.0	418.0	4180	
REDUCE ENERGY CONSUMPTION	GALLONS OF DIESEL FUEL AND GASOLINE CONSUMED/DAY		ALTERNATIVE CONCEPT GOAL	INDEX (MINL) GALLONS HER DAY (THOUSANDS)	1.00	1.00	1.00	1.00	1.00	100	0.87	0.60	1696	0.00	1
DEVELOF A PROJECT COMPATIBLE WITH AD ACENT LAND USES		INCLUDED IN OTHER GOALS. POLLUTION ASSTHETICS, ROBE AND VIEWATION, ACCESS, AND RIGHT-OF-WAY REQUIRED	÷=	INDEX (MIN.)	1.00	1.00	1.00	1.00	1.00	1.00	0.00	0.88	0.90	0.00	
RESOLVE MELSENT FOOR OR DETERIORATING STUATIONS		INCLUDED BI OTHER, GÖALS, POLLUTION AESTHETICS, NOGE AND VIBLATION, ACCESS, AND RIGHZ-OF-WAY REQUIRED									ē				
ABSTHERICS	SUBJECTIVE RATING	CONSIDERS ABOVE GRADE VISUAL IMPACTS	ALTERNATIVE CONCEPT GOAL	COMBINED ABSTHEETCS RATING	26 0.71	35	<b>3</b>	355 100	نې دي.و	35 1.00	35 071	35	35 100	36	
MINIMIZE EXPOSURE TO NOISE VIRATION	Let OR CHEL VIBRATION VELOCITY LEVEL	NUMBER OF SENSITIVE RECEPTORS - MPACTED	ALTERNATIVE CONCEPT GOAL	INDEX (MAX.) NUMBER OF BENSITTVE RECEPTORS IMPACTED	18089	. 8418	8817	10071	12016	8416	16069	94 NG	8817	10876	
		· · · · · · · · · · · · · · · · · · ·		RIDEX (MIR.) TOTAL (INDEX) NORMALIZED TOTAL	0.50 3.20 0.42	48.0 44.6 48.0	1.00 4.00 1.00	0.81 5.81 0.81	9.73 3.90 0.40	6.84 3.84 6.88	98.0 82.6 19.0	0.94 3.89 0.97	1.00 3.95 0.89	0 81 3.76 0.84	
			WRIGHT	ED NORMALIZED SCORE	124	14.4			53.41	14.8	19.2		14.8		
BCC	DNOMIC GOALS	ASSIGNED WEIGHT: 10.09									74.)				
- GOALS - MOINTE ECONOMIC DEVELOPMENT MEAR AND ALONG ALAMEDA CORREDOR	– Milasunsinki pi Critoria – Subective ratiko	- COMMENT - THS GOAL WELLER LIGETOT DIFFERENTIATE BETWEEN NO FRODET AND THE FRO BETT GOAL 4 ADDRESSESTHIS GOAL 8 RELATIVE TO THE INDIVIDUAL ALTERNATIVES.	-TYPE OF GOAL - PROJECT GOAL											<u></u>	
MINIMEZE LAND DEVOTED TO PORT-RELATED BALL PRENDER	TRAIN ROUTE MILES FROM REDONDO JUNCTION TO PORTS	•	ALTERNATIVE CONCEPT GOAL												
OPERATIONS (THROUGHOUT BASIN)				TRAIN ROUTE MEES	10.1	19.1	19.0	18.1	10.1	19.1	19.1	18.1	19.6	19.1	
BURTAIN ECONOMIC GROWTH		CONSIDERED TO BE LOCAL LEVEL BOONOMICS INCLUDED IN THE FIRST BOONOMIC GOAL	<del>~~</del> .	INDEX (MIN.)	1.00	1.00	0.97	<u>مور</u>	100	t.00	1.00	1.40	0.87	1.00	
MAINTAIN/IMPROVE ACCESSTO EXETTING BUSINESSES	NUMBER OF IMPACTED FARCELS	CONSIDERS DIMINISHED ACCESS OPPORTUNITIES RELATIVE TO EXISTING CONDITIONS	ALTERNATIVE CONCEPT GOAL	NUMBER OF IMPACTED PARCELS	TRL	50	817	<b>\$05</b>	•**	411	780	120	810	60	
ROMOTE GROWTH OF INTERNATIONAL TRADE THROUGH THE PORTS		THIS GOAL WILL NOT DIFFER AMONG THE ALTERNATIVES		INDEX (MIN.)	0.63	0.78	0.75	0.81	9.60	1.00	0.54	0.78	0.81	0.81	
MINIMIZE PROPERTY ACQUISITIONS	COSTS OF LAND AND IMPROVEMENTS ACQUIRED FOR RIGHT OF WAY		ALTERNATIVE CONCEPT GOAL	RIGHE OF WAY COSTS (MILLIONS)	\$180.0	\$105.S	8 (168)	\$120.4	\$144.B	\$124.B	\$165.1	\$105.3	<b>663.2</b>	\$119.B	
		· · · · · · · · · · · · · · · · · · ·		INDEX (MRL) TOTAL (INDEX) NORMALIZED TOTAL	0.50 2.03 0.73	2.84 0.95	0.50 2.76 0.50	077 259 0.63	48.0 62.5 48.0	0.76 2.76 0.89	0.50 8.06 0.74	2.05	1.00	2.50 (2.0 (2.0	
	T GOALS	ASSIGNED WEIGHT: 25.07		ED NORMALIZED SCORE	73		••	•3	9.4	7.9	74		120	92	L
~ 00AL3 ~	- MEASUREMENT CRITERIA -	- COMMENT -	-TYPE OF GOAL-	5											×
AJOMIZE COST - EFFECTIVENESS	- BEASURE MEAT CHITERIA - ABSOLUTE COST OF AN ALTERNATIVE	- CURIMENT -	ALTERNATIVE CONCEPT GOAL	ABSOLUTE COSTS (MILLIONS)	\$1,089.8	\$1,978.7	* \$2,040.8	BR DET A	\$1,791.0	82,001 a	Ø1.000.1	61,950 3	61,862.7	\$1,872.B	81.
MAXIMIZE COORDINATION OF CORRIDOR PRO BCT WITH EXISTING PRO BCTS AND PUNDING SOURCES (PRO YACK)	an a	CONSTANT FOR ALL ALTERNATIVES		INDEX (MIN.)	0.89	Car O	0.77	q. <b>1</b> 9	0.80	<u>0</u> .01	1.00		0 81	084	
ABLITY TO IMPLEMENT IN PRASES	NUMBER OF DESCRETE/SE PARATE	······································	ALTERNATIVE CONCEPT GOAL												
	CONSTRUCTION COMPONENTS			CONSTRUCTION PHASES	30	18	17	20	20	18	30	18	17	22	
				INDEX (MAX.) TOTAL (INDEX.)	1,00	0.53	0.57	0.73	0.67	0.63	2.00	134	0 87	180	
				NOR MALIZED TOTAL	1.00	0.67	0.87	0.75	0 77	0 57	1.00	0.87	0.69	0.76	

.

Project cost would be \$1,949 million without SPTC Wilmington Branch or Alameda improvements north of Slauson

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EXHIBIT G - 1

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14/03/91

	ALAMEDA CORRID	ALTERNATIVES											2. 1.		
				MEASURED UNIT	ALT. 1.4	ALT. 21A	ALT. 22	ALT. 23	ALT. 24	ALT. 3	ALT.S	ALT. 41	ALT. 42	ALT. 43	ALT. 44
	CONSTRUCTION GOALS	ASSIGNED WEIGHT:	5.0%												
- 00ALS -	- MEASUREMENT CRITERIA -	- COMMENT -	-TTPE OF COAL-							_					
L MINIMIZE DISCUPTION TO FIGHWA	CONSTRUCTION YEARS FER CONSTRUCTION COMPONENT		ALTERNATIVE CONCEPT GOAL	CONSTRUCTION TIME (YEARS)	6.00		6.76		6.25	0.76			6.76		0.25
2 MAINTAIN ACCESS TO EXISTING BUSINESSES AND RESIDENCES		CONSIDERED TO BE INCLUDED IN GOAL 1		INDEX (MIN.)	1.00	0.89	0.89	9.49	0.99	0.00	1.00	0.48	98.0	0.44	0.88
A MINIMIZE NOISE AND OTHER CONSTRUCTION IMPACTS	NUMBER OF SENSITIVE RECEITORS' WITHIN 20 OF CONSTRUCTION		ALTERNATIVE CONCEPT GOAL	NUMBER OF SENSITIVE RECEPTORS IMPACTED INDEX (MIN.)	601 0.20	-	171	384 0.38	613 0.27	1.00	691 Q.20	138	171		613
							1.00	1.27	1,33	1.00	120	1.00	1.0	127	
				NORMALIZED TOTAL	0.63	1.00	0.89	0.87	0.95	1.00	0.63	1.00	0.00	0.67	0.665
		D NORMALIZED SCORE	32	5.0	4.6	_ 34	3.2	60	32	6.0	4.8	84	32		

			÷			ALTER	NATIV	ES	2			2
and the second sec	MEASURED UNIT	ALT. 1.0	ALT. 2.1A	ALT. 2.2	ALT. 2.3	ALT. 2.4	ALT. 3	ALT. 5	ALT. 6.1	ALT. 6.2	ALT. 6.3	ALT. 6.4

GOALS	MAXIMUN SCORE	ALT. 1.0	ALT. 2.1A	ALT. 2.2	ALT. 2.3	ALT. 2.4	ALT. 3	ALT. 5	ALT. 6.1	ALT. 6.2	ALT. 6.3	ALT. 6.4
TRAFFIC GOALS	17.0	15.1	17.0	17.0	16.3	16.1	16.4	14.2	15.7	15.6	15.0	14.6
SAFETY SECURITY GOALS	8.0	8.0	7.9	7.9	7.9	7.9	7.9	8.0	7.9	7.9	7.9	7.9
RAILROAD GOALS	20.0	20.0	19.0	18.1	19.4	19.7	19.0	20.0	19.0	18.1	19.4	19.7
ENVIRONMENTAL GOALS	15.0	12.4	. 14.8	15. <b>0</b>	14.3	13.4	14.8	12.2	14.6	14.8	14.1	13.2
ECONOMIC GOALS	10.0	7.3	9.5	9.9	9.3	8.4	9.9	7.4	9.5	10.0	9.3	8.4
COST GOALS	25.0	24.9	16.6	16.8	18.6	19.4	14.3	25.0	16.7	17.1	19.7	20.4
CONSTRUCTION GOALS	5.0	3.2	5.0	4.5	3.4	3.2	5.0	3.2	5.0	4.5	3.4	3.2
SCORE		90.9	89.8	89.2	* 89.2	88.0	87.3	89.9	88.4	88.1	88.8	87.4
NORMALIZED SCORE		1.00	0.99	0.98	0.98	0.97	0.96	0.99	0.97	0.97	0.98	0.96

\* Score would be 89.6 if project costs were \$1,949 million

PAGE 1 of 3

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i.

At the conceptual level analysis of this goal, the benefit of any buffering was not considered (i.e., existing tall buildings in close proximity to the Corridor), nor were any vibration concerns. The goal addressing aesthetics is based on a subjective rating. The goal related to noise is the only one of this category that has any measurable difference among the alternatives.

from, ACTC Engineering Keport drall 10/10/91

#### e. Economic Goals

This category of goals was given a weight of importance equal to 10. Three out of the total six options were measured. While the goal of minimizing land devoted to Port-related rail freight does not vary, the goal of minimizing property acquisition was easily quantified, by determining the cost of acquisition of land in millions of dollars. The effect on access to business properties could also be quantified for comparison of the alternatives.

#### f. Cost Goals

This category of goals was given a weight of importance equal to 25. Two of the three goals are quantified. One of the three is the comparison of total project cost by alternative. The second goal compares alternatives by the ability to implement the construction by phases. The number of construction segments was developed by looking at the number of complete operational units or a portion of an operational unit costing in the range of \$400 million. The operational unit, when complete, could be put into service.

#### g. Construction Goals

This category of goals was given a weight of importance equal to 5. Two of the three goals were measured. The sensitive receptors were used to measure the magnitude of noise during construction. The value of this goal is not significant, since there are not many sensitive receptors along the Corridor. The second goal measured the number of years that a particular location would be affected by the proposed construction.

#### 4. CONCLUSION

Initially, and prior to beginning the work of developing the alternative designs, it seemed that the comparison of the adopted goals would result in a favorable ranking of one or a few of the alternatives. The analysis did not provide such "winners." Therefore, other factors would have to be considered for determining the alternatives selection that would proceed into the EIR process.

The four-lane options could be eliminated, because of the results of the traffic analysis performed. Also, the impacts of the four-lane alternatives would be analyzed as part of the status quo alternative.

The environmental review of the shortened, depressed trainway options would be covered by performing the environmental analysis of the full, depressed alternative.

Alternative 3 scored comparatively high. However, when the factor of vibration is considered in the upcoming review period, this alternative may lose any advantage, because of the proximity of the depressed train to the existing buildings. Since this alternative is the most costly, it was eliminated from further consideration. Based on this conceptual stage analysis and the results of comparing the success of the alternatives in comparison to meeting the project goals, Alternatives 1, 2.1 And 2.2 should receive additional environmental review and be considered as the candidates for the Alameda Corridor project.

A Sensitivity Analysis was performed on the results of the decision matrix. The matrix was analyzed by final scores and again by ranking of the scores by category of the goals, with and without costs. The results of this analysis are shown in Exhibits "Alter-Native Scores Without Costs," Alternatives Ranking with Costs," and "Alternatives Ranking Without Costs."

## EXHIBIT G.2: ALTERNATIVE SCORES AND RANKING WITH COST

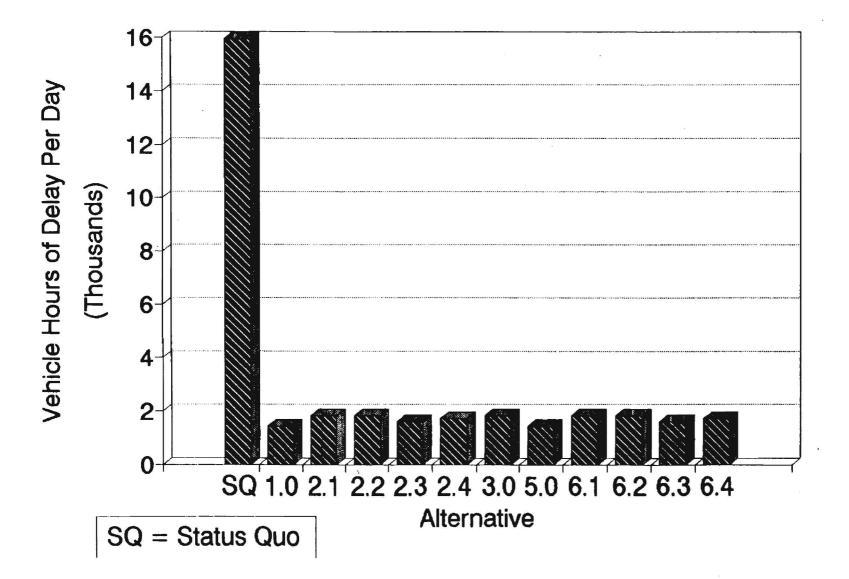
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GOALS	HIGHEST SCORE/RANK	ALT. 1,0	ALT. 2.1A	ALT. 2.2	ALT. 2.3	ALT. 2.4
TRAFFIC GOALS	17.0	15.1 4	17.0 1	17.0 1	16.3 2	16
SAFETY SECURITY GOALS	8.0 1	8.0 1	7.9 2	7.9 2	7.9 2	. 7
RAILROAD GOALS	20.0 1	20.0 1	19.0 4	18.1 5	19.4 3	. 19
ENVIRONMENTAL GOALS	15.0 1	12.4 5	14.8 2	15.0 1	14.3 3	ʻ 13
ECONOMIC GOALS	10.0 1	7,2 7.5	8.9 9.5 3 X	9.4 9.9 2 X	8.4 9.3 43	
COST GOALS	25.0 1	24.9 1	16.6 5	16.8 4	18.6 3	19
CONSTRUCTION GOALS	5.0 1	3.2 4	5.0 1	4.5 2	3.4 3	3
SCORE (MAXIMIZE)	100.0	90,8 98.9	8721-89.8	88.7 80.2	85,3 89.2	1 09 5 RB
TOTAL NUMBER OF GOALS RANKED NO. 1 (MAXIMIZE)	7	3	2	z <i>"S</i>	80,2 <b>69.</b> 2 <b>≩</b> 0	5 1. J ac

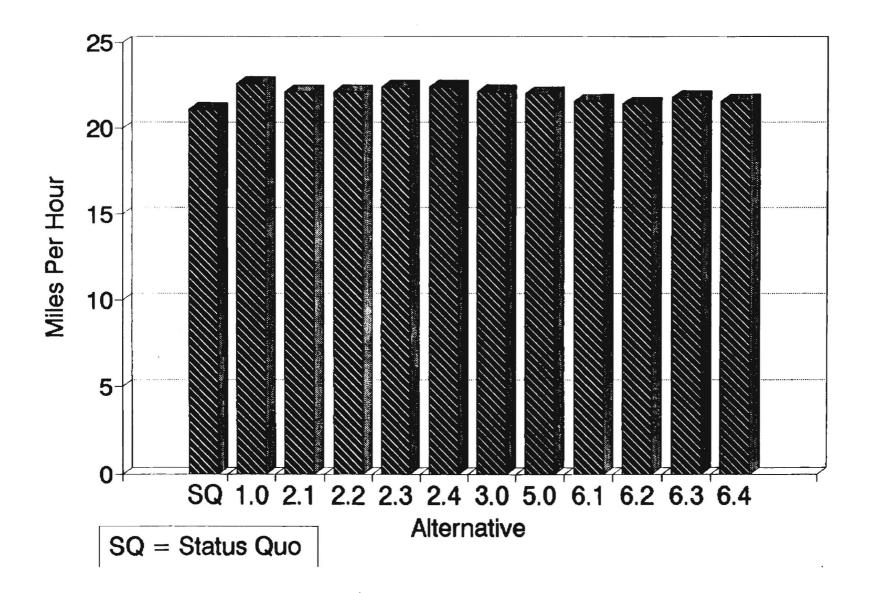
GOALS ALT. 2.4 HIGHEST SCORE/RANK **ALT. 1.0 ALT. 2.1A** ALT. 2.2 ALT. 2.3 TRAFFIC GOALS 17.0 17.0 17.0 16.3 16.1 15.1 1 3 2 1 7,6 7.9 7.6 29 7.6 -7.9 SAFETY SECURITY GOALS 8.0 8.0 7,6 7.9 r 2 2 2 2 RAILROAD GOALS 20.0 20.0 19.0 18.1 19.4 19.7 X 2 5 3 15.0 15.0 13.4 **ENVIRONMENTAL GOALS** 12.4 14.8 14.3 X 2 8,4 10.0 7.2 23 8.9 25 9,4 9.9 9.3 9.9. 8.4 ECONOMIC GOALS x COST GOALS 5.0 CONSTRUCTION GOALS 3.2 5.0 4.5 3.4 3.2 Y 2 1 3 65.8 68.9 72.3 73.2 71.6 72.4 69,3 28.5 69,9 68.7 SCORE (MAXIMIZE) 75.0 TOTAL NUMBER OF GOALS 6 Or 2 28 **RANKED NO. 1 (MAXIMIZE)** 

**EXHIBIT G.3: ALTERNATIVE SCORES AND RANKING WITHOUT COST** 

# Vehicle Hours of Delay Per Day At Railroad Grade Crossings - Year 2020

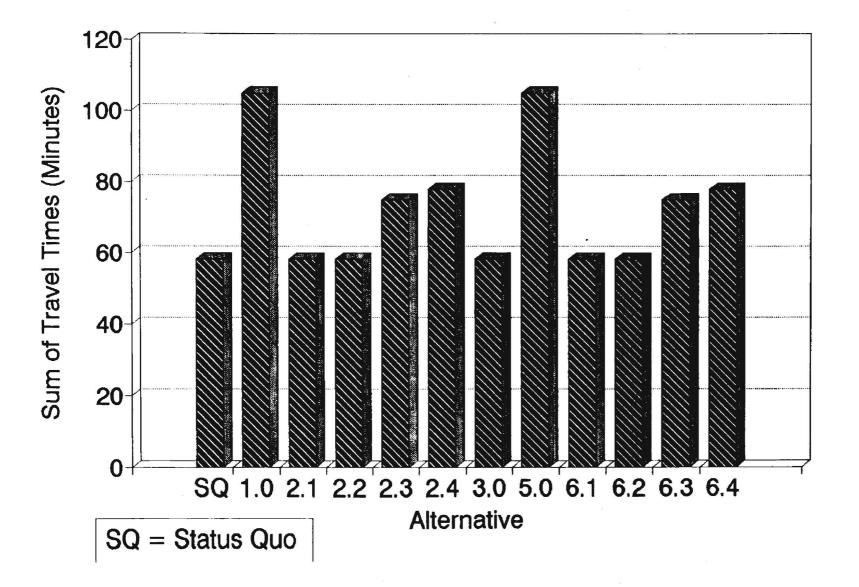


# Average PM Peak Hour Travel Speeds Alameda Street - Year 2020



10/10/91

# Travel Time For Turning Movements From Alameda Street to One Block Away



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PREPARED BY: DMJM/M&N-DRM DATE: 9/19/91 REV: 10/2/91

#### **GOAL ANALYSIS - TECHNICAL MEMORANDUM**

#### GOAL:

Traffic Goal No. 3 - Improve Level of Service at Intersections

#### **INTRODUCTION:**

This memorandum presents a comparative evaluation of Alameda Corridor alternatives relative to their ability to reduce delays of vehicles wishing to negotiate the new Alameda intersections and reach a distant point off the corridor. This information is incorporated into a larger matrix that compares alternatives to improve Alameda Corridor for both freight rail and vehicular traffic.

The measurement criteria used to evaluate the vehicular travel time delays at an intersection is vehicular minutes of travel time delay due to requiring the drivers to take circuitous routes to reach their destinations. It is presented as the projected minutes of delay associated with each alternative.

#### **METHODOLOGY:**

The evaluation is based on estimating total vehicle travel time/delays in minutes by alternative, associated with negotiating the proposed intersection(s) to reach a distant point one block off the corridor. The vehicle must either traverse the grade separations or an atgrade bridge over the depressed trainway to reach the distant point.

The construction of roadway grade separation structures in order to eliminate the traffic conflict with an at-grade trainway will required additional travel distance and time to reach a destination point on the opposite side of the trainway. This additional distance and time can be compared to the travel time associated with negotiating the at-grade intersection of the depressed trainway alternatives. The comparison by alternative of the delay in reaching a certain point of intersection one block off of Alameda is a reasonable approach for comparing the serviceability of the alternatives or under this Goal.

Further, this approach is reasonable for comparing the alternatives, rather than comparing level of service of Alameda intersections, by analyzing the volume to capacity ratio of the intersections along Alameda. It is important to note that the Alameda intersections will be designed to provide a uniform level of service and signal timing with progression, to provide for the most efficient traffic control system practical. Also, the signals placed at the terminal point intersections of the grade separation structures will also be provided with a reasonable efficient design to maintain through traffic-operation along the east-west local streets.

An average speed of 25 mph and average 3 minute signalized intersection phasing were factors used for this analysis.

This goal is developed, where the low score is better than the higher. This differs from the procedures used for the other goals. Therefore, the lower value of time delay in minutes is the best score. The following is the results of this analysis:

## ALTERNATIVE

SEGMENT	<u>1.0/5.0</u>	<u>2.1/6.1</u>	2.2/6.2	<u>2.3/6.3</u>	2.4/6.4	<u>3</u>
A B C D	n/a 53 43 9	n/a 25.1 23.9 9	n/a 25.1 23.9 9	n/a 25.1 40.7 9	n/a 25.1 43.1 9	n/a 25.1 23.9 9
<b>TOTALS</b>	<u>105_min</u>	<u>58 min</u>	<u>58 min</u>	<u>74.8 min</u>	<u>77.8 min</u>	<u>58 min</u>
INDEX	<u>0.55</u>	<u>1.0</u>	<u>1.0</u>	0.77	<u>0.74</u>	<u>1.0</u>

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PREPARED BY: DMJM/M&N-MGG DATE: 9/03/91 REV: 10/2/91

#### **GOAL ANALYSIS - TECHNICAL MEMORANDUM**

#### **GOAL:** Traffic Goal No. 4 - Improve Connections to I-105 and I-10 Freeways

#### **INTRODUCTION:**

This memorandum presents a comparative evaluation of Alameda Corridor alternatives relative to their ability to provide improved connections to the I-105 and I-10 Freeways. This information is intended to be incorporated into a larger matrix that compares alternatives to improve Alameda Corridor for both freight rail and vehicular traffic.

The measurement criteria used to evaluate the improved connections to those freeways provided by each alternative is the time traveled (in minutes) from Alameda to each freeway.

#### **METHODOLOGY:**

The conceptual designs for the Alameda Corridor alternatives provided the basis for this analysis. Each design was reviewed and the distance from the nearest intersection along Alameda to the freeway on-ramp entrance as experienced by a motorist was established. The travel speed was then used to establish the time it would take to traverse this distance. This analysis was prepared for both the westbound as well as the eastbound move and is summarized in Table ATG4-1.

#### **GENERAL OBSERVATIONS:**

The major difference occurs between the depressed railroad condition and the at-grade railroad condition at the connection to the I-105 Freeway. Under the depressed railroad condition, both eastbound and westbound connections are provided. Under the at-grade railroad condition, only the eastbound connection is provided. The westbound connection is handled with the existing connection at the I-105 Freeway/Wilmington Avenue Interchange.

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#### DEPRESSED RAILROAD ALTERNATIVES

#### AT-GRADE RAILROAD ALTERNATIVES

ALAMEDA TO I-105 FWY

TOTAL TRAVEL TIME

7.9

8.3

6.3

6.7

TRAVEL TIME (MIN)	<u>ALT. 1</u>	ALT. 2.1	ALT. 2.2	<u>ALT. 2.3</u>	ALT. 2.4	ALT. 3	ALT. 5 ALT.		<u>ALT. 6.3</u>	ALT. 6.4
	TRAVEL TIME		7.9 8.3	r K		TOTAL TRAVI	TOTAL	- 105 TRAVEL TIM		6.3 6.7
WEST BOUND MOVE TO FWY MERGE	3,900	13	3.4			NOC - MAN IS NOT ON	MOVE TO FWY ME MOVE ALONG FWY	REAL REAL		
ALAMEDA TO I-105 EAST BOUND MOVE TO FWY MERGE	DISTANCE (FEET) 5,200	SPEED (MPH) 13	TRAVEL TIME (MINUTES) 4.5			ALAMEDA TO EAST BOUND	I–105 MOVE TO FWY MER	DISTANCE (FEET) RGE 3,950	SPEED (MPH) 13	TRAVEL TIME (MINUTES) 3 3.5
ALAMEDA TO I-10 EAST BOUND MOVE WEST BOUND MOVE TOTAL TRAV	DISTANCE (FEET) 100 300 EL TIME	SPEED (MPH) 13 13			, , ,		AST BOUND MOVE			

6.3

6.7

6.3

6.7

#### TABLE: ATG4-1 TRAVEL TIME IN MINUTES FROM ALAMEDA TO I-10 AND I-105 FREEWAYS

6.3

6.7

6.3

6.7

7.9

8.3

6.3

6.7

Revised 10/2/5)

6.3

6.7

6.3

6.7

6.3

6.7

PREPARED BY: DMJM/M&N-MGG DATE: 8/29/91 REV: 10/2/81

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### **GOAL ANALYSIS - TECHNICAL MEMORANDUM**

**GOAL:** 

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Traffic Goal No. 5 - Provide Alternate Route to Parallel Freeways (Emphasize Trucks)

#### **INTRODUCTION:**

This memorandum presents a comparative evaluation of Alameda Corridor alternatives relative to their ability to attract Port trucks to Alameda Street and reduce Port trucks on the I-110 and I-710 Freeways which parallel Alameda Street. This information is intended to be incorporated into a larger matrix that compares alternatives to improve Alameda Corridor for both freight rail and vehicular traffic.

The measurement criteria used to reflect the relative difference of the alternatives in their ability to effect truck travel is Port Truck Vehicle Miles of Travel (VMT - Port Trucks).

#### **METHODOLOGY:**

Projections of vehicle travel were made in the Highway Capacity and Level-of-Service Analysis (HC & LOSA) effort for this project. The traffic model used assigned the Port Truck travel independently to the network and the plots with this information were used to develop the VMT-Port Truck analysis. Six transportation models were run in the HC & LOSA effort and analyzed to determine the Port truck volumes on Alameda and the I-110 and I-710 Freeways.

The six transportation models are briefly described below:

- <u>MODEL A</u> Existing conditions in the Year 1990 (mainly used for model calibration efforts)
- <u>MODEL B</u> "Status Quo" (No Build or Null Case) conditions incorporating proposed transportation improvements defined for the transportation network outside of the improvements under consideration for the Alameda Corridor Project.
- <u>MODEL C</u> "Build" (Improve Alameda Corridor) Equivalent to Alternative 1 for Alameda Corridor incorporating a six-lane roadway and an at-grade railway with 25 grade separations of major cross-streets.

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<u>MODEL D</u> - "Build" (Improve Alameda Corridor) Equivalent to Alternative 5 for Alameda Corridor incorporating a four-lane roadway and an at-grade railway with 25 grade separations of major cross-streets.

<u>MODEL E</u> - "Build" (Improve Alameda Corridor) Equivalent to Alternative 2.1 for Alameda Corridor incorporating a six-lane roadway and a depressed railway with 15 intersections north of the Artesia Freeway (SR-91).

<u>MODEL G</u> - "Build" (Improve Alameda Corridor) Truck Only Expressway alternative which incorporates a two-lane (one-lane north and one-lane south) truck only expressway from SR-91 to south of 51st Street with a four-lane roadway and a depressed railway north of SR-91 (Similar to Alternative 2.1)

Using the planning Year 2020, the traffic model network for Average Daily Traffic (ADT) Volumes were analyzed for the numbers of Port trucks on the I-110 and I-710 Freeways and Alameda Street. The Port truck volumes were analyzed by segments established for this project except that Segments A and B were combined because of difficulties associated with quantifying the freeway volumes for these segments independently. North and Southbound traffic were combined for each link<sup>1</sup> within a segment and averaged to represent the volume for that segment of the facility. The actual length of the roadway was defined for each segment and the average volume was multiplied by that length to establish the VMT for Port trucks. This was done for Alameda, the I-110 Freeway, and the I-710 Freeway. This computation was performed for Model Runs C through E in order to develop the relative attraction for each "Build" alternative. Each alternative then compared to Model Run B which represented the volume of Port-trucks on each facility under the "Status Quo" scenario.

The VMT-Port trucks presented represents the net increase on Alameda plus the decrease on each of the freeways ( $\triangle$  Alameda "increase" +  $\triangle$  I-110 "decrease" +  $\triangle$  I-710 "decrease"). If the freeways gained Port trucks in any scenario those Port truck miles were subtracted from the VMT-Port trucks reported ( $\triangle$  Alameda "increase" -  $\triangle$ I-710 "decrease" -  $\triangle$ I-110 "decrease").

Tables ATG5-1, ATG5-2, ATG5-3, ATG5-4 present the results of this analysis.

The Alameda Corridor Alternatives 2.2, 2.3, 2.4, 3, 6.1, 6.2, 6.3, and 6.4 were not modeled in the HC & LOSA. Those alternatives are hybrids of alternatives 1, 2.1, and 5 which were modeled. The VMT-Port trucks for these alternatives have been estimated from the model data available as the similarities with the alternatives that were modeled were considered adequate for the overall evaluation. It is reasonable to assume that slight variances from

<sup>1</sup>Links are model representations for actual streets.

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the estimated VMT-Port trucks would occur if each alternative were modeled. The general approach to developing volumes for alternatives not modeled is presented below:

• Alt. 2.2 - This alternative slightly differs from Alternative 2.1 as it includes a diversion of rail traffic to the Southern Pacific Wilmington alignment for the depressed railroad between Slauson Avenue and 25th Street. The remainder of this alternative is exactly the same as Alternative 2.1. Also, the number of traffic lanes on Alameda Street from Slauson to 25th Street is the same as Alternative 2.1. Therefore, the 2.1 volumes were used for this alternative.

Alt. 3 - This alternative includes a depressed railroad along the east side of Alameda Street with six-lanes on Alameda from SR-91 thru 25th Street. The roadway configuration is very similar to Alt. 2.1 with respect to intersections with cross-street traffic. Therefore, the volumes from 2.1 were considered appropriate for Alt. 3.

Alts. 2.3 & 2.4
 These alternatives are hybrids of Alts. 1 and 2.1. Therefore, where a transition occurred from Alt. 1 to Alt. 2.1, the appropriate volumes were averaged for that segment. The adjacent segment volumes were then adjusted to best reflect the type of roadway configuration and also considering a major facility (such as a freeway connection) within that segment.

This alternative has the same relationship to Alt. 2.1 that Alt. 1 has to Alt. 5 (six-lane with depressed railroad to four-lane with depressed railroad vs. six-lane with at-grade railroad to four-lane with at-grade railroad). Therefore, a decrease in Port trucks on Alameda Street was projected for Alt. 6.1 using the same ratio decrease that was realized between Alt. 1 to Alt. 5 (Model C vs. Model D).

Alt. 6.1 = (Model E) (Model D) Model C (Alameda Street Estimates)

However, the freeways were treated as a net decrease of Port trucks when estimating the Port trucks for Alternative 6.1:

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Alt. 6.1 = Model E - [Model D - Model C] (Freeway Estimates)

A 1+c

Alt. 6.1

3

Alts. 6.2, 6.3 - These alternatives were treated similarly to Alt. 2.2, 2.3, and 2.4 in developing the VMT-Port trucks.

The resulting matrix of VMT-Port trucks is presented in Table ATG-5 with the estimated VMT-Port trucks shown in italics.

#### **GENERAL OBSERVATIONS:**

- Model G results in the highest volumes of Port truck miles affected with approximately 132,800, of this total 44% or approximately 58,000 VMT are caused by a reduction on the freeways.
- Model C (Alt. 1) results in the next highest volumes of Port trucks miles affected with approximately 19,500, of this total approximately 27% or approximately 5,300 are caused by a reduction on the freeways.
- Model E (Alt. 2.1) results in approximately 17,5000 VMT-Port trucks with approximately 39% or approximately 6,700 VMT of those Port trucks attributed to a reduction on the freeways.

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TABLE: ATG5-2

## 2020 - MODEL "D" - 4 LANE ALAMEDA & @ GRADE R.R. PORT TRUCK FREEWAY DIVERSION TO ALAMEDA CORRIDOR 2020 FUTURE TRAFFIC - PORT TRUCKS ONLY - ADT August 21, 1991 - POHTRUCK/PT20C-A.WK3

			-110 FR	EEWAY							STREET						-710 FA					i
SEGMENT NO.	NORTH	SOUTH	IOTAL	NORTH	SOUTH	TOTAL	10" - 18"		SOUTH	TOTAL	NORTH	SOUTH	TOTAL	10° - 18°	NORTH	SOUTH	TOTAL	NORDI	SOUTH	TOTAL	10° - 10"	PERCENT OF TOTAL PORT THUCK MILES
A & B								48	38	86	50	245	295		402	395	797	425	417	842		AFFECTED
	633	717	1350	609	722	1331		74	64	138	73	245	318		1786	2006 2514	3792 5028	1804 2486	2020 2526	3824 5012		FREEWAY
	633	717	1350	609	722	1331		13	12 73	25 133	198 310	186 191	384 501	6	2645	2675	5320	2578	2692	5270		
	796	725	1521	843	769	1612		64	66	130	445	191	636		2797	2842	5639	2764	2843	5607		
1 1	1035	874	1909	1111	1124	2235	. 51	31 32	30 30	61 62	443 548	244 266	687 914									
	1654 1672	1695 <u>1722</u>	3349 3394	1773 1790	1903 1903	3676 3693		38 73	38 52	76 125	766	266	1032									
SEG SUM	6423	6450	12873	6735	7143	13878		453	411	864	3008	2059	5067		10144	10432	20576	10057	10498	20555	5	<i>n</i>
# OF UNKS	6 1071	5 1075	6 2146	6	6	6 2313		10 45	10	10 86	9 334	9 229	9 563		5 2029	5 2066	5 4115	5 2011	5 2100	4111		
							~	000.07					÷					41. 		6.4		
P.T. VEH. MILES	5.7 6104.7	5.7 6127.5	5.7	5.7 6401.1	5.7 6768.7	5.7 13184.1	951.9	4.8 216	4.8 - 196.8	412.8	4.8 1603.2	4.8 1099.2	4.8 2702.4	2289.6	6.4 12965.6	.6.4 13350.4	6.4 26336	6.4 12870.4	6.4 13440	26310.4	- 25.6	-67.9% N/A
l																						
c	2158	2316	4476	2048	2382	4430		π	56	133					3022	3062	6084	2970	3078	6048		÷
	2134 2229	2309 2500	4443	2029 2153	2357 2508	4386 4661		86	62	148			23		3300	3338	6638	3248	3352	6600	8	1
	2301	2666	4967	2217	2613	4830		168	77	245	771	269	1040									
		*						168	87	255	811 795	398 503	1209	×	12						1	
			<i>n</i> .					618	588	1206	826	529	1355	0							1	
								639	587	1226	826	529	1355									
	2414 2660	2756 <u>2898</u>	5170 5558	2338 2575	2701	5039 5407	e.	679	608	1287	867 652	557 545	1424 1197		4234 4344	4252	8486 8668	4236	4333 4431	8569 6770		
								690	642	1332	1004	701	1705									
								729	752 930	1481 1892	1052	731	1783									
SEG SUM	100/05					00262						6.000					00.030				:	
OF UNKS	13895	15447	29343	13360	15393	28753		4816 10	4389	9205 10	8643 9	5493 9	14138		14900	14976	29876	14793	15194	29987		
AVG. VOL	2316	2575	4891	2227	2568	4792	2	462	430	921	960	610	1571		3725	3744	7469	3698	3799	7497		
SEGMENT DIST P.T. VEH. MILES	6 3 14550.8	6.3 16222.5	6.3 30613.3	6.3 14030.1	6.3 16165.8	6.3 30189.6	-623.7	6 2892	6 2634	6 5526	6 5760	6 3660	6 9426	3900	5.7 21232.5	5.7 21340.8	5.7 42573.3	5.7 21078.6	5.7 21654.3	5.7 42732.9	159.6	10.6%
- D	2794	3043	5837	2745	2969	5734		1261	1156	2407							40000					
	3723 4328	3350	7073	3608	3260	6888		1251	1130	2407	1384	1270	2654	6	5260 5351	5626 4975	10885 10326	5297 5438	5760 5276	11057 10714		
i I	4979	4318	7849 9297	4141 4782	3448 4198	7589 8980		2524	2818	5342	2336	2450	4786		5439 8475	5201 6579	10640 13054	5452 6517	5306 6340	10758 12857		
- C	5274 5274	4411 4631	9685 <u>9905</u>	5080 5080	4319 4512	9399 <u>9592</u>		2895	3328	6223	2972	3212	6184		6694 5530	6838 5777	13532 11307	6743 5764	8600 5585	13343		
								3319 3522	3738 4056	7057	3411 3814	3636 4120	7047 7934									
1								4281 4310	4389	867G	4741	5052	9793				e			Į		8
[						e (					4814	5081	9895								).	
1								<u>5058</u>	5110	10168	5648	5832	11480									
SEG SUM	26372	23274	49646	25436	22746	48182		27160	29013	56173	29120	30653	59773		34749	34996	69745	35211	34867	70078		
# OF LINKS	6 4395	6 3879	6 8274	6 4230	3791	8 0006		8 3395	8 3627	8 7022	8 3640	8 3632	8 7472		571/2	6 5833	6 11624	6 5059	6. 5011	6 11680		
SEGMENT DIST	64	84	6.4	64	6.4	64		6.8	6.8	6.8	6.8	68	68		65	6.5	6.5	65	65	6 5		
P.T. VEH. MILES	28128	24625.6	52953.6	2/129.6	24262.4	51362	- 1561.6	23085	24663 6	4//49.6	24752	26057.6	50609.6	3060	3/648	37814 5	75556	38148.5	37711.5	756630	364	28.1%
9																						
		203	N 200005 20		S	jub total	-1233.4					\$	Sultonal	9249.6					S	idutotal	498	7.5%

PORTRUCK/PT20D-AWKJ

TOTAL PORT TRUCK MILES AFFECTED: 9985

TABLE: ATG5-3

## 2020 – MODEL "E" – 6 LANE ALAMEDA & DEPRESSED R.R. PORT TRUCK FREEWAY DIVERSION TO ALAMEDA CORRIDOR

2020 FUTURE TRAFFIC - PORT TRUCKS ONLY - ADT August 21, 1991 - PT20E-A.WK3

			-110 FA	EEWAY			a			ALAMED	STREET					]	-710 FF	EEWAY				
SEGMENT NO.	NORTH	SOUTH	TOTAL		SOVIH	TOTAL	T-8	NORTH	SOUTH	TOTAL	NORTH	SOUTI	TOTAL	- T	NOTTI	SOUTH	TOTAL	NORTH	SOUTH	TOTAL	E - B	PERCENT OF TOTAL PORT THUCK MILES
AAB								48	38	86	73	130	203		402	395	797	351	414	765		AFFECTED ATTRIBUTED TO
	633	717	1350	557	705	1262		74 20	64 8	138 28	102	142	244 87		1786 2514	2006 2514	3792 5028	2055 2298	1734 2590	3789 4668		FREEWAY
-	633	717	1350	557	705	1262		13	12 73	25 133	178 238	33 37	211 275		2645	2675	5320	2389	2753	5142		
	798	725	1523	713	714	1427		64	66	130	157	37	194		2797	2842	5639	2575	2914	5489	121	
	1035	874	1909	931	1084	2015		31 32	30 30	61 62	198 227	53 80	251 307								22	
	1654	1695	3349	1727	1905	3632		38	38	76	276	81	357									
	1672	1722	3394	1753	1980	3733	2	13	52	125												
SEG SUM	6425	6450	12875	6238	7093	13331		453	411	864	1523	606	2129		10144	10432	20576	9668	10405	20073		r.
# OF LINKS AVG. VOL	1071	1075	2146	1040	1182	2222		45	41	86	189	67	237		2029	2066	4115	1934	2081	4015		
SEGMENT DIST	5.7	5.7	5.7	5.7	5.7	5.7		4.8	4.8	4.8	4.8	4.6	4.6	-	64	6.4 13350.4	6.4 26336	6.4 12377.6	6.4 13318.4	6.4 25696	-640	22.2%
PORT T. MILES	6104.7	6127.5	12232.2	5928	6737.4	12665.4	433.2	216	196.8	412.8	811.2	321.6	1137.6	724.8	12965.6	13330.4	20030	12377.6	13310.4	2.4080	-010	22.2.8
								·	-													l
c	2158	2318 2309	4476 4443	1962 1904	2180 2181	4142		77 86	56 62	133 148	280 289	85 92	365 381		3022	3062	6084	2778	3140	5918		
	2229	2500 2666	4729 4967	1980 2084	2333 2468	4313		168	77	245	682	394	1076		3300	3338	6638	3056	3412	6468		
	1	2000	1001	2001				168	87	255	682 990	459 868	1141 1858							•		
						1		618	588	1206	1041	900	1941					i				
								0.0000000				0.00	0057		a							
	2414	2756	5170	2161	2577	4738		639	587	1226	1071	986 1010	2057 2085		4234	4252	8486	4241	4315	8556		
	2660	2698	<u>5558</u>	2396	2717	5113		679	608	1287	1075	1023	2098		4344	4324	<u>8668</u>	4347	4389	8736		2
						[		690 729	642 752	1332 1481	629 1320	1030	1659									
						1		962	<u>930</u>	1892												a *
SEG. SUM	13896	15447	29343	12487	14476	26963		4816	4389	9205 10	9134 11	7960	17114		14900	14976	29876	14422	15255	29678		
AVG. VOL	2316	2575	4891	2061	2413	4494		482	439	921	830	725	1556		3725	3744	7469	3606	3814	7420		
SEGMENT DIST	6.3	6.3	6.3	63	6.3	63		6	6	6	6	6	6		5.7	5.7	5.7	5.7	5.7	5.7		
PORT T. MILES	14590.8	16222.5	30613.3	13110.3	15201.9	28312.2	-2501.1	2892	2634	5526	4980	4350	9336	3810	21232.5	21340.8	42573.3	20554.2	21739.8	42294	- 279.3	42.2%
I																		-				
D	2794 3723	3043 3350	5837 7073	2517 3588	2869 3168	5366		1251	1156	2407	1409	1690	3099	1.0.10	5260 5351	5626 4975	10886	5346 5442	5642 5066	10986		
	4328 4979	3521 4318	7849	4178 4639	3332 4079	7510 8918		2524	2818	5342	2776	3001	5777		5439 6475	5201 6579	10640	5500 6184	5148 6204	10648		
	5274 5274	4411 4631	9685 9905	5131 5131	4193	9324 9510		2895	3328	6223	3126	3571	6697		6694	6838	13532	6424	6509	12933		
	No.L.Y	1001	****	<b>*1</b> ×1	241.	22.9		3319	3738	7057	3577	4001	7578		<u>5530</u>	5771	11307	5324	5697	11021		
								3522 4281	4056 4389	7578	4077 4789	4673 5179	8750 9968									. *
1								4310	4418	8728	4854	5222	10076									
								5058	<u>5110</u>	10168	5634	5945	11579									
SEG SUM	26372	23274	49646	25384	22020	47404		27160	29013	56173	30242	33262	63524		34749	34996	69745	34220	34266	68486		e 6
OF LINKS	6 4395	8 36/39	6 82/4	6 4231	6 3670	6 7901		8	8 3627	8 7022	3780	8 4 160	8		5/192	6 5633	6 11624	6 5703	5/11	5		8
SEGMENT DIST								- 177 G.	•	1000					2722022070			0				
PORT T. MILES	6 4 26126	6.4 248:5 6	6 4 52953 8	27078.4	6.4 23488	0.4 50568.4	~ 2387.2	6.8 23085	6 8 24663.8	47749.6	6.8 25704	6 8 28288	6.8 5'UU0.0	5249.2	6.5 37648	6 5 37914.5	6.5 75056	6.5 370£9,5	37121.5	6.5 74191	- 1365	37.5%
			d			lubiotal	- 4455.1						kubtotal	10784							-2284.3	38 9%

PT20E-A WK3

.

#### TOTAL PORT TRUCK MILES AFFECTED: 17523.4

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## 2020 - MODEL "G" - TRUCK EXPRESSWAY & DEPRESSED R.R. PORT TRUCK FREEWAY DIVERSION TO ALAMEDA CORRIDOR

ALAMEDA STREET I-710 FREEWAY I-110 FREEWAY MODEL \*8\* PERCENT OF TOTAL PORT TRUCK MILES MODEL "G" NORTH SOUTH TOTAL "G" - "B" MODEL "G" NORTH SOUTH TOTAL MODEL 'G' NORTH SOUTH TOTAL "G" - "B" NORTH SOUTH TOTAL "G" - "B" NOATH SOUTH TOTAL TOTAL AFFECTED ATTRIBUTED TO 84 8 FREEWAY 25 133 130 13 60 REDUCTIONS 2421 2314 888 12 73 1523 2797 2642 5839 3791 

## 2020 FUTURE TRAFFIC - PORT TRUCKS ONLY - ADT August 13, 1991 - PORTRUCKP 120G-A.WK3

#### PORTRUCK/PT20G-A.WK3

SEGMENT

NO

ALB

MODEL "B"

#### TOTAL PORT TRUCK MILES AFFECTED:

TABLE: ATG5-4

	633	717	1350	448	449	895		20 13	12	28 25 133	1420 1587 2478	1434 1591 2421	2654 3178 4697		2514 2645	2514 2675	5028 5320	1649	1629 1737	3278 3470		REDUCTIONS	
	798 1035	725 874	1523 1909	392 709	496 809	888 1518		80 84 31	73 66 30	130	2513 2491	2314 2397	4827		2797	2642	5839	1692	1699	3791			
	1654	1895	3349	1055	1345	2403		32 38	30 38	62 76	1553 2710	2585	4138										
	1672	1722	3394	1063	<u>1350</u>	2433		. 73	52	125													
# OF LINKS	6425	6450	12675	4131	4901	9032		453	411	864	18739	17312	34051		10144	10432	20576	 5	7156	14149			
AVG. VOL.	1071	1075	2146	ueiQ	817	1505		45	41	80	1860 -	1924	3783		2029	2066	4115	1397	1433	2630			
SEGMENT DIST PORT T. MILES	5.7 8104.7	57 6127.5	5.7 12232.2	5.7 3427 3	5.7 4656.9	5.7 6678.5	- 3653,7	4.8 218	48 196.8	4.8 412.8	4.8 B¥28	4 8 9235 2	4.8 18168 4	17745.8	64 12985 6	6 4 13350.4	64 26336	6.4 8940.5	0.4 9171.2	0.4 18112	~6224	40,1%	
c	2156	2318 2309	4478	1251	1466	2717		77	56 62	133 148	2725	2577 2642	5302 5412		3022	3062	8064	2086	2082	4168			
	2229 2301	2500 2666	4729	1240	1546	2765 2591	-	100	π	245	2920	3132	6052 6951		3300	3358	6638	2355	2353	4708		(C)	
								166	67	255	2967	3102	6069										
								618	588	1206													
								639	587	1226										2			
	2414 2660	2758	5170 5558	1376 1560	1685	3063 3365	1	679	806	1287	3265 3301	3440 3484	6706 5785		4234 4344	4252 4324	8466 8668	3527 3599	3144 3185	6671 6784			
								690 729 952	642 752 930	1332 1481 1892										•			
SEG SUM	13896	15447	29343	7699	9588	17487		4810	4389	9205	20689	21407	42298		14900	14975	29876	11567	10764	22331			
# OF LINKS AVG, VOL.	2310	8 2575	4891	1317	6 1598	6 2915		10 462	10 439	10 921	7 2964	3058	6042		3725	3744	4	4	4 2091	4 5583			
SEGMENT DIST PORT T. MILES	6 3 14560 6	8 3 16222 5	6.3 30613.3	8.3 8297.1	6.3 10067.4	6.3 18364.5	- 12448.8	6 2592	8 2634	6 5526	6 17904	6 18345	6 36252	30726	5.7 21232.5	5.7 21340.8	5.7 42573.3	5.7 16484.4	5.7 15336.7	5.7 31,823.1	- 10750.2	43.0%	
D	2764	3043 3350	5837 7073	1694 2531	1973 2267	3057 4016		1251	1156	2407		3733			5260 5351	5626 4975	10686 10326	4707	4390	9097 9363			
	4328	3521 4318	7649	3136	2452	5590		2524	2618	5342	3698 4435		7431		5430 6475	5201	10640	4900	4840	9740			
	5274 5274	4411 4631	9885	4122	3303 3463	7425		2895	3328	6223	4767	4375	8610		0694 5530	6579 6636 5777	13532	6200 5057	5945 5054	11543 12145 10111			
			2222	1166	*2264	1904		3319 3522	3738 4058	7057 7578	5208 5222	5292 5833	10498		2230	3717	TING	2007	202	10111			
								4281	4389	8670 8726	5439 8097	6062 6453	11501										
· ·								5058	5110	10168	6157 6933	6482 7195	12550 12639 14129										
SEG SUM	26372	23274	49646	19427	10687	30114		27100	29013	56173	47954	50201	96215		34749	34996	89745	31584	30435	61099			
AVG. VOL.	4365	3679	8274	6 32:16	2781	0019		3365	8 3627	7022	6328	9 5685	9 10913		5792	5633	11824	6201	50/3	10333			
SEGMENT DIST	64	6.4	64	8.4	84	-8.4		0.5	6.8	6.6	6.8	6.8	88		8.5	6.5	8.5	6.5	6.5	85			
PORT T. MILES	26126	24825.8	52953.6	20723.2	17798.4	38521.6	- 14432	23066	24663.6	47749.6	36230.4	37978	74208.4	28458 8	37648	37914.5	75658	34198.5	32974.5	67104.5	-8391.5	40.3%	
				· · · ·		ubtotal	- 30535						ubtotel	74930.4			أرجعهم		R	ubtotal	-27366	43 6%	
					-		000000					-	1.000	1.1000.1									

PREPARED BY: DMJM/M&N-MGG DATE: 9/11/91 REV: 10/2/91 i0/10191

#### **GOAL ANALYSIS - TECHNICAL MEMORANDUM**

## GOAL: Safety/Security Goal No. 1 - Improve Vehicular Safety

#### **INTRODUCTION:**

This memorandum presents a comparative evaluation of Alameda Corridor alternatives relative to their ability to improve vehicular safety for rail/vehicle interface and along Alameda. This information is intended to be incorporated into a larger matrix that compares alternatives for both freight rail and vehicular traffic.

The measurement criteria used to reflect the relative difference of the alternatives in their ability to improve vehicular/railroad safety is the number of trains times the average daily traffic at each grade crossing.

#### **METHODOLOGY:**

The measurement for vehicle/railroad conflicts was established as the number of cars at an at-grade rail crossing times of the number of trains occurring at that crossing. The number of grade crossings have been identified by rail company along with the volume of average daily traffic. This information was obtained from the 1984 study prepared by SCAG and updated for this project. Exhibit ASSG1-1 contains this information for both the "Status Quo" routing of trains and the "Consolidated Corridor" routing of trains. A definition of "Status Quo" and "Consolidated Corridor" follows:

- "Status Quo" (No consolidation of port rail traffic and no improvements to the Alameda Corridor) - Under this scenario, all existing railroad routes would remain unchanged and projected increases of Port related freight rail movements would occur on existing trackage. All Alameda Corridor highway improvements would be limited to those facilities already funded or those projects programmed and approved for funding.
- "Consolidated Corridor" (Consolidate port rail traffic and improve Alameda Corridor to one of the Alameda Corridor alternatives under study) - Under this scenario, the Southern Pacific Railroad-San Pedro main line would be used as the consolidated route for all port related rail movements for all railroads serving the Ports; AT&SF, Southern Pacific, and Union Pacific. The through rail movements to and from the Ports along the Alameda Corridor would be grade separated at selected major cross streets with all other railroad grade crossings eliminated.

The projections of through-trains for each rail company is presented in Table ASSG1-1 and is consistent for both the "Status Quo" routing as well as the "Consolidated Corridor" condition.

#### THROUGH-TRAIN MOVEMENTS PER PEAK DAY

RAIL COMPANY	2020
AT&SF	24
Southern Pacific	41
Union Pacific	_34
<b>Total Train Movements</b>	99

TABLE: ASSG1-1 Through-train movements by rail company.

This analysis presents a comparison of vehicle/train conflicts for the "Status Quo" vs. the "Consolidated Corridor" routing of through train movements. The "Consolidated Corridor" addresses all Alameda alternatives as equal when analyzing through-train movements. However, one of the key differences with the alternatives occurs in the handling of the rail traffic for industry service along the Alameda Corridor. Under the "Consolidated Corridor", a drill track<sup>1</sup> is necessary to accommodate the rail traffic for industry service. The Alameda Corridor alternatives which incorporate an at-grade railroad provide a mitigation for potential vehicular/railroad conflicts associated with the train operations on the drill track because all rail crossings by cross street traffic are eliminated through grade separations or closures . Under the Alameda Corridor depressed railroad alternatives, the drill track remains at-grade and cross street traffic which is accommodated with grade separations (bridges over the depressed main line) still results in travel across the at-grade drill track. Thus, a potential conflict of vehicles ana trains occurs. These differences in vehicle/railroad conflicts are also included in this analysis. The following table presents the daily trains projected for industry service in the Year 2020.

<sup>&</sup>lt;sup>1</sup>Capacity requirements as defined in Volume 4 - Railroad Capacity and Operations Analysis identifies a two track main line for through train movements with a dedicated drill track to provide for industry service.

### PLANNING YEAR

#### TYPE OF TRAIN

<u>2020</u>

Daily industry service<sup>2</sup> along Consolidated Corridor

10

TABLE:ASSG1-2Daily industry service along Alameda Corridor.

The potential conflicts of vehicles and trains at grade crossings are quantified in the same fashion as the through-train conflicts:

ADT \* No. of Trains = vehicle/railroad conflict index

<sup>&</sup>lt;sup>2</sup>Estimated as a typical day's activity for industry service along Alameda Corridor occurring on the drill track only.

#### SOUTHERN PACIFIC - GRADE X-INGS. ADT'S, TR. & ADT & TRAINS PER PEAK DAY

INT. <u>No.</u> 1		ria <u>Des</u>	GRADE X-ING STREET NAME	2020 <u>ADT</u>	18	2020 ADT X TR PER DAY	INT. <u>No.</u> (	æg	RR DESC	GRADE X-ING <u>STREET NAME</u>	2020 ADT	18	2020 ADT X TR PER DAY
			PASADENA JUNCTION TO J	- YARD (UPF	R)		18	24	BG2	FERNWOOD AV	4270	31	132370
				ANGUNU A HUND			17	24	BG2	IMPERIAL HWY	14820	31	459420
1	30	3	PERRINO PL	470		4700	18			LYNWOOD AV	12820		397420
2	30	3	SANTA FE	37560	1000	375600	19			BUTLER ST	410	1000	12710
3	30	з	WASHINGTON	30910	10	309100	20			WEBER AV	3340		103540
			Total ADT x Trains/Day:			689,400	21 22			EL SEGUNDO PINE ST	26750 10970		829250 340070
			WILMINGTON BRANCH				23			ROSECRANS	30250 (		0
							24			ELMST	2300		106100
1	29	BBH1	ALAMEDA W	37720	3	113160	25			PALMER AV	4580		215260
2	29		24TH ST	9960	3	29940	26	22	BG2	COMPTON BL	35190	47	1653930
3	29		LNG BCH AV E	8760	7	61320	27	_		LAUREL ST	1530		71910
4	29		M.L. KING BL	1220	7	8540	28			ALONDRA AV	22100		1038700
5	29		41ST ST	15150	3	45450	29				15270		717690 0
6	29 29		48TH ST	18940 1690	3	56820 5070	30 31			DEL AMO BL DOMINGUEZ	31990 ( 12660		595960
8	29		55TH ST	9380	3	28140	32			CARSON ST	14540 0		0
9	29		SLAUSON AV	42450	3	127350	33		BG3	SEPULVEDA	26850		778650
10	27		BOTH ST	2440	19	46360	34	8	BG3	PCH HWY	50890	29_	1475810
11	27	BBH2	GAGE AVE	31000	19	569000				Total ADT x Trains/Day:		_	15,457,700
12	27		FLORENCE AV	44580		647020							
13	27		NADEAU ST	19550		371450			LAHA	ABRA BRANCH			
14 15	27 27		92ND ST 97TH ST	12520	S. S	237880 174040	1	32	88J	PASSONS BL	15020	18	240320
18			103RD ST	22900		435100	2		BBJ	SERAPIS AV	7030		112480
17			108TH ST	7490		142310	3				26420		422720
18	25	88H3	WILMINGTON AV	15270	19	290130	4	32	BBJ	PARAMOUNT	16490	18	263840
19	25		IMPERIAL HWY	42790	19	813010	5			TELEGRAPH	30100		481600
20			119TH ST	3970		75430	6		BBJ		22900		366400
21	_		124TH ST	1380		26220	7		BBJ	GRNWD AV	2300		36800
22 23	0.000		EL SEGUNDO BL 130TH ST	32620 610		623680	8		BBJ	GARFIELD EASTERN	29610 27390		473760 438240
24			STOCKWELLAV	5810		11590 110390	10		BBJ		3830		61290
25			ROSECRANS AV	39250	0	0	11			HELIOAV	4650		74400
26			ELM ST	2690	0	Ō	12		BBJ	KING AV	1990		31840
27	23	BBH3	COMPTON BL	29000	0	0	13		BBJ	ATLANTIC	36810	16	568960
28			PALMST	4310	0	0	14		BBJ	PINE	4440		71040
29 30			LAUREL ST	2580	0	0	15		BBJ	GIFFORD ST	5240		83640
31			MYRAH ST INDIGO ST	1310 900	0	0	16 17		BBJ BBJ	CARMELITA MAYWOOD AV	1980 12400		31680 198400
32			ALONDRA BL	27150	ō	ŏ	18		BBJ	STATE ST	21660		346880
33			GREENLEAF BL	9620	ō	o	19			RANDOLPHST	5930		94880
34	23	BBH3	MANVILLE ST	310	0	0	20		BBJ	ARBUTUS	2190		35040
35	23	BBH3	ALAMEDA ST	14040	0	0	21		BBJ		26120	16	417920
			Total ADT x Trains/Day:			5,289,300	22			SEVILLE	7040		112640
							23			RITA AVE	1930		30880
			SAN PEDRO BRANCH				24 25			PACIFIC BLVD RUGBY AVE	20920 3360		334720 53760
1	28	BG1	25TH ST	9980	17	169560	26			MALABAR	4250	16	66000
2	28		ALAMEDAE	12210		207570	27		88J	SANTA FE		16	385920
3	28	BG1	41ST ST	15150	17	257550	28	32	BBJ		1420		22720
- 4	28		VERNON AV	22260	17	378760	29		88J	REGENT ST	410	16	6560
5	28		51ST ST	790		13430	30		BBJ	ALAMEDA W	30300		484800
6 7	28 28		55TH ST SLAUSON AV	9380		159460	31	32		WILMING AV	14650		234400
á			RANDOLPH ST	42600		727600 174930	32	32	88J	HOLMES AV Total ADT x Trains/Day:	11300	16	180800
-			GAGE AVE	26120		444040				, san rari a tiang day.			6,787,520
10			FLORENCE AV	51040		867680				PUENTE/SANTA ANA BRANCH			
11			NADEAU AV	29370		499290							
12			FIRESTONE	41230		1278130	1	34	BK	PIONEER	19700	16	315200
13			SOUTHERN AV	12630		391530			BK	LOS NIETOS	10230		153450
14			TWEEDY BL	16370	31	507470	3	34	BK	SMITH AV	2380		35700
15	24	BG2	CENTURY BL	14510	31	449810	4		BK	FLORENCE	14600		219000
							5	34	BK	ORR& DAY	650	15	9750
										(CONTINED PAGE 2)			

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#### SOUTHERN PACIFIC - GRADE X-INGS, ADT'S, TR. & ADT x TRAINS PER PEAK DAY

INT.

123456

RR GRADE X-ING

INT. <u>No.</u> 5	<u>æg</u>	rir <u>Des</u>	GRADE X ING STREET NAME PUENTE/SANTA ANA BRANCH (		IR D)	2020 ADT x TR PER DAY
6	33	BK	STUDEBAKER	18660	15	279900
7	33	BK	HOXIE AV	160	15	2400
8	33	BK	REGENTVAV	3660	15	54900
9	33	BK	STEWARD/GR	12820	15	192300
10	33	BK	WOODRUFF	21380	15	320700
11	33	BK	LAKEWOOD	51910	15	778650
12	33	BK	PATTON	12210	15	183150
13	33	BK	BROOKSHR	12990	15	194850
14	33	BK	DOLAN AV	560	15	8400
15	33	BK	DOWNEY AV	14490	15	217350
16	33	BK	PARAMOUNT	37720	15	565600
17	33	BK	RIVES AV	12210	15	183150
18	33	BK	FIRESTONE	65810	15	987150
19	33	BK	GARFIELD	36810	15	552150
20	33	BK	ATLANTIC	38170	15	572550
21	33	BK	OTIS ST	18330	15	274950
22	33	BK	SAN JUAN	4580	15	68700
23	33	BK	CALIF AVE	16790	15	251850
24	33	BK	STATE ST	18330	15	274950
25	33	BK	LNG BCH BLVD	27950	15	419250
26	33	BK	SANTA FE AV	13600	15	204000
			Total ADT x Trains/Day:			7,320,200

No.S	EG	DE	SG STREET NAME	ADT	TR	TR PER DAY
12	39	в	ARDEN	18630	12	223560
13	39	в	BALDWIN	35550	12	426600
14	39	в	TEMPLE CITY	30540	12	366480
15	39	в	LOWER AZUSA	16790	12	201480
16	39	8	ENCINITA	7940	12	95280
17	39	в	WALNUT GROOVE	6880	12	82560
18	39	в	SAN GABRIEL	46660	12	559920
19	39	B	DEL MAR	20270	12	243240
20	30	в	MISSION	22160	12	265920
21	39	В	RAMONA	24200	12	290400
22	39	8	VALLEY	29780	12	357360
23	39	в	BOCA	8860	12	106320
24	39	в	VINEBURN	3090	12	37080
25	39	B	SAN PABLO	12070	12	144840
			Total ADT x Trains/Day:		-	6,104,040
			BARTOLO JCT. TO INDUSTRY	(UPRR)		

2020

2020 ADT x

38	3	STIMSON	12070	31	374170
38	3	TURNBULL	15860	31	492260
36	3	7TH AVENUE	34960	31	1084380
38	3	WORKMAN MILL	22460	31	696260
38	3	MISSION MILL	2340	31	72540
38	3	ROSE HILLS	8800	31	272800
		Total ADT x Trains/Day:		the second s	2,992,430

#### WEST LINE (PASADENA JCT. TO BARTOLO JUNCTION)

1	41	в	CALIFORNIA	8380	12	100560
2	41	в	SUNSET	24480	12	293760
з	41	в	ORANGE	5550	12	66600
4	41	в	PUENTE	23070	12	276840
5	41	в	VINELAND	9190	12	110260
6	41	в	TEMPLE	27280	12	327360
7	41	в	COGSWELL	8550	12	102600
8	41	в	PECK	47130	12	565560
9	41	в	RAMONA	47330	12	567960
10	41	в	CYPRESS	5660	12	67920
11	41	B	TYLER	18630	12	223560

#### TOTAL SPTC ADT x TRAINS/DAY 44,620,590

SEG = RR Mainline Segment RR DES = Railroad Mainline Branch Number ADT = Average Daily Traffic Volume TR = Trains per Day

GS = Grade Separation

## EXHIBIT: ATG1-1 ADT x TRAINS PER DAY "STATUS QUO 2020" - SPTC

#### SANTA FE (AT&SF RR) - GRADE X-INGS, ADT'S, TR, ADT & TRAINS PER DAYS PER PEAK DAY

INT	RA	GRADE X-ING	2020	2020 ADT x	INT	88	GRADE X-ING	2020		2020 ADT x
No.SEG		STREET NAME	ADT 1				STREET NAME		TR	TR PER DAY
110.020	04.0	ATSF HARBOR SUBDIVISH			110.01.0	01.0	On the I would	1021		
1 42	2H1	HARRIET ST	690		51 43	2H3	IVY AV	2,360	24	56640
2 42		25TH ST	840		52 43		EUCALYP. AV	14,810		355440
3 42		26TH ST	14,110		53 43		N. CEDAR	1,250		30000
4 42		27TH ST	1,830		54 43		OAK ST	2,290		54960
5 42		28TH ST	310		55 43		HYDE PK BL	4,940		118560
6 42		37TH/38TH ST	19,440		56 43		LA CIEN, BL	29,080		697920
7 42		E.VERNON AV	14,970		57 43		HINDRY AV	4,580		109920
8 42		PACIFIC BLVD	27,360		58 43		MANCH. AV	49,320		1183660
9 42		49TH ST	3,050		59 43		ARBOR V.	22,760		546240
10 42		FRUITLAND AV	8,090		60 43		104TH ST	15,480		371520
11 42		52ND ST	970		61 43		111THST	2.350		56400
12 42		53RD ST	1,660		62 43		IMPERIAL HWY	76,490		1835760
13 42		54TH ST	850		63 43		118TH ST	6,460		155040
14 42		55TH ST	5,130		64 43		120TH ST	2,810		62640
15 42		56TH ST	990		65 43		DOUGLAS ST	19,050		457200
18 42		57TH ST	1.420		66 1		COMPTON AV	24,260		582240
17 42		58TH ST	1,290		67 1		INGLEWOOD	36,690		926560
18 42		SANTA FE AV	27,050		68 1		MANHATTAN BL	2,900		69600
19 42		2ND ST	2,290		69 1	-	159TH ST	770		18480
20 42		ALAMEDA E/W	32,680		70 1		160TH ST	770		18480
21 42		HOLMES AV	1,100		71 1		161ST ST	770		18480
22 43		LONG BCH W.	9,440		72 1		162ND ST	2,290		54960
23 43		COMPTON AV	17,450		73 1		170TH ST	2,290		54960
23 43		HOOPERAV	12,190		74 1		182ND ST	18,660		447840
25 43		CENTRAL AV	23,890		75 1		TORRANCE BL	38,120		914680
26 43		MC KINLEY AV	6,020		76 1		SONOMAST	1,850		44400
27 43		PALOMA AV	930		77 1		CARSON ST	37,120		890880
28 43		AVALON BL	23.540		78 1		WASHINGTON AV	1,170		26060
29 43		TOWNEAV	2,710		79 1		ARLING, AV	21,520		516480
30 43		SAN PEDRO ST	10,480		80 1		CABRIL AV	11,210		269040
31 43		S. MAIN ST	19,270		61 1		BORDER AV	1,860		44640
32 43		S. BROADWAY	31,420		62 1		SEPULVEDA	55,690		1336560
33 43		FIGUEROA	34,400		63 1		WESTERN AV	38,960		935520
34 43		HOOVERST	18,950		84 1		S. FIGUEROA	13,800		331200
35 43		VERMONTAV	31,160		85 1		AVALON BL	18,760		450240
36 43		BUDLONG AV	5,880		86 1		BROAD ST	1.640		39360
37 43		NORMANDIE AV	29,560	The second se	87 1		LAKME ST	2,240		53760
38 43		DENKER AV	10,360		88 1		WILMIN. AV	18,640		399360
		SLAUSON AV	39,860		69 2	1000 1000	LOMITA	26,030		624720
40 43		WESTERNAV	35,460		90 3		PCH	43,620		1046880
41 43		VAN NESS	17,620			2110	Total ADT x Trains/Day:	10,020	-	32,543,760
42 43		4TH AV	3,620				toutriot a manapag.		-	01,010,00
43 43		8TH AV	25,400		ATSE SAM	REP	NARDINO SUBDIV HOBAR	TTOOT I	NCTION	
44 43		11TH AV	2,960							
45 43		67THST	4,970		1 36	2	PIONEER BLVD.	19,690	24	472560
46 43		CRENSHAW BL	35,080		2 36		PASSONS BLVD	16,340		392160
47 43		VICTORIAAV	1,140		3 36		SERAPIS AVE	6.040		144960
48 43		BRYNHURST AV	1,050		3 30	2	Total ADT x Trains/Day:	0,040	*T	1,009,680
		CENTINELA	25,070				TOTAL ADT & HERBIDIDELY.			1,009,000
		ULI CI III LLI	20.010	LT UU100U						
49 43		LABREA ST								
49 43 50 43		LA BREA ST	34,110		TOTAL	TOP	ADT x TRAINS/DAY			33,553,440

SEG = RR Mainline Segment RR DES = Railroad Mainline Branch No.

ADT = Average Daily Traffic Volume

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TR = Trains per Segment

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## EXHIBIT: ATG1-3 ADT x TRAINS PER DAY "STATUS QUO 2020" - UPRR

5 7 8 9 10 11	38 37 37 37 37 37	3 3 3 3 Tot	DURFEE MONTEBELLO GREENWOOD MAPLE VAIL al ADT x Trains/Day: TAL UPRR ADT x TRAINS/DAY	14600 14200 12800 10300 16100	34 34 34 34 34 34	496400 482800 435200 350200 615400 5,304,000 29,219,600
6 7 8 9 10	37 37 37 37	3 3 3 3	DURFEE MONTEBELLO GREENWOOD MAPLE VAIL	14600 14200 12800 10300	34 34 34 34	496400 482800 435200 350200 615400
6 7 8 9 10	37 37 37 37	3 3 3 3	DURFEE MONTEBELLO GREENWOOD MAPLE	14600 14200 12800 10300	34 34 34 34	496400 482800 435200 350200
6 7 8 9	37 37 37	3 3 3	DURFEE MONTEBELLO GREENWOOD	14600 14200 12600	34 34 34	496400 482800 435200
6 7 8	37 37	3 3	DURFEE MONTEBELLO	14600 14200	34 34	496400 482800
6 7	37	з	DURFEE	14600	34	496400
6						
	38					
		3	ROSE HILLS	6900	34	234600
5	38	3	MISSION	21400	34	23800
4	38	3	WORKMAN MILL	21400	34	727600
3	38	3	7TH AVENUE	33400	34 34	411400
1	38 38	3 3	STIMSON TURNBULL	11500 12100	34	391000 411400
				44500		001000
UNK		CIEK	MAIN TRACKS EAST L.A.TO CITY OF INDUSTRY		1	
		Tot	al ADT x Trains/Day:			23,915,600
33	14	3A	WARDLOW RD	17500	34	595000
32	14	3A	CARSON ST	300	34	10200
31	14	3A	CANDLEWOOD	21300	34	724200
30	14	3A	SOUTHST	32600	34	1106400
29	14	34	ARTESIA BL	23900	34	812600
28	14	3A	JACKSON ST	12400	34	421600
27	14	3A	ALONDRA BL	23900	34	812600
25	14	3A	JEFFERSON AV	3500	34	119000
24	14	3A 3A	COMPTON BL	21300	34	724200
23 24	14	3A 3A	CENTURY BL ROSECRANS AVE	4600 37900	34 34	156400 1288600
22	14	3A	MAIN ST	8400	34	285600
21	14	34	GARDNDALE AV	9400	34	319600
_	14	3A	GARFIELD AV	19700	34	008698
20	14	ЗA	IMPERIAL HWY	72800	34	2475200
19 20				1500	34	51000

SEG = RR Mainline Segment RR DES = Railroad Mainline Branch No. ADT = Average Daily Traffic Volume TA = Trains per Segment

#### UNION PACIFIC - GRADE X-INGS, ADT'S, TR. & ADT x TR PER PEAK DAY

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2020 ADT x

2020

INT.		RR	GRADE X-ING	2020		2020 ADT x
No	<u>seg</u>	DES	STREET NAME	<u>ADT</u>	TR	TR PER DAY
		UNK	DN PACIFIC - SAN PEDRO BRANCH EAST L.A. T	O THENARD		
1	14	ЗA	26TH ST	6600	34	224400
2	14	3A	BANDINI BLVD	36700		1247800
3	14	34	CHARTER ST	2800	-	95200
4	14	34	EXCHANGE AV	4500	-	153000
5	14	34	FRUITLAND BL	21400		727600
6	14	3A	SLAUSON AV	42900		1458600
7	14	3A	DISTRICT BLVD	23000		782000
8	14	3A	RANDOLPH ST	11800		401200
	14	34	GAGE AVE	32500		1105000
10	14	34	BELLAVE	4200		142800
11	14	3Å	FLORENCE AVE	62800		2135200
12	14	3A	OTIS ST	15800		537200
13	14	3A	SANTA ANA	11500	-	391000
14	14	3A	ABDINE ST	1500		51000
15	14	3A	ATLANTIC AVE	38200	-	1296800
18	14	3A	FIRESTONE BL	67000		2276000
17	14	34	RAYO AVE	9200		312800
18	14	34	SOUTHERN AVE	1500		51000
19	14	3A	IMPERIAL HWY	72800	34	2475200
20	14	3A	GARFIELD AV	19700		669800
21	14	3A	GARDNDALE AV	9400		319600
22	14	3A	MAIN ST	6400		265600
23	14	3A	CENTURY BL	4600	34	156400
24	14	3A	ROSECRANS AVE	37900	34	1288600
25	14	3A	COMPTON BL	21300		724200
26	14	3A	JEFFERSON AV	3500		119000
27	14	3A	ALONDRA BL	23900	34	812600
28	14	3A	JACKSON ST	12400	34	421600
29	14	3A	ARTESIA BL	23900	34	812600
30	14	3A	SOUTHST	32600	34	1108400
31	14	3A	CANDLEWOOD	21300	34	724200
32	14	3A	CARSON ST	300	34	10200
33	14	3A	WARDLOW RD	17500	34	595000
		Tota	I ADT x Trains/Day:		,	23,915,600
UNIC	ON PA	CIFIC	MAIN TRACKS EAST L.A. TO CITY OF INDUSTRY			
1	38	з	STIMSON	11500	34	391000
2	38	3	TURNBULL	12100	34	411400
з	38	3	7TH AVENUE	33400	34	1135800
4	38	з	WORKMAN MILL	21400	34	727800
5	38	з	MISSION	700	34	23800
6	38	з	ROSE HILLS	6900	34	234600
7	37	з	DURFEE	14600	34	496400
8	37	3	MONTEBELLO	14200	34	482800
9	37	3	GREENWOOD	12800	34	435200
10	37	3	MAPLE	10300	34	350200
11	37	3	VAIL	18100	34	815400

INT.

RR GRADE X-ING

#### CONSOLIDATED CORRIDOR - GRADE X-INGS, ADT'S, TR, & ADT & TRAINS PER PEAK DAY

INT	RR GRADE X-ING	2020		2020 ADT x	INT	RR GRADE X-ING	2020		2020 ADT x
No. SEG	DES STREET NAME	ADT	TR	TR PER DAY	No. SEG	DES STREET NAME	ADT	TR	TR PER DAY
	ALAMEDA CORRIDOR DRILL TRACK (SEE NOTE 1)				23 22	BG2 ROSECRANS	30250	10	302500
						(FUTURE GRADE SEPARATION BY OTHERS)			
1 28	BG1 25TH ST	9980	10	99800	24 22	BG2 ELM ST	2300	10	23000
	(FUTURE GRADE SEPARATION)					(FUTURE RR CROSSING CLOSURE)			
2 28	BG1 ALAMEDA E	12210	10	122100	25 22	BG2 PALMERAV	4580	10	45800
	(FUTURE GRADE SEPARATION)					(FUTURE RR CROSSING CLOSURE)		in the second	
3 28	BG1 41ST ST	15150	10	151500	26 22	BG2 COMPTON BL	35190	10	351900
	(FUTURE GRADE SEPARATION)				-	(FUTURE GRADE SEPARATION)	4500	40	45000
4 28	BG1 VERNON AV	22280	10	222800	27 22		1530	10	15300
5 00	(FUTURE GRADE SEPARATION)	700	10	7900	00.00	(FUTURE RR CROSSING CLOSURE)	22100	10	221000
5 28	BG1 51ST ST (FUTURE RR CROSSING CLOSURE)	790	10	7900	28 22	BG2 ALONDRA AV (FUTURE GRADE SEPARATION)	22100		221000
6 28	BG1 55TH ST	9380	10	93800	29 22	BG2 GREENLEAF	15270	10	152700
0 20	(FUTURE RR CROSSING CLOSURE)	2000	10	00000	LJ LL	(FUTURE GRADE SEPARATION)	TOLIO		TOLTO U
7 28	BG1 SLAUSON AV	42800	10	428000	30 21	BG3 DELAMO BL	31990	10	319900
. 20	(FUTURE GRADE SEPARATION)	12000				(FUTURE GRADE SEPARATION BY OTHERS)			
8 28	BG1 RANDOLPH ST	10290	10	102900	31 21	BG3 DOMINGUEZ	12680	10	126800
	(FUTURE RR CROSSING CLOSURE)					(FUTURE RR CROSSING CLOSURE)			
9 26	BG1 GAGE AVE	26120	10	261200	32 21	BG3 CARSON ST	14540	10	145400
	(FUTURE GRADE SEPARATION)					(FUTURE GRADE SEPARATION BY OTHERS)			
10 26	BG1 FLORENCE AV	51040	10	510400	33 19	BG3 SEPULVEDA	26850	10	268500
	(FUTURE GRADE SEPARATION)					(FUTURE GRADE SEPARATION)			
11 26	BG1 NADEAU AV	29370	10	293700	34 6	BG3 PCH HWY	50890	10	508900
40.04	(FUTURE GRADE SEPARATION)		40	440000		(FUTURE GRADE SEPARATION)			
12 24	BG2 FIRESTONE	41230	10	412300	TOTAL				6,357,000
13 24	(FUTURE GRADE SEPARATION) BG2 SOUTHERN AV	12630	10	126300	IUIALA	DT x TRAINS PER DAY ALTS 2.1A, 2.2, 2.3, 2.4, 3.0, 6.1, 6.	2, 0.3, 0.4	-	0,337,000
10 24	(FUTURE RR CROSSING CLOSURE)	12000	10	120300	TOTAL A	DT x TRAINS PER DAY ALTS 1.0, 5.0 (SEE NOTE 1)			0
14 24	BG2 TWEEDY BL	16370	10	163700				-	
	(FUTURE GRADE SEPARATION)								
15 24	BG2 M.L. KING	14510	10	145100	ATSF SAI	N BERNARDINO SUBDIV HOBART TO DT JUNCTION			
	(FUTURE RR CROSSING CLOSURE)								
16 24	BG2 FERNWOOD AV	4270	10	42700	1 36	2 PIONEER BLVD	19690		472560
	(FUTURE RA CROSSING CLOSURE)				2 36	2 PASSONS BLVD	16340		392160
17 24	BG2 IMPERIAL HWY	14820	10	148200	3 36	2 SERAPIS AVE	6040	) 24_	144960
19 04	(FUTURE GRADE SEPARATION) BG2 LYNWOOD AV	40000		400000	TOTAL	DY TRUMA DED DAV			4 000 000
18 24	(FUTURE RR CROSSING CLOSURE)	12820	10	128200	IUIALA	DT x TRAINS PER DAY		-	1,009,680
19 24	BG2 BUTLER ST	410	10	4100					
10 21	(FUTURE RR CROSSING CLOSURE)	410	10	4100					
20 24	BG2 WEBERAV	3340	10	33400					
	(FUTURE GRADE SEPARATION)	0010	10	00400					
21 24	BG2 EL SEGUNDO	26750	10	267500					
	(FUTURE GRADE SEPARATION)	201.00		201000					
22 24	BG2 PINE ST	10970	10	109700					
	(FUTURE AR CROSSING CLOSURE)		5	10/5/5/7 17 7					
									2

NOTE 1: AT-GRADE DRILL TRACK X-INGS IN ALTS. 2.1A, 2.2, 2.3, 2.4, 3.0, 6.1, 6.2, 6.3, 6.4 ONLY. ALTS. 1.0, 5.0 HAVE NO AT-GRADE DRILL TRACK X-INGS.

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EXHIBIT: ATG1 - 4 ADT x TRAINS PER DAY "CONSOLIDATED CORRIDOR 2020"

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#### CONSOLIDATED CORRIDOR - GRADE X-INGS, ADT'S, TR. & ADT & TRAINS PER PEAK DAY

INT No. SEG		GRADE X-ING STREET NAME IE - PASADENA JCT. TO INDUSTRY	2020 <u>ADT</u>	TR	2020 ADT x TR PER DAY
SFIC WE	.91 LIN	E - FASADENA JOI, TO INDUSTRI			
1 41	в	CALIFORNIA	8380	41	343580
2 41	8	SUNSET	24480	41	1003680
3 41	В	ORANGE	5550	41	227550
4 41	8	PUENTE	23070	41	945870
5 41	8	VINELAND	9190	41	376790
6 41	8	TEMPLE	27280	41	1118480
7 41	B	COGSWELL	8550	41	350550
8 41	B	PECK	47130	41	1932330
9 41	В	RAMONA	47330	41	1940530
10 41	В	CYPRESS	5660	41	232060
11 41	B	TYLER	18630	41	763830
12 39	B	ARDEN	18630	41	763830
13 39	B	BALDWIN	35550	41	1457550
14 39	В	TEMPLE CITY	30540	41	1252140
15 39	B	LOWER AZUSA	16790	41	688390
16 39	B	ENCINITA	7940	41	325540
17 39	В	WALNUT GROVE	6880	41	282080
18 39	8	SAN GABRIEL	46660	41	1913060
19 39	B	DELMAR	20270	41	831070
20 39	В	MISSION	22160	41	908560
21 39	B	RAMONA	24200	41	992200
22 39	B	VALLEY	29780	41	1220980
23 39	В	BOCA	8860	41	363260
24 39	B	VINEBURN	3090	41	126690
25 39	в	SAN PABLO	12070	41	494670
TOTALA	DTxl	TRAINS PER DAY			20,855,470

INT	eco	RR	GRADE X-ING STREET NAME	2020 ADT	18	2020 ADT x TR PER DAY
	<u>Seg</u> on p	DES	MAIN TRACKS EAST LA. TO CITY OF INDUSTRY	<u> </u>	10	Intenent
- 1	38	3	STIMSON	11500	34	391000
2	38	3	TURNBULL	12100	34	411400
3	38	3	7TH AVENUE	33400	34	1135600
4	38	3	WORKMAN MILL	21400	34	727600
5	38	3	MISSION	700	34	23800
6	38	3	ROSE HILLS	6900	34	234600
7	37	3	DURFEE	14600	34	496400
8	37	3	MONTEBELLO	14200	34	482800
9	37	3	GREENWOOD	12800	34	435200
10	37	3	MAPLE	10300	34	350200
11	37	3	VAIL	18100	34 _	615400
TOT	TAL A	DTxl	TRAINS PER DAY		_	5,304,000

TOTAL ADT x TRAINS PER DAY ALTS 2.14, 2.2, 2.3, 2.4, 3.0, 6.1, 6.2, 6.3, 6.4	28,178,830
TOTAL ADT x TRAINS PER DAY ALTS 1.0, 5.0 (SEE NOTE 1)	27,169,150

SEG = RR Mainline Segment RR DESG = Railroad Mainline Branch No. ADT = Average Daily Traffic Volume TR = Trains per Day

EXHIBIT: ATG1 - 4 ADT x TRAINS PER DAY "CONSOLIDATED CORRIDOR 2020"

	STATUS QUO ADT x TR/DAY	CORRIDOR	REDUCTION IN ADT x TR/DAY CORRIDOR v. STATUS QUO	
SOUTHERN PACIFIC				
PASADENA JCT TO J-YARD (UPRR) WILMINGTON BRANCH SAN PEDRO BRANCH LA HABRA BRANCH PUENTE/SANTA ANA BRANCH WEST LINE (PASADENA TO BARTOLO JCT.)	689400 5269300 15477700 6787520 7320200 6104040 0000420	0 0 0 20855470	689400 5269300 15477700 6787520 7320200 14751430 *INCREASE IN ADTxTR/0 2992430	YAX
BARTOLO TO INDUSTRY (UPRR)	2992430	Ŭ	2332430	
SAN PEDRO BRANCH EAST L.A. TO INDUSTRY ACTHISON, TOPEKA AND SANTA FE	23915600 5304000	0 5304000	23915600 0	
			ν.	
HARBOR SUBDIVISION HOBART TO DT JUNCTION	32543760 1009680	0 1009680	32543760 0	
CONSOLIDATED CORRIDOR				
PORTS TO DOWNTOWN CONNECTIONS ALTS. 1.0, 5.0 ALTS. 2.1A, 2.2, 2.3, 2.4, 3.0, 6.1, 6.2, 6.3, 6.4	N/A N/A	0 6357000		
TOTAL ADT x TRAINS PER DAY ALTS. 1.0, 5.0 ALTS. 2.1A, 2.2, 2.3, 2.4, 3.0, 6.1, 6.2, 6.3, 6.4	107413630	27169150 33526150	80244480 73887480	
REDUCTION IN ADT & TRAINS PER DAY, ALTS. 1.0,	5.0 v. STATUS QUO	·	80,244,480	
REDUCTION IN ADT & TRAINS PER DAY, ALTS. 2.1A	, 2.2, 2.3, 2.4, 3.0, 6.1, 6	.2, 6.3, 6.4 v. STATUS Q	JO 73,887,480	

## EXHIBIT ATG1 – 5: REDUCTION ADT x TRAINS PER DAY SUMMARY, CORRIDOR v. STATUS QUO, YEAR 2020

PREPARED BY: DMJM/MN-WJM DATE: 9/09/91 REV: 10/2/91

#### GOAL ANALYSIS - TECHNICAL MEMORANDUM

#### GOAL: Safety/Security Goal No. 2 - Improve Safety for Pedestrians

## **INTRODUCTION:**

This memorandum presents a comparative evaluation of Alameda Corridor alternatives relative to their ability to provide an Improvement of Safety for Pedestrians from railroad operations. This information is intended to be incorporated into a larger matrix that compares alternatives to improve Alameda Street Corridor for both rail and vehicular traffic.

The measurement criteria used to quantify pedestrian safety is the <u>Reduction in Trains per</u> Day x Grade Crossings.

#### **METHODOLOGY:**

Significant railroad hazards to pedestrians result from exposure at At-Grade crossings. Even with crossings fully protected with gates and flashers (as proposed for the Corridor and recommended by the California Public Utilities Commission) pedestrians and motorists will chose to ignore the warning and walk into the path of an oncoming train.

A quantitative comparison of the alternatives can be made by tabulating the number of grade crossings and multiplying by the number of trains expected to operate over those crossings. In order to quantify the effects on rail lines beyond the Corridor this criteria was tabulated for grade crossings from the Ports to the City of Industry (SPTC, UPRR) or D.T. Junction (ATSF). This tabulation is presented in Table SS2 - 1. It should be noted that there are no At-Grade crossings of Corridor main track in any of the Alternatives, however the Depressed Alternatives include an at-grade drill track which is included in the tabulations. It is this tabulation that is carried over into the Matrix Evaluation of the Alternatives.

#### **CONCLUSIONS:**

From Table SS2 - 1 it can be seen that Alternatives 1.0 and 5.0, the At-Grade Trainway present the least pedestrian exposure to At-Grade crossing hazards. The reasoning behind this is because the pedestrians will be encouraged to use the overcrossing structures to traverse Alameda. This travel path would be encouraged in areas where continuous sound walls would be constructed along the trainway. This tabulation is carried over into the Matrix Evaluation of Alternatives.

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#### **GENERAL OBSERVATIONS:**

The results of consolidating trains onto the Corridor trainway can have the effect of consolidating the "potential of pedestrian hazard" also. This in turn would automatically reduce the hazard exposure on the routes from which rail traffic was diverted. The effect of consolidating the trains and protecting the pedestrians with continuous fences and grade separations will largely eliminate pedestrian exposure to the Corridor trainway.

The Depressed Trainway should be less of an "attractive nuisance" to pedestrians attempting to cross Alameda. The Depressed Trainway will provide readily available crossings over the trainway at nearly all existing grade crossings. Also, pedestrian crossings over the Trainway can be readily installed where desirable. It will also be considerably difficult to get in and out of the Depressed Trainway. However, pedestrians may attempt to cross on utility bridges or spandrels. These facilities would be designed with property safeguards.

In that is an interesting and unusual structure, the Depressed Trainway may be more of an "attractive nuisance" to vandals and vagrants, particularly at the maintenance of way access ramps. This potential problem can be minimized by sufficient security patrols, lighting, and locked gates at the ramps.

Although it involves fewer grade crossings, the At-Grade Trainway Alternative is somewhat less amenable to the needs of pedestrians along Alameda. To utilize the over or under-pass structures, pedestrians will walk considerably out of their way east or west of Alameda St. Furthermore separate pedestrian crossings will necessitate large structures to be constructed involving stairs or long ramps along Alameda St.

Recent local experience on the Metro-Rail Blue Line is that the first year saw frequent, sometimes daily, cutting of the steel picket fence. The SCRTD followed a policy of A.S.A.P. repair of damaged fence panels. Recently vandalism of the fence has fallen to almost zero.

SEGMENT	GRADE X - INGS	TRAINS PER DAY	HAZARD	REDUCTION IN HAZARD PROJECT V. STATUS QUO
SOUTHERN PACIFIC	2			* k
PASADENA JCT. TO J YARD		0		
STATUS QUO	3	10	30	600.000 /
PROJECT	0	41	0	30
WILMINGTON BRANCH				
STATUS QUO	35	23	320	
PROJECT	35	0	0	320
SAN PEDRO BRANCH				
STATUS QUO	31	41	605	505
PROJECT	0	41	0	605
A HABRA BRANCH		2 × 2		
STATUS QUO	32	16	512	610
PROJECT	32	0	0	512
PUENTE/SANTA ANA BR's				
STATUS QUO	26	15	390	1
PROJECT	26	0	0	390
WESTUNE				
STATUS QUO	25	12	300	
PROJECT	25	41	1025	-725
BARTOLO JCT TO INDUSTRY		×		
STATUS QUO	6	31	186	<i>a</i>
PROJECT	6	41	246	-60
UNION PACIFIC				
SAN PEDRO BRANCH				ο.
STATUS QUO	32	34	1088	
PROJECT	32	0	0	1088
EAST LA. TO INDUSTRY				
STATUS QUO	11	34	374	· ·
PROJECT	11	34	374	0
AT & SF RY.				
ARBOR SUBDIVISION				
STATUS QUO	90	24	2160	
PROJECT	90	0	0	2160
OBART TO DT JUNCTION				2 ·
STATUS QUO	3	24	72	1
PROJECT	3	24	72	0
OTAL STATUS QUO	294	99	5663	1
OTAL PROJECT (MAINLINE)	260	99	1717	3946
		· · · ·		
CONSOLIDATED CORRIDOR				REDUCTION INCLUDING
ALT. 1.0, 5.0 MAIN TRACK	0	99	0	DRILL TRACK
DRILL TRACK	õ	8	ő	3946
ALT. 2.1, 3.0, 6.1		6400.0X		
	0	99	0	2822
DRILL TRACK ALT. 2.2, 6.2	21	8	124	3822
MAIN TRACK	0	99	0	
DRILL TRACK	21	8	124	3822
ALT. 2.3, 6.3 MAIN TRACK	0	99	o	
DRILL TRACK	17	8	104	3842
ALT. 2.4, 6.4	-		2 (200)	
MAIN TRACK DRILL TRACK	0 10	99	0	2865
DRILL INAUX	10	8	80	<u>3866</u>

TABLE SS2 - 1 PEDESTRIAN EXPOSURE TO AT-GRADE CROSSING HAZARDS

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PREPARED BY: DMJM/M&N - WJM DATE: 9/26/91 REV:10/2/91

#### GOAL ANALYSIS - TECHNICAL MEMORANDUM

# GOAL: Railroad Goal No. 1 - Improve Railroad Operating Efficiency and Flexibility

#### **INTRODUCTION:**

This memorandum presents a comparative evaluation of Alameda Corridor alternatives relative to railroad efficiency. This information is intended to be incorporated into a larger matrix that compares alternatives to improve Alameda Street Corridor for both rail and vehicular traffic.

The measurement used to quantify efficiency includes a tabulation of physical characteristics affecting route efficiency as follows: length of line, quantity of special trackwork, total curvature, total rise and fall, maximum gradients, quantity of at-grade RR crossings. Flexibility is measured by the number of route between two points and is also included in this tabulation.

#### **METHODOLOGY:**

An important project goal is the improvement of the overall rail operations in the study area. The improvement in overall operating efficiency of the Corridor network over the Status Quo network has been modelled by computer simulation<sup>1</sup>. The quantifying by computer simulation operating efficiency accounts for such factors as train and ship arrivals, dispatching efficiency and priorities, loading and unloading times, probabilities of equipment failure and the impacts of passenger trains receiving track priority. All of the rail alternatives are very similar and satisfy the general operational rail network requirements stipulated in and verified by the computer simulations. Also, the actual procedures and practices of train operations that will be instituted by the railroads are beyond the scope of this project, and therefore cannot be included in determining the efficiency of any Alternative. In order to quantitatively compare the rail alternatives a detailed tabulation of the <u>physical</u> features of the alternatives representing Route Efficiency was prepared. This tabulation is presented by Segment (note: no railroad work in Segment A) in Tables RR1 -1 and 2.

<sup>1</sup><u>Railroad Capacity and Operation Analysis, Alameda Consolidated Transportation</u> <u>Corridor Project</u> - Leachman and Associates, 1991.

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The short length of the corridor and the similarity of alternatives result in only minor differences when they are compared using conventional railroad quantitative parameters. The following measures of the physical characteristics of a railroad have been tabulated in order to define and compare the Route Efficiency of each alternative.

#### Length of Line:

Operating times tend to increase over a longer line. Construction and maintenance of way (M/W) cost will vary directly with length, i.e. more track to build and maintain.

#### Special Trackwork

Turnouts and at-grade crossing frogs are very expensive to install and maintain as compared to typical straight track. Also, special trackwork present a somewhat greater potential for operating inefficiencies as compared to typical track. In addition to the added construction and maintenance expense of the special trackwork required, at-grade crossings will tend to lower efficiency in that one train must wait for the other. It is assumed that Corridor traffic will be given priority in any crossing conflicts with other freight movements.

#### Total Curvature:

The total degree of curvature is a traditional measure of line "quality". Curves limit operating speeds and require somewhat increased maintenance. The impact of curves increases with degree or sharpness of the curve. The sharper curves also incur an increased potential for operating inefficiencies.

#### Total Rise and Fall

Total rise and fall is the sum of the differences between all summits and sags of a line. It is another traditional measurement of line quality. Its greatest effect is on operating and equipment cost, with minor impact on M/W cost. Note: the costs of the Depressed Trainway structure itself are compared elsewhere to costs of grade separation structures.

#### Maximum Gradients (Grades)

Greater gradient tends to increase operating costs, and to some degree M/W costs. lower construction cost.

#### Flexibility

Flexibility is defined as the number of routes available between two points. In the case of this study it is desirable to maintain and enhance, if possible, existing connections to the other rail lines in the area in order to maintain operational capacity should the Corridor incur an extended delay or maintenance operation. All alternatives maintain connections to the existing lines, and enhance overall access to the Ports. However, the Depressed Trainway Alternatives, 2.1, 2.2, 3.0, 6.1, and 6.2 which require the SPTC Santa Ana Branch to be connected to the Drill Track without access to the main track until the Artesia Fwy. are least flexible (subjective score 3). Depressed Alternatives 2.3 and 6.3, which require that the Santa Ana Branch be connected to the Drill Track but allows access to the Corridor Main tracks north of Rosecrans Ave. are somewhat more flexible and are given a subjective score 5). Note, Depressed Alternatives 2.4 and 6.4 are At-Grade at Firestone Blvd. and therefore allows the Santa Ana Branch the same access to Corridor Main Tracks as the At Grade Alternatives.

#### Alternatives Comparison

For the purposes of comparison, each characteristic of each Alternative was measured or subjectively ranked, as appropriate. These values where then normalized for each criteria and summed for each Alternative. These sums were then normalized and compared to determine the most desirable Alternative. Refer to Tables RR1 - 1 and RR1-2.

#### CONCLUSIONS:

By comparing the tabulation presented in TABLE RR1 - 1, it can be seen that from a railroad perspective all alternatives are very similar. However, it can be concluded that Alternatives 1.0 and 5.0, the At-Grade Trainway is the most desirable and Alternatives 2.2 and 6.2, which involve the Depressed Wilmington Diversion are the least.

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ITEM	AT-GRADE 1.0, 5.0		DEPRESSED 2.1, 3.0, 0	5.1	WILMINGTON DIV. 2.2,	6.2	DEPRESSED 2.3, 6.3		DEPRESSED 2.4, 6.4	
							х Э			
	ACTUAL	REL.		REL.		REL.		REL.		REL.
LENGTH	19.14 MILES	1.00	19.14 MILES	1.00	19.55 MILES	0.98	19.14 MILES	1.00	19.14 MILES	1.00
SPECIAL TRACKWORK	58 EACH	1.00	58 EACH	1.00	58 EACH	1.00	58 EACH	1.00	58 EACH	1.00
TOTAL CURVATURE	380 DEGS	1.00	380 DEGS	1.00	507 DEGS	0.75	380 DEGS	1.00	380 DEGS	1.00
TOTAL RISE AND FALL	262 FEET	1.00	322 FEET	0.81	322 FEET	0.61	322 FEET	0.81	322 FEET	0.81
MAXIMUM GRADE	1.5 %	1.00	1.5 %	1.00	1.5 %	1.00	1.5 %	1.00	1.5 %	1.00
FLEXIBILITY RATING	5	1.00	3	0.60	3	0.60	. 4	0.80	5	1.00
EVALUATION		6.00		5.41		5.14		5.61		5.81
NORMALIZED EVALUATION		1.00		0.90		0.86		0.94		0.97

TABLE RR1 - 1: RAILROAD ROUTE EFFICENCY TABULATION - SUMMARY B, C, & D

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ITEM	AT-GRADE 1.0, 5.0		DEPRESSED 2.1, 3.0,	8.1	WILMINGTON DIV. 2.2,	6.2	DEPRESSED 2.3, 6.3		DEPRESSED 2.4, 6.4	4
		1					2			
	ACTUAL	REL.	ACTUAL	REL.		REL. RANK	ACTUAL	REL.	ACTUAL	REL. RANK
LENGTH	5.59 MILES	1.00	5.59 MILES	1.00	6 MILES	0.93	5.50 MILES	1.00	5.59 MILES	1.00
SPECIAL TRACKWORK	BEACH	1.00	8 EACH	1.00	8 EACH	1.00	8 EACH	1.00	8 EACH	1.00
TOTAL CURVATURE	90 DEGS	1.00	90 DEGS	1.00	217 DEGS	0.41	90 DEGS	1.00	90 DEGS	1.00
TOTAL RISE AND FALL	110 FEET	1.00	140 FEET	0.79	140 FEET	0.79	140 FEET	0.79	170 FEET	0.65
MAXIMUM GRADE	0.8 %	1.00	1 %	0.80	1%	0.60	1 %	0.90	1 %	0.00
FLEXIBILITY	3	1.00	1	0.33	1	0.33	2	0.67	3	1.00
EVALUATION		6.00		4.72		4.07		5.05		5.25
NORMALIZED EVALUATION		1.00		0.79		0.68	00 g	0.84		0.87

TABLE RR1 - 1: RAILROAD ROUTE EFFICENCY TABULATION - SEGMENT B

ITEM	AT-GRADE 1.0, 5.0		DEPRESSED 2.1, 3.0, 0	9,1	WILMINGTON DIV. 2.2,	6.2	DEPRESSED 2.3, 6.3		DEPRESSED 2.4, 6.4	
		REL.		REL		REL.	ACTUAL	REL.	ACTUAL	REL
LENGTH	5.83 MILES	1.00	5.83 MILES	1.00	5.83 MILES	1.00	5.83 MILES	1.00	5.83 MILES	1.00
SPECIAL TRACKWORK	14 EACH	1.00	14 EACH	1.00	14 EACH	1.00	14 EACH	1.00	14 EACH	1.00
TOTAL CURVATURE	25 DEGS	1.00	25 DEGS	1.00	25 DEGS	1.00	25 DEGS	1.00	25 DEGS	1.00
TOTAL RISE AND FALL	55 FEET	1.00	85 FEET	0.65	85 FEET	0.65	85 FEET	0.65	55 FEET	1.00
MAXIMUM GRADE	0.4 %	1.00	1 %	0.40	1 %	0.40	1 %	0.40	0.4 %	1.00
FLEXIBILITY	1	1.00	1	1.00	1	1.00	. 1	1.00	1	1.00
EVALUATION		6.00		5.05		5.05	π.	5.05		6.00
NORMALIZED EVALUATION		1.00		0.64		0.84		0.84		1.00

TABLE RR1 - 1: RAILROAD ROUTE EFFICENCY TABULATION - SEGMENT C

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ITEM	AT-GRADE 1.0, 5.0		DEPRESSED 2.1, 3.0,	6.1	WILMINGTON DIV. 2.2,	62	DEPRESSED 2.3, 6.3		DEPRESSED 2.4, 6.	4
	ACTUAL	REL.	ACTUAL	REL.		REL.	ACTUAL	REL.	ACTUAL	REL.
LENGTH	7.72 MILES	1.00	7.72 MILES	1.00	7.72 MILES	1.00	7.72 MILES	1.00	7.72 MILES	1.00
SPECIAL TRACKWORK	38 EACH	1.00	38 EACH	1.00	38 EACH	1.00	36 EACH	1.00	36 EACH	1.00
TOTAL CURVATURE	265 DEGS	1.00	265 DEGS	1.00	265 DEGS	1. <b>0</b> 0	265 DEGS	1.00	265 DEGS	1.00
TOTAL RISE AND FALL	97 FEET	1.00	97 FEET	1.00	97 FEET	1.00	97 FEET	1.00	97 FEET	1.00
MAXIMUM GRADE	1.5 %	1.00	1.5 %	1.00	1.5 %	1.00	1.5 %	1.00	1.5 %	1.00
FLEXIBILITY	1	1.00	1	1,00	1	1.00	1	1.00	1	1.00
EVALUATION		6.00	0	6.00		6.00		6.00		6.00
		1.00		1.00		1.00		1.00		1.00

TABLE RR1 - 1: RAILROAD ROUTE EFFICIENCY TABULATION - SEGMENT D

PREPARED BY: MFA - GLP DATE: August 27, 1991 REVISED: September 26, 1991

### **GOAL ANALYSIS - TECHNICAL MEMORANDUM**

GOAL:

Environmental Goal No. 2 - Minimize Projected Air Pollution

#### Introduction

This memorandum presents a comparative evaluation of Alameda Corridor alternatives in light of air quality and energy consumption. This information is intended to be incorporated into a larger evaluation matrix that compares alternatives to improve the Alameda Street corridor for both freight rail and vehicular traffic in terms of a number of goals that were established at the outset of the project. The criterion by which air pollution was to be evaluated was tons/day of criteria pollutants.

#### Methodology

Estimates of daily criteria pollutant production were developed by applying emission factors to daily estimates of vehicle miles of travel (VMT) for each of four alternatives: (1) existing (1990), (2) null (year 2020), (3) a six-lane at-grade facility for both rail and vehicles (2020), (3) a 4-lane at grade roadway facility with an at-grade railroad facility, and (5) an alternative with six vehicular travel lanes and railroad in trench. The VMT estimates that were used in this exercise were taken from previous travel modeling that had been conducted by DKS Associates. In this previous work, the modeling alternatives were referred to as A, B, C, D and E, respectively, and therefore these references are also used in this memo.

In the modeling work that was previously done, estimates were provided for daily vehicular travel by trucks and all vehicles in a study area bounded by I-110 on the west, I-710 on the east, I-10 on the north, and the Ports on the south. Using this information, daily auto travel was derived as the remainder of total vehicle miles of travel minus truck travel. Moreover, the estimates were developed for three facility types; freeway, arterials in the study area, and Alameda Street itself. The modeling work also estimated a surrogate for speed by dividing total daily miles of travel by total daily hours of travel, also by the three facility types. This information is shown in Table 1.

To the information shown in Table 1 were applied emission factors for the following pollutants: carbon monoxide, reactive organic gases, nitrogen oxides, particulates, and sulfur oxides. All but sulfur oxides are produced by both autos and trucks. Sulfur oxide production by autos is negligible. The emission factors (see Table 2) were taken from <u>Air Quality Handbook for Preparing EIRs</u>, South Coast Air Quality Management District, 1987. The emission factors are expressed in grams per mile. For automobiles, they vary by speed, and they are available for

1990 and for a number of intervening years up to 2002. They are not published by the SCAQMD for years beyond 2002, and therefore the 2002 factors were used for the future year of 2020 in the comparison among alternatives. This would tend to overstate the effects uniformly for all alternatives. For trucks, diesel engine emission factors were used. These are not published for any year beyond 1987, and they do not vary by speed. Diesel rail locomotive emissions were not included in the analysis because they would be equal for all alternatives. The effects associated with vehicular delays at railroad crossings were also not included. These emissions will be addressed in the project environmental document as will emissions associated with locomotive operation. The equation used to estimate emissions was as follows:

#### Emissions = VMT (by vehicle & roadway type) X emission factor

Table 3 shows the results of the analysis, for each of the modeled alternatives, by each pollutant type. Also provided in the table is a summation across all pollutants. In reality this has little meaning, but it is here used as a simple indicator of the total quantity of pollutants generated under each of the alternatives.

Table 4 presents a summary of total pollutant burden for the alternatives being considered for the corridor. Alternatives 2.2, 2.3, and 2.4 were judged to be sufficiently similar to Alternative 2.1 such that the numerical values obtained for Alternative 2.1 would apply. Alternative 6.1, 6.2, 6.3, and 6.4 are the same as Alternatives 2.1 through 2.4, except that four lanes of highway are provided instead of six. Pollutant levels for these alternatives were calculated by applying the ratio of pollutant burden for Alternative 5 divided by pollutant burden for Alternative 1 to the calculated value for Alternatives 2.1 through 2.4.

#### **General Observations**

- All alternatives result in an increase in pollutants produced in the study area, as compared with the present due to substantial increases in travel. Improvements in internal combustion engine emissions characteristics over time are not sufficient to overcome the effects of increased travel.
- None of the alternatives can be differentiated by level of pollutants generated. Each performs nearly the same as all others.
- All of the project alternatives improve operating conditions within the corridor and study area to the extent that they attract trips to the study area that otherwise would not occur under the Null alternative. This results in small increases in emissions as compared to the Null Alternative, because they are directly linked to trip making.

_		ALTERN	ATIVES		
	A (Existing)	B (NULL)	C (Alt. 1)	D (Att. 5)	E (Alt. 2.1)
		Daily VMT			
Trucks	£				
<ul> <li>Freeway</li> </ul>	1,040,937	1,962,567	1,952,415	1,972,281	1,947,683
<ul> <li>Arterials</li> </ul>	496,422	734,710	723,793	759,341	726,842
<ul> <li>Alameda</li> </ul>	29.364	90.660	144.333	<u>117.092</u>	144,158
Total	1,566,723	2,787,937	2,820,541	2,848,714	2,818,633
Autos		30 30	а 5 Г		
Freeway	12,191,775	15,385,573	5,255,287	15,318,127	15,246,205
Arterials	9,072,516	10,822,891	10,737,881	11,620,440	10,730,476
<ul> <li>Alameda</li> </ul>	<u>268,319</u>	<u>559,920</u>	877,729	705,426	805,13
Total	21,532,610	26,768,384	26,870,897	27,643,993	26,781,818
	Derived	Speed (VMT/VHT)			
Freeway	28	20	20	20	20
Arterials	21	13	13	14	1:
Alameda	24	21	21	21	2

			TABLE 2 LUTANT EMISS Grams Per Mile			
		Carbon Monoxide (CO)	Reactive Organic Gases (ROG)	Nitrogen Oxides (NO <sub>x</sub> )	Particulates	Sulfur Oxides (SO <sub>x</sub> )
Composite	Factors <sup>(1)</sup>	9				
Year	Speed (mph)			ж 2		
1990	20	12.40	0:97	1.48	.308	đ
	25	9.87	0.78	1.42	.308	Negligible
	30	8.00	0.64	1.40	.308	
2002	15	9.20	0.71	1.23	.268	
	20	7.27	0.57	1.14	.268	
Heavy Du	tty <sup>(2)</sup> Diesel	8.37	2.93	17.20	3.3	3.2

Source: South Coast Air Quality Management District, Air Quality Handbook for Preparing EIRs, April 1987.

Footnote: <sup>(1)</sup> EMFAC 7C composite fleet factors were taken from Appendix D. Emission factors were not published for years beyond 2002. EMFAC 7C diesel truck emission factors were taken from Appendix L. Diesel factors not published by

12} speed.

	×		P	OLLUTANT			
Alternative	Component	со	ROG	NO <sub>x</sub>	Part	SO,	Total Burder
	Trucks	14.4	5.0	29.6	5.7	5.5	,
	Autos						
Α .	<ul> <li>Freeway</li> </ul>	107.3	8.6	18.8	4.1		
(Existing)	Arterials	123.7	9.7	14.8	3.1	•	
	Alameda	<u>2.9</u>	<u>0.2</u>	0.4	<u>0.1</u>		
2.5	TOTAL	248.3	23.5	63.6	13.0	5.5	353.9
а 9	Trucks	25.7	9.0	52.7	. 10.1	9.8	
	Autos	*			e .		
B	<ul> <li>Freeway</li> </ul>	123.0	9.6	19.3	4.5	8	
(NULL)	<ul> <li>Arterials</li> </ul>	109.5	8.4	14.6	3.2	а. 1	
	Alameda	4.5	<u>0.4</u>	0.7	<u>0.2</u>		a.
	TOTAL	262.7	27.4	87.3	18.0	9.8	405.2
	Trucks	26.0	9.1	53.4	10.2	9.9	
	Autos						<i>.</i>
С	• Freeway	122.0	9.6	19.1	4.5		
(Alt. 1)	<ul> <li>Arterials</li> </ul>	108.7	8.4	14.5	3.2		
	Alameda	<u>7.0</u>	<u>0.6</u>	<u>1.1</u>	<u>0.3</u>		С.
	TOTAL	263.7	27.7	88.1	18.2	9.9	407.6
	Trucks	26.2	9.2	53.9	10.3	10.0	
	Autos						
D	<ul> <li>Freeway</li> </ul>	122.5	9.6	19.2	4.5		<i>a</i>
(Alt. 5)	Arterials	117.6	8.3	15.7	3.2		
	<ul> <li>Alameda</li> </ul>	5.6	0.4	0.9	0.2	5	2
				-29			
	TOTAL	271.9	27.5	89.7	18.2	10.0	417.3
	Trucks	25.9	9.1	53.3	10.2	9.9	
	Autos						
E	<ul> <li>Freeway</li> </ul>	121.9	9.6	19.1	4.5		
(Alt. 2.1)	<ul> <li>Arterials</li> </ul>	108.6	8.4	14.5	3.2		
	Alameda	<u>6.4</u>	<u>0.5</u>	<u>1.0</u>	0.2		
	TOTAL	262.8	27.6	87.9	18.1	9.8	406.3

ESTIMATED TOTAL	BLE 4 POLLUTANT BURDEN NS/DAY)
ALTERNATIVE	TOTAL BURDEN
1	407.6
2.1	406.3
2.2	406.3
2.3	406.3
2.4	406.3
3	406.3
5	417.3
6.1	416.0
6.2	416.0
6.3	416.0
6.4	416.0

PREPARED BY: MFA - GLP DATE: August 27, 1991 REVISED: September 26, 1991

### GOAL ANALYSIS - TECHNICAL MEMORANDUM

GOAL:

Environmental Goal No. 3 - Reduce Energy Consumption

#### Introduction

This memorandum presents a comparative evaluation of Alameda Corridor alternatives in light of energy consumption. This information is intended to be incorporated into a larger evaluation matrix that compares alternatives to improve the Alameda Street corridor for both freight rail and vehicular traffic in terms of a number of goals that were established at the outset of the project. The criterion by which energy consumption was to be measured was gallons/day of diesel fuel and gasoline consumed.

#### Methodology

Energy consumption was estimated for several modeling alternatives that were developed by DKS Associates. Vehicle miles of travel, from previously prepared estimates, were divided by fuel economy rates, resulting in estimates of truck diesel fuel and auto gasoline consumption per day. The fuel economy rates were taken from a 1984 study prepared by the Southern California Association of Governments for the Long Beach-Los Angeles Rail Transit Project, which provided fuel economy rates for the years 1984 and 2010. These rates were assumed to yield reasonable results for the Alameda corridor analysis, for the years 1990 and 2020. The results of the analysis are displayed in Table 1.

For alternatives that were not modeled, the results shown in Table 1 were factored to generate estimates for the remaining alternatives. Alternatives 2.2, through 3 were judged to be sufficiently similar to Alternative 2.1 that the estimates for Alternative 2.1 would apply. Estimates for Alternatives 6.1 through 6.4 were obtained by applying the ratio of fuel consumption for Alternative 5 divided by the results of Alternative 1 to the estimates for Alternatives 2.1 through 2.4. The results are shown in Table 2.

### **General Observations**

- Owing to improvements in automobile fuel economy, a 25% decrease in study area fuel consumption can be expected between the present and any of the future year alternatives. This occurs despite an estimated increase in truck travel of 34% and auto travel of 14%.
- The variance among the alternatives, including the Null Allternative, is so slight as to not meaningfully distinguish among them. All alternatives are regarded as equal for purposes

of this analysis. It is not anticipated that the detailed analysis to be conducted for the environmental document would change this finding.

Alternative	A (Existing)	B (NULL)	C (Alt. 1)	D (Alt. 5)	E (Alt. 2.1)
Truck VMT	1,566,723	2,787,937	2,820,541	21,848,714	2,818,633
Truck Fuel Econ. (mpg.)	5.2	5.2	5.2	5.2	5.2
Gallons Diesel	301,293	536,142	542,412	547,830	542,045
Auto VMT	21,532,610	26,768,384	26,870,897	27,643,993	26,781,818
Auto Fuel Econ. (mpg.)	13.9	24.0	24.0	24.0	24.0
Gallons Gasoline	1,549,109	1,115,349	1,119,621	1,151,833	1,115,909
TOTAL GALLONS	1,850,402	1,651,491	1,662,033	1,699,663	1,657,954

TABLE 2 SUMMARY OF ENERGY ESTIMATES (Gallons Per Day)							
ALTERNATIVE	FUEL CONSUMPTION						
1	1,662						
2.1	1,658						
2.2	1,658						
2.3	1,658						
2.4	1,658						
3	1,658						
5	1,700						
6.1	1,696						
6.2	1,696						
6.3	1,696						
6.4	1,696						
Source: Myra L. Frank	« & Associates, Inc., 1991.						

PREPARED BY: MFA-GLP DATE: August 27, 1991 REVISED: September 26, 1991

#### **GOAL ANALYSIS - TECHNICAL MEMORANDUM**

GOAL:

Environmental Goal No. 6 - Aesthetics

### Introduction

This memorandum presents a comparative evaluation of Alameda Corridor alternatives in light of aesthetics. This information is intended to be incorporated into a larger evaluation matrix that compares alternatives to improve the Alameda Street corridor for both freight rail and vehicular traffic in terms of a number of goals that were established at the outset of the project. A subjective rating was identified as the criterion by which aesthetics would be measured.

### Methodology

The aesthetic effects of proposed corridor improvements relate to the physical presence of above-ground structures constructed as part of the project and also as a result of daily railroad operations along the corridor. Consequently, these two components were selected as focal points for the analysis.

#### At-Grade Railroad Exposure

Once the project is in operation, it will facilitate the daily movements of up to 90 trains, each of which could be several thousand feet in length. The presence of this level of freight traffic would result in a frequent inability to see across Alameda Street, if the trains were to be operating at grade. This could be viewed as an adverse visual effect and it is amenable to measurement. The number of miles of at-grade railroad operations permitted under each of the alternatives was used as the means by which this would be assessed.

As was noted in the Introduction, a subjective rating was established as the means by which aesthetics would be addressed for the alternatives analysis. In order to translate the number of at-grade railroad miles into a ranking mechanism, a scoring system was devised in which a raw score of between 1 and 5 was assigned to each alternative in each segment. A score of 5 was selected to represent "best" and a score of 1 was selected to represent "worst". The score assigned to each alternative was determined by taking into account both the amount of at-grade railroad exposure and the visual sensitivity of the segment in question. Some segments have very little residential or other uses which are normally considered sensitive to issues such as aesthetics. Segment A, for example, contains no residential uses adjacent or in close proximity

to the corridor. Segment D has approximately 10% of its nearby land use in residential use. The same is true for the Alameda Street portion of Segment B. The Long Beach Avenue portion of Segment B has approximately one-half of its surrounding use as residential and Segment C has about 40% residential uses in proximity. These two portions would therefore be the most sensitive of all study areas. Using the length of at-grade railroad facility while at the same time considering the proportion of nearby sensitive use yielded a score for each alternative.

In order to assess the desirability of each alternative in its entirety, the scores for each segment were summed to yield a total raw score, which was then normalized to the 1 to 5 score system devised at the outset. Table 1 presents the results of this step in the analysis.

### Grade Separations

In a fashion similar to that used for the railroad exposure analysis, an assessment was made of the degree to which above-ground project structures would be present under the project alternatives. The number of above-grade east-west grade separations were determined for each alternative and tabulated. Once again, taking into account the relative sensitivity of the specific segment in question, scores were assigned to each segment. And as in the above analysis, the scores were summed and normalized. Table 2 presents the results of this step in the analysis.

#### Combined Ratings

In order to account for both railroad exposure and above-grade structures, the scores for each were added by segment, totaled, and normalized. The results are presented in Table 3.

### **General Observations**

- Alternatives 1 and 5 are least attractive throughout the project area, because it has the greatest amount of at-grade railroad exposure and the most above-ground grade separation structures. This alternative was rated particularly low in project segments B and C.
- Alternatives 2.4 and 6.4 did not score as high as other alternatives in Segment B and as low as Alternatives 1 and 5 in Segment C. As a result, these two alternatives scored second best overall. This alternative provides for nearly all of Segment B in a depressed configuration, and it would eliminate four above-grade structures proposed by Alternative 1, which makes it more desirable in that portion of the corridor.
- All remaining alternatives are comparable to one another, both in terms of at-grade railroad exposure and the presence of above-ground grade separations. All remaining alternatives would appear to be substantially superior to Alternatives 1 and 5.
- Alternatives 2.2 and 6.2 in Segment B do not show differences when compared with most other alternatives, because no at-grade railroad or above-grade roadway structures are proposed. Segment B under this alternative contains a high proportion of adjacent residential land use and therefore if an at-grade rail facility were to be proposed here, a substantial adverse effect could be expected.

	ALTEI PART O	T/ CORRIDOR 1 RNATIVE EVA NE - AT-GRA W SCORES:	LUATION: A		5	-						
		ALTERNATIVE										
		1/5	2.1/6.1	2.2/6.2	2.3/6.3	2.4/6.4	3					
_	RR miles at-grade	0	0	0	0	0	0					
Segment A	Score (1)	5	5	5	5	5	5					
12	RR miles at-grade	5.59	1.30	1.30	1.30	1.36	1.30					
Segment B	Score	2 <sup>(2)</sup>	5	5	5	4	5					
	RR miles at-grade	6.10	0	0.	0.32	6.10	0					
Segment C	Score	1 <sup>(3)</sup>	5	5	4	1 <sup>(3)</sup>	5					
	RR miles at-grade	4.66	4.66	4.66	4.66	4.66	4.66					
Segment D	Score <sup>(4)</sup>	2	2	2	2	2	2					
	RR miles at-grade	16.35	5.44	5.44	6.28	12.11	5.44					
Combined	Total of Raw Scores	10	17	17	16	12	17					
Segments	Normalized Score	2.9	5.0	5.0	4.7	3.5	5.0					

Source: Myra L. Frank & Associates, Inc., July 1991.

Footnotes:

<sup>(1)</sup> Segment A has no residential or other sensitive uses adjacent to the corridor. Scores reflect visibility ratings/view blockages as perceived by the general public.

<sup>(2)</sup> All of the route in this segment is at-grade under this alternative. Sensitive uses are estimated to contribute a small proportion of adjacent properties.

<sup>(3)</sup> All of the route in this segment is at-grade under this alternative. Sensitive uses comprise 35-40% of properties adjacent to the corridor.

<sup>(4)</sup> Sensitive uses are estimated to comprise 10% of properties adjacent to the corridor.

TABLE 2         ALAMEDA CORRIDOR TRANSPORTATION PROJECT         ALTERNATIVES EVALUATION: AESTHETICS         PART TWO - GRADE SEPARATIONS         (RAW SCORES: 5=BEST; 1=WORST)											
х.	ALTERNATIVES										
8	1/5	2.1/6.1	2.2/6.2	2.3/6.3	2.4/6.4	3					
Number Of Above Grade Structures		Ŧ									
Segment A	0	0	Ó	0	O	0					
Segment B	4	0	0	0	0	O					
Segment C <sup>(1)</sup>	8(2)	0	0	4 <sup>(2)</sup>	8 <sup>(2)</sup>	0					
Segment D	3	3	3	3	3	3					
Total	15	3	3	7	11	3					
Raw Scores		а.									
Segment A	5	5	5	5	5	5					
Segment B	3 .	5	5	5	5	5					
Segment C	2	5	5	4	2	5					
Segment D	5	5	5	5	5	5					
Total	15	20	20	19	17	20					
Normalized Score	3.75	5.0	5.0	4.75	4.25	5.0					

Source: Myra L. Frank & Associates, Inc., July 1991.

Footnotes:

<sup>(1)</sup> Excludes Rosecrans Avenue.

<sup>(2)</sup> Four grade separations are currently proposed as above-grade structures. The remainder could be abovegrade or under passes.

<sup>(3)</sup> These scores pertain only to the portion of Segment D in which alternatives 7.1 and 7.2 propose improvements.

ALA	TABLE 3 ALAMEDA CORRIDOR TRANSPORTATION PROJECT ALTERNATIVES EVALUATION: AESTHETICS COMBINED AESTHETICS RATING									
	ALTERNATIVES									
	1/5	2.1/6.1	2.2/6.2	2.3/6.3	2.4/6.4	3				
Combined Raw Scores (1)										
Segment A	10	10	10	10	10	10				
Segment B	5	10	10	10	9	10				
Segment C	. 3	10	10	8	3	10				
Segment D	7	7	7	7	. 7	7				
Total	25	35	35	35	29	35				
Normal Score	3.6	5.0	5.0	5.0	4.1	5.0				

Source: Myra L. Frank & Associates, Inc.

Footnotes: <sup>(1)</sup> Scores shown are the addition of the at-grade railroad exposure and grade separations ratings. <sup>(2)</sup> These scores pertain only to the portion of Segment D in which alternatives 7.1 and 7.2 propose improvements.

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PREPARED BY: MFA - GLP DATE: September 9, 1991 REVISED: September 27, 1991

#### **GOAL ANALYSIS - TECHNICAL MEMORANDUM**

GOAL:

Environmental Goal No. 7 - Minimize Exposure to Noise and Vibration

### Introduction

This memorandum presents a comparative evaluation of Alameda Corridor alternatives in light of operational noise and vibration. This information is intended to be incorporated into a larger evaluation matrix that compares alternatives to improve the Alameda Street corridor for both freight rail and vehicular traffic in terms of a number of goals that were established at the outset of the project. The criterion used for evaluating noise and vibration effects was the number of sensitive receptors affected by noise and vibration.

#### Methodology - Operational Noise and Vibration

The analysis of operational noise and vibration considered the effects on five groups of sensitive receptors; residences, schools, hospitals/medical centers, churches, and parks. In order to provide a more detailed accounting, residences have been broken down into three subgroups; single family, multi family housing with 2 to 4 units, and multi family housing with 5 or more units, including mobile home parks.

At full operation, up to 90 trains per day will traverse the corridor. In order to determine the potential effects of this level of train use and also to consider the contributing effects on vehicular travel along the corridor, Harris Miller Miller and Hanson developed a prediction model that provided estimates of impact distances under varying assumptions of vehicular traffic and train volumes, as well as consider the different structure configurations. Noise projections were made based on measured train noise and standard Federal Highway Administration (FHWA) models for traffic noise. Both Urban Mass Transportation Administration (UMTA) and FHWA criteria were considered and as a result, a criterion of 63 dBA CNEL was selected for the analysis. Using this criterion, impact distances were estimated. The initial impact distance estimates were subsequently refined to account for alternatives that separated rail traffic from vehicular traffic. The refined impact distances are as follows: (1) at-grade trains and vehicular traffic could be expected to produce a significant effect (exceed the criterion) on properties up to 1,500 feet on either side of the alignment, (2) depressed trains together with at-grade vehicular traffic could affect properties up to 1,000 feet on either side of the alignment, (3) at-grade vehicular traffic alone could affect properties up to 700 feet away, (4) at-grade trains alone could have an impact distance of 1200 feet, and (5) depressed trains alone could have an impact distance of 335 feet. The documentation in support of these estimated impact distances is attached to this technical

memorandum. The impact distances predicted by the noise model were used to reflect the effects of vibration as well, recognizing that a more detailed analysis of this issue will be required during subsequent project development phases. This decision was made because it is unlikely that the depressed or at-grade rail configurations would have markedly different vibration characteristics. Low frequency vibrations are expected in either case, with associated long wave lengths.

A land use map was prepared at a scale of 1" = 400' in order to identify the impact areas in the context of individual properties. Assessor's Index Sheets were used for this purpose, supplemented with a collection of Assessor's Page Maps encompassing the corridor. When necessary, the boundaries were split at the parcel level. After these parcel splits were made, each alternative was defined according to Assessor's Book, Page, and Parcel, taking into account the noise contours that applied for each segment.

In order to account for the widest range of potential effect, transitions from at-grade to depressed railroad cross sections were assumed to be at the full at-grade band-width until full descent into the trench was reached. Likewise, transitions from depressed to at-grade were considered to be at the full at-grade band-width at the earliest point of the ascent out of the trench. Thus, the noise contour of all at-grade and grade-transition sections for trains plus vehicular traffic is 1,500', and is 1,000' only for fully depressed train sections, where trains and vehicular traffic operate adjacent to one another. This is a conservative assumption that would be refined for subsequent studies.

Once the band-widths had been determined for each alternative in each segment, a series of computer programs were developed and run in order to identify individually affected parcels. These programs were designed to identify and categorize all parcels defined within the impact contours, according to the land use types discussed above. These programs were applied to a database containing parcel-level land use information for this corridor, consisting of approximately 9,000 parcels. The programs isolated each parcel containing a sensitive use, and performed various processing operations, depending on the type of use encountered. For example, residential uses were counted by number of units, where single family was counted as one unit and all other residential parcels counted the number of units given on the database for that particular parcel. Schools, churches, and hospitals/medical centers were each counted as one unit, and parks comprised of multiple parcels per park were counted as one unit.

The sensitive-use parcels were totalled according to where they coincided with the appropriate band-width of effect for each alternative. These totals occur by segment and alternative, and are totalled for each alternative including all four segments. The results are presented in Tables 1 through 8.

#### **General Observations**

Alternatives 1 and 5 generally affected substantially more sensitive receptors than any of the other alternatives. This large difference is accounted for by the fact that these alternatives, being at-grade, have the widest band-width. Understanding that the 1,500' band-width is 50% larger than the area of the 1,000' band-width, this means that the area between 1,000' and 1,500' is predominantly comprised of sensitive receptors. Two exceptions to this finding are hospitals/medical centers and parks, which were evenly affected throughout all the alternatives.

- The alternatives with the next largest effect, 2.4 & 6.4, are also the alternatives with the next largest amount of at-grade rail.
- All of the remaining alternatives are relatively comparable, and the majority of the effect of all of the alternatives takes place in segments B and C.
- o Alternatives 1 & 5 would affect nearly twice as many school parcels as any other alternative.
- Alternatives 1 & 5 would adversely affect the greatest number of churches, followed by Alternatives 2.4 & 6.4. The remaining alternatives would have a somewhat diminished and nearly equivalent effect on churches.
- Few parks or medical centers would be affected by any alternatives.

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Taking into account all receptors, Alternatives 1 & 5 would have the greatest adverse effect. Alternatives 2.1, 6.1, 2.2, 6.2, & 3 have the least effect and are nearly equivalent to one another.

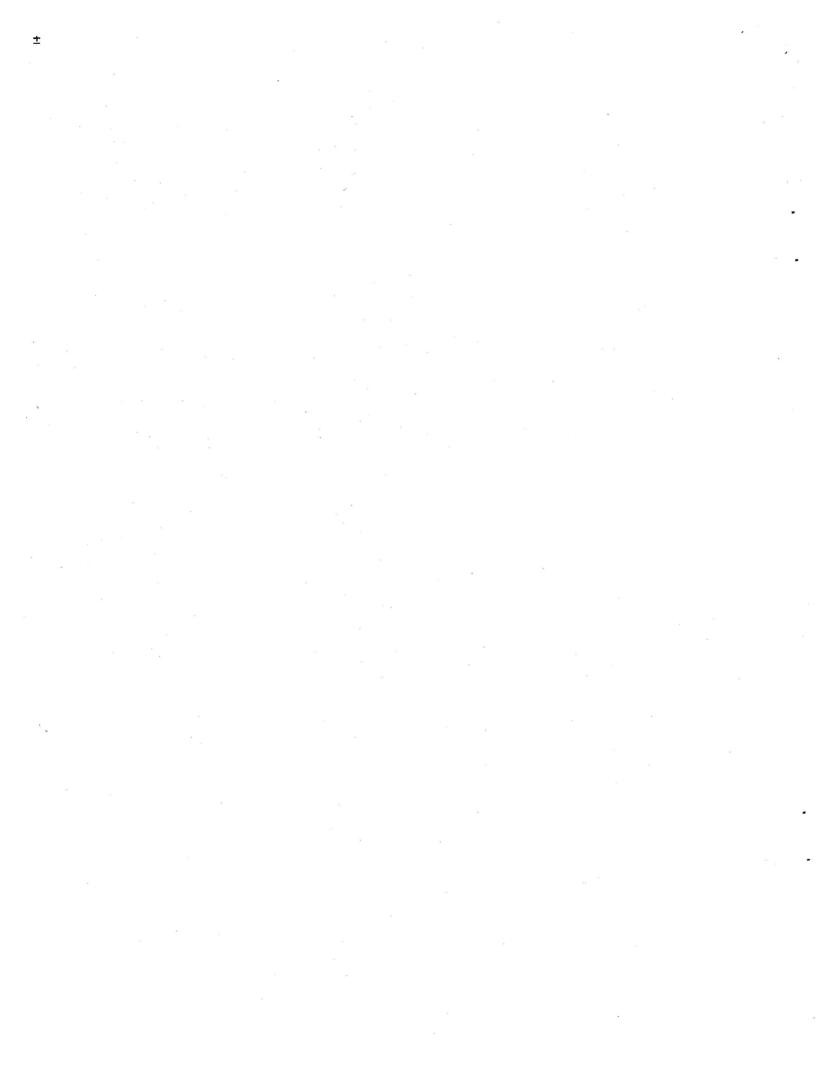


 Table 1 Number of Units - Single Family Residential

SEGMENT		ALTERNATIVES								
	1, 5	2.1, 6.1	2.2, 6.2	2.3, 6.3	2.4, 6,4	3				
Α	29	29	29	29	29	29				
В	2062	1091	867	1091	1350	1091				
С	3366	2081	2081	2729	3366	2081				
D	924	924	924	924	924	924				
TOTALS	6381	4125	3901	4773	5669	4125				

 Table 2 Number of Units - Multi Family Residential (2 - 4 Units)

SEGMENT	ALTERNATIVES								
	1, 5	2.1, 6.1	2.2, 6.2	2.3, 6.3	2.4, 6.4	3			
A	6	6	6	6	6	6			
В	3178	1681	1374	1681	1914	1681			
С	2313	1408	1408	1960	2313	1408			
D	276	276	276	276	276	276			
TOTALS	5773	3371	3923	3923	4529	3371x			

SEGMENT		ALTERNATIVES								
	1, 5	2.1, 6.1	2.2, 6.2	2.3, 6.3	2.4, 6.4	3				
Α	0	0	. 0	0	0	0				
В	1070	541	471	541	559	541				
С	1064	651	651	901	1064	651				
D	666	666	666	666	666	666				
TOTALS	2800	1858	1788	2108	2289	1858				

Table 3 Number of Units - Multi Family Residential (5 or more units, mobile homes)

 Table 4 Number of Parcels - Schools

SEGMENT		ALTERNATIVES								
	1, 5	2.1, 6.1	2.2, 6.2	2.3, 6.3	2.4, 6.4	3				
A	0	0	0	0	0	0				
В	20	5	16	. 5	5	5				
С	22	12	12	13	22	12				
D	0	0	0	0	0	0				
TOTALS	42	17	28	18	27	17				

Table 5 Hospitals / Medical Centers

SEGMENT	ALTERNATIVES								
	1, 5	2.1, 6.1	2.2, 6.2	2.3, 6.3	2.4, 6.4	3			
A	0	0	0	0	0	0			
В	1	1	1	1	1	1			
с	1	1	1	1	1	. 1			
D	0	0	0	0	0	0			
TOTALS	2	2	2	2	2	2			

### Table 6 Churches

SEGMENT	ALTERNATIVES								
SEGMENT	1, 5	2.1, 6.1	2.2, 6.2	2.3, 6.3	2.4, 6.4	3			
A	0 .	0	0	0	0	0			
В	28	17	8	17	17	17			
C .	36	20	20	29	36	20			
D ·	3	3	3	3	3	3			
TOTALS	67	40	31	49	56	40			

# Table 7 Parks

SEGMENT		ALTERNATIVES								
	1, 5	2.1, 6.1	2.2, 6.2	2.3, 6.3	2.4, 6.4	3				
Α	0	0	0	0	. 0	0				
В	3	2	2	2	2	2				
С	1	1	1	1	1	1				
D	0	0	0	0	0	0				
TOTALS	4	3	3	3	3	3				

# Table 8 ALL RECEPTORS

SEGMENT			ALTERN	ATIVES		
	1.5	2.1, 6.1	2.2, 6.2	2,3, 6,3	2.4, 6.4	3
A	35	35	35	35	35	35
B	6362	3338	2739	3338	3848	3338
С	6803	4174	4174	5634	6263	4174
D	1869	1869	1869	1869	1869	1869
TOTALS	15,069	9416	8817	10,876	12,015	9416

HARRIS MILLER MILLER & HANSON INC.

429 Marrett Road Lexington, Mass. 02173 Tel. (617) 863-1401 Fax (617) 861-8188

### TECHNICAL MEMORANDUM

- TO: Gary Petersen Myra L. Frank and Associates
- FROM: Yuki Kimura Hugh Saurenman Harris Miller Miller & Hanson Inc.

DATE: July 5, 1991

nmmin

SUBJECT: Preliminary Noise Impact Analysis, Consolidated Transit Corridor HMMH Job No. 291080

A noise impact analysis was performed for the proposed rail line along the Alameda Corridor using both projected traffic volumes on Alameda Street and train pass-bys for the various alternatives. The results of this analysis are outlined below.

#### SUMMARY OF RESULTS

Noise projections were made based on measured train noise and standard Federal Highway Administration (FHWA) models for traffic noise. Two different noise impact criteria were considered: the criteria proposed by HMMH for the Urban Mass Transportation Administration (UMTA), which have not been officially adopted by UMTA, and existing FHWA noise abatement criteria. The FHWA criteria are based on peak or noisiest hour of the day; an implicit assumption is a typical distribution of traffic between day and night. The UMTA procedure involves comparison of the existing, pre-project ambient noise level in terms of Community Noise Equivalent Level (CNEL) and then comparing it with the predicted project CNEL. We recommend the UMTA criteria as more appropriate for the Alameda Corridor project since a high percentage of the freight traffic is expected to occur during nighttime hours when most people are more sensitive to community noise.

The projections assume that freight traffic is equally distributed through the day; automobile and truck traffic volumes were taken directly from the projections made by DKS. We made projections using a range of different traffic volumes, representing different points along the corridor. The final impact screening distances are not very sensitive to traffic volume because the train noise tends to dominate the noise environment.

The worst-case impact distance was found to be approximately 1500 feet with the at-grade train configuration, and 900 feet with depressed track, for which a shielding adjustment of 12 to 15 dB was assumed. Mitigation provided by the depressed rail could be improved by enclosing the trench

# HARRIS MILLER MILLER & HANSON INC.

Technical Memo to Gary Petersen, Myra L. Frank & AssociatesJuly 5, 1991Preliminary Noise Impact Analysis:Alameda CorridorPage 2

more, applying sound-absorptive material to the walls of the trench, or constructing side barriers above-grade. With these enhancements a 20 dB sound reduction may be possible.

Additional shielding adjustments can be assumed for rows of buildings between the source and receiver. About 3 dB is provided by the first row when the buildings occupy 40 to 65 percent of the length of the row and 5 dB when the buildings occupy 65 to 90 percent of the length of the row. The standard assumption is an additional attenuation of 1.5 dB for each successive row up to a maximum attenuation of 10 dB, which is the maximum attenuation that this mechanism provides.<sup>1</sup>

### NOISE IMPACT CRITERIA

nmmh

<u>UMTA Criteria</u>: These criteria were recently developed by HMMH for UMTA transit projects in urban areas.<sup>2</sup> They are based on comparison of the existing noise levels and future noise levels with the proposed project. Because the criteria are based on 24-hour noise exposure as expressed by CNEL or  $L_{dn}$ , they account for community annoyance caused by late night or early morning train service as well as the varying sensitivity of communities to projects under different background noise conditions. Three categories of impact are defined: i) Severe Impact, under which noise mitigation should be required for the affected areas; ii) Impact, which represents sufficient impact such that noise mitigation should be considered and included in the project if practical and cost-effective; and iii) No Impact, where noise from the project may be audible, but is considered to cause only a minor change in the community noise environment.

<u>FHWA Criteria</u>: When mass transit projects will be integrated with existing highways, noise impact and noise abatement guidelines are often determined using existing FHWA procedures. The procedures include FHWA Noise Abatement Criteria, current FHWA noise prediction models and the guidlines for considering noise abatement measures. According to these criteria, traffic noise impacts occur when the predicted traffic noise levels approach or exceed the FHWA Noise Abatement Criteria based on land use (exterior peak hour  $L_{eq} = 67$  dBA for residential land use), or when the predicted traffic noise levels substantially exceed the existing noise levels. An increase greater than or equal to 10 or 15 decibels is considered substantial. The regulations further prescribe that noise impact should be assessed for the noisiest hour of the day in the design year.

<sup>&</sup>lt;sup>1</sup> Barry, T.M. and Reagan, J.A., FHWA Highway Traffic Noise Prediction Model, Report No. FHWA-RD-77-108, Federal Highway Administration, December 1978.

<sup>&</sup>lt;sup>2</sup> Guidance Manual for Transit Noise and Vibration Impact Assessment, prepared by Harris Miller Miller & Hanson Inc. for U.S. Department of Transportation, Urban Mass Transportation Administration, June 1991.

# HARRIS MILLER MILLER & HANSON INC.

Technical Memo to Gary Petersen, Myra L. Frank & Associates Preliminary Noise Impact Analysis: Alameda Corridor July 5, 1991 Page 3

### NOISE PROJECTION MODELS

Future noise levels were predicted using mathematical models of freight train and highway traffic noise propagation along the Alameda Corridor. The scenarios evaluated were for the year 2020 (90 train movements per peak day), including at-grade trainway with grade-separated east/west traffic, and depressed trainway with at-grade traffic. The geometries for both center and east-side track alignments were considered for the two track configurations. Worst-case traffic volumes for each scenario were assumed in projecting traffic noise.

The projections of train noise are based on the measurements that we performed in October 1990. As mentioned above, we assumed that the 90 trains per day (45 in each direction) were equally distributed through the daytime and nighttime hours. Because of the 10 dB penalty included in calculation of CNEL for nighttime noise (10 pm to 7 am), computationally one nighttime train is equivalent to 10 daytime trains.

#### IMPACT SCREENING DISTANCES

Noise levels were computed over a range of distances separately for trains and traffic. An existing CNEL of 68 dBA was assumed for the entire corridor. The propagation characteristics for train and traffic CNEL/Peak Hour  $L_{eq}$  were then combined, and using the above guidelines, distances to the different impact levels were extracted. These are summarized below in Table 1.

It was found that differences in traffic volumes on Alameda Street had little impact on the total CNEL for the at-grade railway alternatives, since train noise will be the dominant noise source. For the depressed railway alternatives, however, shielding provided by the deep 27-foot trench is significant enough that street traffic noise is projected to be higher than noise from train movements.

The impact distances using the FHWA criteria are significantly less than the impact distances using the UMTA criteria. If the train traffic were concentrated during the daytime hours as is typical for highway traffic, the two criteria would be in close agreement. To give some idea of this effect, we estimated FHWA criteria impact distances assuming that there are twice as many trains during the peak hour. As indicated in the table, with the railroad tracks at-grade, this increases the impact distances by a factor of approximately 1.5. With the railroad tracks depressed, this results in only a small change in the impact distances.

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# Noise Impact Distances - Year 2020

ALTERNATIVE DESCRIPTION	Receiver Location	Distance to Significant impact (ft) (CNEL = 63 dBA)			Distance to Severe Impact (ft) (CNEL = 69 dBA)		
		Trains + Traffic	Traffic only	Trains only	Trains + Traffic	Traffic only	Trains only
Alt. C2: At-Grade RR with one-way couplet (Alameda)	East/West <sup>in</sup>	1510	700	1240	680	255	560
Alt. C2: At-Grade RR east side with 6-lane Alameda west side	West <sup>(e)</sup>	1490	715	1180	640	270	505
	East <sup>(b)</sup>	1480	605	1210	630	160	530
<u>Alt. E2</u> : Depressed RR with one-way couplet at-grade (Alameda)	East/West <sup>(*)</sup>	890	710	335	325	260	75
Alt. E2: Depressed RR east side with 6-lane Alameda west side	West®	900	735	285	330	280	25
	East <sup>ei</sup>	870	680	335	295	220	75

(a) (b) Distance measured from center line of near traffic tane on Alameda.

Distance measured from center line of frontage road.

(C)

Distance measured from center line of drill track.

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		ALTERNATIVES								
SEGNENT	1, 5	2.1, 6.1	2.2, 6.2	2.3, 6.3	2.4, 6.4	3				
٨	29	29	29	29	29	29				
8	2062	781	1070	• 1091	1350	1091				
C	3366	2081	2081	2729	3366	2081				
D	924	924	924	924	924	924				
TOTALS	6381	3815	4104	4773	5669	4125				

 Table 1 Number of Units - Single Family Residential

Table 2 Number of Units - Multi Family Residential (2 - 4 Units)

	ALTERNATIVES								
SEGMENT	1, 5	2.1, 6.1	2.2, 6.2	2.3, 6.3	2.4, 6.4	3			
Α	6	6	6	6	6	6			
В	3178	1103	1529	1681	1914	1681			
C	2313	1408	1408	1960	2313	1408			
D	276	276	276	276	276	276			
TOTALS	5773	2793	3219	3923	4529	3371x			

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# ALAMEDA CORRIDOR TRANSPORTATION PROJECT NOISE & VIBRATION SENSITIVE RECEPTORS

. ALTERNATIVES SEGMENT 2.3, 6.3 2.4, 6.4 2.2, 6.2 1, 5 2.1, 6.1 В С D TOTALS 

Table 3 Number of Units - Multi Family Residential (5 or more units, mobile homes)

Table 4 Number of Parcels - Schools

	ALTERNATIVES							
SEGMENT	1, 5	2.1, 6.1	2.2, 6.2	2.3, 6.3	2.4, 6.4	3		
Α	0	0	0	0	0	0		
B	20	16	16	5	5	5		
С	22	12	12	13	22	12		
D	0	0	0	0	0	0		
TOTALS	42	28	28	18	27	17		

Source: Myra L. Frank & Associates, Inc., August 1991.

10/10/91

# Table 5 Hospitals / Medical Centers

SEGMENT	ALTERNATIVES							
	1, 5	2.1, 6.1	2.2. 6.2	2.3, 6.3	2.4, 6.4	3		
Α	0	0	0	0	0	0		
В	1	0	1	1	1	1		
c	1	1	1	. 1	- 1	1		
D	0	0	0	0	0	0		
TOTALS	2	1	2	2	2	2		

# Table 6 Churches

	ALTERNATIVES							
SEGMENT	1, 5	2.1, 6.1	2.2, 6.2	2.3, 6.3	2.4, 6.4	3		
A	0	0	0	0	0	0		
В	28	3	11	17	17	17		
С	36	20	20	29	36	20		
D	3	3	3	3	3	3		
TOTALS	67	26	34	49	56	40		

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# ALAMEDA CORRIDOR TRANSPORTATION PROJECT NOISE & VIBRATION SENSITIVE RECEPTORS

# Table 7 Parks

SEGMENT	ALTERNATIVES							
	1, 5	2.1, 6.1	2.2, 6.2	2.3, 6.3	2.4, 6.4	3		
A	0	0	0	0	0	0		
B	3	1	2	2	2	2		
C	1	1	1	1	1	1		
D	0	0	0	0	O	0		
TOTALS	4	2	3	3	3	3		

# Table 8 ALL RECEPTORS

SEGMENT	ALTERNATIVES							
	1.5	2.1, 6.1	2.2, 6.2	2.3, 6.3	2.4, 6.4	3		
Α	35	35	35	35	35	35		
В	6362	2347	3166	3338	3848	3338		
С	6803	4174	4174	5634	6263	4174		
D	1869	1869	1869	1869	1869	1869		
TOTALS	15,069	8425	9244	10,876	12,015	9416		

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PREPARED BY: DMJM/M&N-WJM DATE: 9/09/91 REV: <u>10/2/91</u>

10/10/91

### **GOAL ANALYSIS - TECHNICAL MEMORANDUM**

# **GOAL:** Economic Goal No. 2 - Minimize Land Devoted to Port Related Rail Freight Operations (Throughout Basin)

### **INTRODUCTION:**

This memorandum presents a evaluation of Alameda Corridor alternatives relative to minimizing land devoted to Port related rail freight operations as compared to the "Status Quo" network. This information is intended to be incorporated into a larger matrix that compares alternatives to improve Alameda Street Corridor for both rail and vehicular traffic.

The measurement criteria used to quantify land devoted to Port related rail freight operations is <u>Train Route Miles from the City of Industry to the Ports.</u>

### METHODOLOGY:

The Train Routes Miles for each of the lines in the "Status Quo" network and for each of the Alternatives was measured and tabulated in order to compare the "Status Quo" to the "Consolidated Corridor" and to compare the Alternatives to each other. The Train Route Miles are tabulated in Tables EG2 - 1 and EG2 - 2.

### **CONCLUSIONS:**

Referring to Table EG2 - 1, it can be seen that the Corridor network will result in decreases of over 70 percent in Train Route Miles as compared to the "Status Quo" network.

Referring to Table EG2 - 2, it can be seen that all alternatives will have equal Train Route Miles, except Alternatives 2.2 and 6.2 (Wilmington Diversion Alternatives), which are slightly longer.

BRANCH OR SUBDIVISION	ROUTE MILES	MILES UTILIZED STATUS QUO	MILES UTILIZED CORRIDOR
STATUS QUO			
SPTC SAN PEDRO WILIMINGTON (MC-5 TO J-YARD) LA HABRA SANTA ANA/PUENTE WEST LINE (PASADENA JCTINDUSTRY) J-YARD TO PASADENA JCT.	19.5 8.3 19.6 15.8 17.8 3.8	19.5 8.3 19.6 15.8 17.8 3.8	0 0 0 17.8 3.8
UPRR SAN PEDRO EAST L.A. TO INDUSTRY	19.7 11.3	19.7 11.3	0.0 11.3
ATSF HARBOR REDONDO JCT TO DT JUNCTION	28.5 8.9	28.5 8.9	0 8.9
CONSOLIDATED CORRIDOR PORTS TO DOWNTOWN CONNECTIONS (NOTE: ALTS. 2.2 AND 6.2 - 19.6 MILES)	19.1	N/A	19.1
TOTALS		153.2	41.8

REDUCTION IN RAILROAD ROUTE MILES "CONSOLIDATED CORRIDOR" V. "STATUS QUO"					
ALL ALTERNATIVES EXCEPT 2.2 AND 6.2 111.4 MILES 73% REDUCTION					
ALTERNATIVES 2.2 AND 6.2 110.9 MILES 72% REDUCTION					

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#### TABLE EG2 - 1: RAILROAD ROUTE MILES - PORTS TO CITY OF INDUSTRY/DT JUNCTION "STATUS QUO" v. "CONSOLIDATED CORRIDOR" ALTERNATIVES

	COMPARISON OF TRAIN ROUTE MILES											
		ALTERNATIVES										
	1.0	2.1	2.2	2.3	2.4	3.0	5.0	6.1	6.2	6.3	6.4	
TRAIN ROUTE MILES	41.8	41.8	42.3	41.8	41.8	41.8	41.8	41.8	42.3	41.8	41.8	
NORMALIZED SCORE	1.00	1.00	0.99	1.00	1.00	1.00	1.00	1.00	0.99	1.00	1.00	

TABLE EG2 – 2: COMPARISON OF TRAIN ROUTE MILES AMONG "CONSOLIDATED CORRIDOR" ALTERNATIVES

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PREPARED BY: MFA - GLP DATE: August 27, 1991 REVISED: October 9, 1991

#### **GOAL ANALYSIS - TECHNICAL MEMORANDUM**

**GOAL:** Economic Goal No. 4 - Maintain/Improve Access to Existing Businesses

#### Introduction

This memorandum presents a comparative evaluation of Alameda Corridor alternatives in light of business access considerations. This information is intended to be incorporated into a larger evaluation matrix that compares alternatives to improve the Alameda Street corridor for both freight rail and vehicular traffic in terms of several goals that were established at the outset of the project. A subjective rating was identified as the criterion by which this would be measured.

#### Methodology

The initial criterion of subjective rating was replaced by a quantitative approach, described as follows. Affected businesses were defined as commercial or industrial structures located adjacent to Alameda Street or adjacent to roadway/rail improvements that would have to be constructed for the respective alternative. Roadway/rail improvements could be overpasses, underpasses, or new frontage/access roads. It should be noted that the proposed project would generally improve access to businesses and promote increased economic activity due to its improved operating conditions. To the extent these improvements occur, they would generally affect business activity in the corridor uniformly, and thus would not distinguish among alternatives. Therefore, the analysis methodology was focused on adverse access issues that, would more readily exist in the evaluation of competing alternatives.

Residential dwellings were excluded from this analysis. Using 1" = 100' engineering drawings and comparing present access on Alameda Street with future access determined the impact of a given alternative in terms of business access. The perspective of a motorist provided the method for the analysis, focusing on the businesses' current and future accessibility by Alameda and the cross streets, wherever applicable. Access to a business would be considered inconvenient if the motorist could not, starting from the business location, reach the north and south bound traffic on Alameda as conveniently as he could at present. Similarly, access to those businesses around the overpasses and underpasses would be considered inconvenient since the motorist would have to travel circuitous routes to get to the businesses. Before the analysis was begun, assumptions were made concerning the impact on access. The following is a list of these assumptions.

\* U-turns were defined as not being an inconvenience for drivers if the travel distance (before making the U-turn) was less than 1,000 feet.

\* Building footprints on the engineering drawings were generally counted as one structure unless aerial maps (1"=50') clearly delineated the outline of more than one structure.

\* Rail traffic would be so heavy that crossing the tracks would only be possible at overpasses and underpasses.

Crossing structures on the engineering drawings were assumed to be overpasses unless explicitly defined as an underpass. Overpasses have a greater impact on the surrounding area.
Only frontage roads that ran parallel to Alameda Street were measured. Present and future frontage roads satisfying this condition were included.

\* Only the front row buildings along Alameda and the cross streets were included in the analysis.

Businesses with inconvenient access resulted from the construction and/or operation of the corridor were identified using the aerial photos and engineering drawings provided by DMJM/M&N. In addition, structures that would be taken for right of way purposes were also reported. While in a strict sense this is not a post-project access consideration, it does provide an additional indication of overall effects on businesses. Lengths of at-grade or depressed rail alignments, frontage roads, and overpasses/underpasses were also recorded from each engineering drawings using a map wheel. This information , including number of overpasses and underpasses, was recorded in tabular format. Schematic drawings of each alternative have also been prepared to show frontage roads, overpasses/underpasses, and rail alignments. Special Note: A few segments among the alternatives did not have an engineering drawing. In these cases, the missing portion was noted on the schematic and further noted on the table. Further, the number of improved parcels taken should be regarded as an approximation because there could be more than one business in each building footprint. The following tables show the results of the analysis.

#### General Observations

\* Alternative 1 would result in the greatest number of improved parcels taken, thus the largest number of businesses affected. It should be noted, however, that the number of businesses with inconvenient access under this at-grade railroad option would only be slightly higher than that under the depressed railroad Alternative 2.1. Adverse impacts due to the penetration of overpasses/underpasses into the surrounding neighborhood would be offset by easy access to north and south bound traffic on Alameda Street. With the six-lane Alameda Street on the west side of the railroad tracks (starting in Segment C), no businesses on the west side of Alameda would be affected. Some of the businesses on the east side, however, would have greater difficulty in reaching Alameda Street due to the closure of some of the cross streets.

The Wilmington Branch depressed railroad segment would have effects quite similar to those expected along Alameda Street, with the exceptions that inconvenient business access would be reduced.

\* Alternative 2.4, which has a depressed railroad configuration primarily confined to Segment B, has the same number of improved parcels taken as Alternative 1/5 in Segment C.

\* Inconvenient access is substantial with the depressed railroad options due to the separation of north and south bound traffic on either side of the tracks. The combination of depressed railroad and overpasses/underpasses in Alternative 2.3 results in the largest number of inconvenient business access. Moving the depressed railroad alignment to the Wilmington Branch (Alternative 2.2) reduces the degree of inconvenient access.

\* The presence of frontage roads help to reduce the impact on business access if access to these frontage roads is adequate. Frontage roads with limited access do not mitigate the problems associated with increased travel distance and inconvenient U-turns.

\* Depressed rail alignments generally have a lesser impact on business access because railroad crossings can be provided by at-grade bridges. These structures do not penetrate into the surrounding community and are easier to reach from Alameda Street.

# TABLE 1: ALAMEDA CORRIDOR TRANSPORTATION PROJECT SUMMARY: BUSINESS ACCESS EVALUATION

Evaluation Factor			Aliei	mative		
	1/5	2.1/8.1	2.2/8.2	2.3/6.3	2.4/8.4	з
Improved Parcels Taken						
Segment A	12	12	12	12	12	12
Segment B	146/138	99	92/85	99	99	73
Segment C.	84/56	25	25	49	56	53
Segment D	68	68	68	68	68	68
Totai	310/274	204	197/190	228	235	206
Inconvenient Access						
Segment A	0	0	0	0	0	0
Segment B	136	95	67	95	90	0
Segment C	119	160	160	164	119	26
Segment D	42	40	40	42	42	40
Total	297	295	267	301	251	66
Total Structures and Businesses Affected	607/571	499	464/457	529	486	272

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Date: October 9, 1991 Prepared by: Myra L. Frank & Associates, Inc.

# TABLE 2: ALAMEDA TRANSPORTATION JJECT: BUSINESS ACCESS EVALUATION

				ALTE	RNATIVE 1/5				
Segment	Railroad (i	Railroad (in miles)		Ον	erpasses	Underpasses		No. of Structures with	No. of improved
	At-grade	Depressed	(miles)	Number	Total miles	Number	Total miles	Inconvenient access	parcels taken
Segment A	0.00	0.00	0.00	0	0.00	0	0.00	0	12
Segment B	5.59	0.00	0.36	4	1.61	7	2.42	136	. 146/138
Segment C	6.10	0.00	5.44	7 <sup>b</sup>	3.87	2	0.58	119	84/56
Segment D	4.66	0.00	0.96	3	2.71	2	0.80	42	68
Total	16.35°	0.00	6.76	14	8.19	11	3.80	297	310/274

				ALTERN	ATIVE 2.1/6.1 <sup>d</sup>				
Segment	Railroad (in miles)		Frontage Road	Overpasses		Underpasses		No. of Structures with	No. of improved
	At-grade	Depressed	(miles)	Number	Total miles	Number	Total miles	Inconvenient access	parcels taken
Segment A	0.00	0.00	0.00	0	0.00	0	0.00	0	12
Segment B	1.30	4.31	0.00	8	0.96	1	0.28	95	99
Segment C	0.00	6.09	3.25	10 <sup>6</sup>	0.77	0	0.00	160	25
Segment D	4.14	0.83	0.96	3	2.71	2	0.79	40	68
Total	5.44°	11.23	4.21	21	4,44	3	1.07	295	204

Source: Myra L. Frank & Associates, Inc., October 9, 1991.

<sup>\*</sup> Table for Alternative 5 is the same as Alternative 1, except for number of improved parcels taken.

<sup>&</sup>lt;sup>b</sup> Rosecrans Avenue fly-over and Artesia Blvd overcrossing are excluded in the total number of overpasses.

<sup>&</sup>lt;sup>c</sup> This total excludes the At-Grade Trainway length of the Port Access Demonstration Project between Del Amo Blvd and Sepulveda Blvd.

<sup>&</sup>lt;sup>d</sup> Table for Alternative 6.1 is the same as Alternative 2.1 except that there may be 10-12 fewer structures taken with an estimated 12-foot reduction in the right-of-way width from 6-lanes to 4-lanes.

TABLE 2	NTINUED)	
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				ALTERN	IATIVE 2.2/6.2*				
Segment	Railroad (in miles)		Frontage Road	Ove	rpasses	Underpasses		No. of Structures with	No. of improved
	At-grade	Depressed	(miles)	Number	Total miles	Number	Total miles	Inconvenient access	parcels taken
Segment A	0.00	0.00	0.00	0	0.00	0	0.00	0	12
Segment B	1.30	5.31	0.00	12	0.90	1	0.28	67	92/85
Segment C	0.00	6.09	3.25	10 <sup>6</sup>	1.02	0	0.00	160	25
Segment D	4.14	0.83	0.96	3	2.71	2	0.79	40	68
Total	5.44ª	12.23	4.21	25	4.63	3	1.07	267	197/190

ALTERNATIVE 2.3/6.34									
Segment	Railroad (in miles)		Frontage Road	C	Overpasses	Underpasses		No. of Structures with	No. of improved
	At-grade	Depressed	(miles)	Number	Total miles	Number	Total miles	Inconvenient access	parcels taken
Segment A	0.00	0.00	0.00	0	0.00	0	0.00	0	12
Segment B	1.30	4.31	0.00	8	0.96	1	0.56	95	99
Segment C	2.21	3.90	5.26	10 <sup>6</sup>	2.20	0	0.00	164	49
Segment D	4.66	0.00	0.96	3	2.71	2	0.80	42	68
Total	8.17°	8.21	6.22	21	5.87	3	1.36	301	228

Source: Myra L. Frank & Associates, Inc., October 9, 1991.

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<sup>•</sup> Table for Alternative 6.2 is the same as Alternative 2.2 except that there may be 10-12 fewer structures taken with an estimated 12-foot reduction in the right-of-way width from 6-lanes to 4-lanes.

<sup>&</sup>lt;sup>b</sup> Rosecrans Avenue fly-over and Artesia Blvd overcrossing are excluded in the total number of overpasses.

<sup>\*</sup> This total excludes the At-Grade Trainway length of the Ports Access Demonstration Project between Del Amo Blvd and Sepulveda Blvd.

<sup>&</sup>lt;sup>d</sup> Table for Alternative 6.3 is the same as Alternative 2.3 except that there may be 10-12 fewer structures taken with an estimated 12-foot reduction in the right-of-way width from 6-lanes to 4-lanes.

#### TABLE 2 ITINUED)

				ALTER	NATIVE 2.4/6.4°				
Segment	Railroad (in miles)		Frontage Road	Ov	erpasses	Underpasses		No. of Structures with	No. of improved
	At-grade	Depressed	(miles)	Number	Total miles	Number	Total miles	Inconvenient access	parcels taken
Segment A	0.00	0.00	0.00	0	0.00	0	0.00	0	12
Segment B	1.36	4.53	0.24	7	0.84	1	0.28	90	99
Segment C	6.10	0.00	5.44	7 <sup>6</sup>	3.86	2	0.58	119	56
Segment D	4.66	0.00	0.96	3	2.71	2	0.80	42	68
Total	12.12°	4.53	6.64	17	7.46	5	1.66	251	235

<sup>\*</sup> Table for Alternative 6.4 is the same as Alternative 2.4 except that there may be 10-12 fewer structures taken with an estimated 12-foot reduction in the right-of-way width from 6-lanes to 4-lanes.

<sup>&</sup>lt;sup>b</sup> Rosecrans Avenue fly-over and Artesia Blvd overcrossing are included in the total number of overpasses.

<sup>\*</sup> This total excludes the At-Grade Trainway length the Ports Access Demonstration Project between Del Amo Bivd and Sepulveda Blvd,

				ATIVE 3 WIT	H OPTIONAL FRO	L. Coleman -			
Segment	Railroad (in miles)		Frontage Road	Overpasses		Underpasses		No. of Structures with	No. of improved
	At-grade	Depressed	(miles)	Number	Total miles	Number	Total miles	Inconvenient access	parcels taken
Segment A	0.00	^ 0.00	0.00	0	0.00	0	0.00	0	12
Segment B	1.30	4.36	1.11	32	0.87	. 1	0.28	0	73
Segment C	0.00	6.14	5.42	11 <sup>b</sup>	0.41	0	0.00	26	53
Segment D	4.14	0.83	0.96	3	2.71	2	0.80	40	68
Total	5.44°	11.33	7,49	46	3.99	3	1.08	66	206

Source: Myra L. Frank & Associates, Inc., October 9, 1991.

Segment	Railroad (i	n miles)	Frontage Road (miles)	Overpasses		Underpasses		No. of Structures with	No, of improved
	Al-grade	Depressed		Number	Total miles	Number	Total miles	Inconvenient access	parcels taken
Segment A	0.00	0.00	0.00	0	0.00	0	0.00	0	12
Segment B	1.30	4.36	0.23	52	1.11	1	0.28	0	73
Segment C	0.00	6.14	5.42	11*	0.41	0	0.00	26	53
Segment D	4.14	0.83	0.96	3	2.71	2	0.80	40	68
Total	5.44 <sup>6</sup>	11.33	6,61	66	4.23	3	1.08	178	206

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Source: Myra L. Frank & Associates, Inc., October 9, 1991.

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<sup>\*</sup> Rosecrans Avenue fly-over and Artesia Blvd overcrossing are included in the total number of overpasses.

<sup>&</sup>lt;sup>b</sup> This total excludes the At-Grade Trainway length of the Ports Access Demonstration Project between Del Amo Blvd and Sepulveda Blvd.

# PREPARED BY: DMJM/M&N-WJM DATE: 10/02/91

# **GOAL ANALYSIS - TECHNICAL MEMORANDUM**

# GOAL: Cost Goal No. 1 - Maximize Cost Effectiveness.

## **INTRODUCTION:**

This memorandum presents an evaluation of Alameda Corridor alternatives relative to their ability to maximize Cost Effectiveness of the Corridor Project Improvements. This information is intended to be incorporated into a larger matrix that compares alternatives for the Alameda Transportation Corridor.

The measurement criteria used to quantify Cost Effectiveness is <u>Absolute Costs of the</u> <u>Alternative.</u>

#### **METHODOLOGY:**

In order to compare the costs of each Alternative the total project costs in 1991 dollars were taken from the Alameda Corridor Project Cost Estimate in the Project Report and tabulated below.

Cost (millions)
\$
1588.8
1978.7
2040.9
2087.4
1791.0
2581.8
1580.1
1960.3
1962.7
1872.9
1642.6

# **CONCLUSIONS:**

Referring to the table above, it can be seen that the Alternative 5.0 has the greatest Cost Effectiveness, and Alternative 3.0 the least.

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PREPARED BY: DMJM/M&N-RT/DRM DATE: 9/11/91 Rev. 10/2/9/

DMJM/M&N

#### GOAL ANALYSIS - TECHNICAL MEMORANDUM

GOAL: Cost Goal No. 3 - Ability to Implement in Phases

# **INTRODUCTION:**

This memorandum presents a comparative evaluation of Alameda Corridor alternatives relative to the number of discrete/separate construction components, where more components is better for Alameda Street under each alternative. This information is intended to be incorporated into a larger matrix that compares alternatives to improve Alameda Street Corridor for both rail and vehicular traffic.

The measurement criteria used to identify the subjective value is the number of construction segments to build the Alameda improvements.

The construction segments were determined as follows:

The total alternative was divided into segments which would result in either:

a. A complete operational unit (i.e. grade separation, one useable section of railroad or highway), or

b. A portion of an operational unit costing around \$400 million (escalated) to construct.

Estimates were made for Alternatives 1 and 2.1A. The values for other alternatives were based on a proration of these two comparative totals.

The discrete/separate construction components are:

1

# **ALTERNATIVE 1**

NO.	PROJECT	LIMITS	PROJECT TYPE
1	Henry Ford/T.I. Fwy	Schuyler Heim Br. to Dom. Channel	Construct Grate Separation
2	Anaheim St. Bridge	Anaheim St. at Dominguez Channel	Reconstruct Overhead
3	Pacific Coast Highway OH	P.C.H. at Santa Fe R.R.	Construct Overhead
4	Sepulveda Blvd. OH	Sepulveda Blvd. at Alameda St.	Construct Overhead
5	Reconstruct Alameda	Laurel Park to SR 91	Reconstruct Highway & Grade Sep.
6	Compton Creek RCB	Compton Creek at Alameda	Highway/Railway Structure
7	Southern Trainway	Badger Ave. Br. to Thenard Jct.	Railway/Structures
8	Compton Blvd.	Compton Blvd. at Alameda	Overhead
9	Alondra Blvd.	Alondra Blvd. at Alameda	Underpass
10	Wilm./MC-5 Conn. (incl. Dom. Chan. Strs.)	Thenard Jct. to Dominguez Jct.	Railway (One Track)
11	Florence Ave.	Florence Ave. at Alameda	Underpass
12	Imperial Hwy.	Imperial Hwy. at Alameda	Underpass
13	El Segundo	El Segundo at Alameda	Overhead
14	Firestone Blvd.	Firestone Blvd. at Alameda	Underpass
	Gage Ave.	Gage Ave. at Alameda	Overhead
16	Slauson Ave.	Slauson Ave. at Alameda	Overhead
17	Vernon Ave.	Vernon Ave. at Alameda	Underpass
18	Widen Alameda to 6 Lns.	SR 91 to El Segundo	Reconstruct and Widen Street & Construct Drill Track
19	Widen Alameda to 6 Lns.	El Segundo to 85th Street	Reconstruct and Widen Street & Construct Drill Track
20	Widen alameda to 6 Lns.	85th St. to Slauson incl. Alameda Crossover	Reconstruct and Widen Street & Construct Drill Track
21	Widen alameda to 6 Lns.	Slauson to I-10 incl. N. Bd. Alameda	Reconstruct and Widen Street & Construct Drill Track
22	Central Trainway	Compton creek to J Yard	Construct Trainway
23	Northern Trainway	J Yard to Hobart	Construct Trainway
24	Greenleaf	Greenleaf at Alameda	Construct Overhead
25	Weber	Weber Ave, at Alameda	Construct Overhead
26	Tweedy	Tweedy Ln. at Alameda	Construct Overhead
27	Southern	Southern at Alameda	Construct Overhead
28	Nadeau	Nadeau Ave. at Alameda	Construct Overhead
20	Contract Con		
30	38th/41st St. 25th St.	38th/41st at Alameda	Construct Underpass Construct Underpass
50	20th St.	25th St. at Alameda	Construct Onderpass

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# **ALTERNATIVE 2.1A**

1	Henry Ford/T.I. Fwy	Schuyler Heim Br. to Dom. Channel	Construct Grade Separation
2	Anaheim St. Bridge	Anaheim St. at Dominguez Channel	Reconstruct Overhead
3	Pacific Coast Hwy. OH	P.C.H. at Santa Fe R.R.	Construct Overhead
4	Sepulveda Blvd. OH	Sepulveda Blvd. at Alameda St.	Construct Overhead
5	Compton Creek RCB	Compton Creek at Alameda	Highway/Railway Structure
6	Southern Trainway	Badger Ave. Br. to Thenard Jct.	Railway/Structures
7	Reconstruct Alameda - 4 Lanes	Laurel Park to Rosecrans	Reconstruct Highway & Drill Track
8	Construct Depressed Railway (Dep Cmptn	Thenard Jctn. to El Segundo	Railway, Compton & Alondra Gr. Seps. & Temporary Hwy
1×	Cr El Segundo)		Crossover
9	Widen Alameda - 4 lns & Drill Track	Rosecrans to 85th St.	Construct Hwy on New Alignment and Temporary Drill Track
10	Widen Alameda - 4 lns & Drill Track	85th St. to Slauson	Construct Hwy on New Alignment and Temporary Drill Track
	Widen Alameda - 4 lns & Drill Track	Slauson to I-10	Construct Hwy on New Alignment and Temporary Drill Track
12	Construct Trainway (Depressed)	Rosecrans to 85th St.	Construct Depressed Trainwa and Relocate Drill Track
13	Construct Trainway (Depressed)	85th St. to Slauson	Construct Depressed Trainwa and Relocate Drill Track
14	Construct Trainway (Depressed)	Slauson to J-Yard	Construct Depressed Trainwa and Relocate Drill Track
15	Construct Trainway	J-Yard to Hobart	Construct RR Track and Structures
16	Widen Alameda to 6 Lanes	Rosecrans to Washington Blvd.	Widen Highway

# TABLE CG3-1 NUMBER OF CONSTRUCTION UNITS BY ALTERNATIVE

ALTERNATIVE	1	2.1	2.2	2.3	
NO. OF UNITS	30	16	17	22	
ALTERNATIVE	_2,4	_3	5	6.1	
NO. OF UNITS	20	16	30	16	
ALTERNATIVE	6.2	6.3	6.4		
NO. OF UNITS	17	22	20	ai.	

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DMJM/M&N 🕸

PREPARED BY: DMJM/M&N-RT/DRM DATE: 9/11/91 REV: 10/2/91

#### GOAL ANALYSIS - TECHNICAL MEMORANDUM

GOAL:

Construction Goal No. 1 - Minimize Disruption to Highway and Rail Users

# **INTRODUCTION:**

This memorandum presents a comparative evaluation of Alameda Corridor alternatives relative to the duration in years required to construct improvements on Alameda Street under each alternative. This information is intended to be incorporated into a larger matrix that compares alternatives to improve Alameda Street Corridor for both rail and vehicular traffic.

The measurement criteria used to identify construction duration is the <u>Years of Construction</u> of each Alternative.

# **METHODOLOGY:**

Four representative locations along the Corridor were analyzed. They were:

Slauson Firestone El Segundo

Compton

The estimates for construction were developed for these four locations. Thus an average duration (divided by four) was determined. Then the total construction components developed previously was used to determine the construction duration for each alternative.

ALTERNATIVE	1	2.1	2.2	2.3
CONSTRUCTION DURATION	6	6.75	6.75	6.25
ALTERNATIVE	2.4	3	5	6.1
CONSTRUCTION DURATION	6.25	6.75	6	6.75
ALTERNATIVE	6.2	6.3	6.4	
CONSTRUCTION DURATION	6.75	6.75	6.25	л. ж

#### **CONCLUSIONS:**

It can be seen from the above table that Alternatives 1.0 and 5.0, the At-Grade Trainway have the minimum disruption to highway and rail users.