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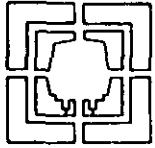
PHASE II PART A

TASK 1.2.7A

SYSTEMS INTERIM STATUS REPORTS

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June 29, 1989

Mr. James E. Crawley, P.E.  
Director of Transit Facilities  
Transit Systems Development  
Southern California Rapid Transit District  
425 South Main Street  
Los Angeles, CA 90014

Subject: Metro Rail Project  
Completion of Phase II, Part A

Purpose: Information Transmittal

File No: P001G082

Dear Mr. Crawley:

The Phase II Interim Status Reports enclosed with this letter comprise the deliverable work products required by the Phase II, Part A Scope of Services for the MRTC Systems Division. These deliverables fulfill the requirements of Tasks Number 1.F and 1.G of the District's Authorization To Proceed letter Number 88-06545, December 21, 1988.

Complete Statements of Tasks required and Tasks performed are contained in the enclosures representing work accomplished by our Train Control, Traction Power, Communications, Operations and Maintenance, and Safety Assurance and Security disciplines. There were no major equipment deliverables scheduled to be completed during the Part A, preliminary design period. Part A work was extended from April 28, 1989 to June 30, 1989 (by District direction) and the systems tasks were completed within the Part A systems budget.

Mr. James. E. Crawley  
June 29, 1989  
Page 2

Work will be resumed on the Systems Division Tasks in accordance with the previously negotiated Scope of Services when District authorization is received.

Should additional copies of the submittals be required, we will be pleased to respond.

METRO RAIL TRANSIT CONSULTANTS



K.N. Murthy  
Project Director

Encl:       Phase II Interim Status Reports  
              Automatic Train Control (4)  
              Traction Power (4)  
              Communications (4)  
              Operations and Maintenance (4)  
              Safety/Assurance and Security (4)  
              Systemwide Equipment Procurement Study (4)

cc:   W. Rhine, SCRTD  
      J. Sandberg, SCRTD  
      H. Storey, SCRTD  
      TSD-DCC  
      DCC (2)

PHASE II INTERIM STATUS REPORT  
AUTOMATIC TRAIN CONTROL  
JUNE 30, 1989

This report summarizes work completed for Automatic Train Control System during the period ending with the cessation of work on June 30, 1989.

The following excerpts from the Phase II Work Program describe the Automatic Train Control work tasks assigned and the work accomplished to an interim level during the period ending June 30, 1989:

- ° Establish power, grounding and equipment mounting space requirements on the wayside as well as in the ATC and Communications room and verify incorporation into MOS-2 facilities design planning.
- ° Refine and analyze block designs for combined MOS-1 and MOS-2 six car train operation at minimum headway taking into consideration operations in subway and on elevated guideway. Prepare analysis report utilizing ATC Block Design Computer Program.
- ° Refine design requirements for MOS-2 interlocking, including diamond cross-overs, pocket tracks, turnouts and turn back at terminal stations based on selected MOS-2 configuration. Incorporate requirements in preliminary specifications.

The work program, which began in January 1989, called for work products to be developed on the following schedule:

<u>Production</u>	<u>Completion Scheduled</u>
Alignment & Operating Parameters	March 17, 1989
Interim Status Report	April 28, 1989
Draft Report (Part B Task)	July 28, 1989
Final Report (Part B Task)	Sept. 29, 1989

Initially the cessation of Phase II design work was scheduled to occur on April 28, 1989, so that the product at that interim period would have been the Interim Status Report. Phase II Part A work was rescheduled from April 28 to June 30 by RTD concurrence and direction.

## Summary of Work Completed.

### 1. Work originated by Systems.

- ° Worked with the Civil Engineers and Operations/Maintenance Planners in the preparation of an interim status report on Phase II alignment and operating parameters. (Copy Attached)
- ° Provided support in the preparation of the systemwide equipment procurement study.
- ° Prepared approximate equipment lists, quantities, and space requirements in providing support for preparation of the Phase II cost estimates.

### 2. Review of design products originated by others.

Reviewed preliminary facilities designs (MacArthur Park interlocking, Wilshire/Vermont, Vermont/Beverly, Vermont/Sunset, Hollywood/Vine, and B330) to verify, to the extent practicable, that existing standards and requirements for ATC are being incorporated; also the reviews provided some insight for defining the issues that must be addressed as the Phase II work progresses. The ensuing activities and definitions from the reviews are as follows:

- ° Verified that the MOS-1 criteria regarding ATC equipment space, power, and conduit provisioning requirements are applicable to Phase II.
- ° Identified the criteria for locating and sizing additional train control rooms (other than at stations) to accommodate the full-pocket interlockings and long station-to-station distances; investigated the restraints and other factors for determining and optimizing the locations.
- ° Identified possible impacts to block design resulting from characteristics of the preliminary alignment; the effects of curves and grades on operations, headway, and run time must be closely monitored when Phase II design resumes.
- ° Identified signaling alternatives for the Phase II interlockings; alternatives regarding automatic routing, terminal operation, pocket track operations, call-on, train storing, back-up routing, converge/diverge operations, station stopping, location of block boundaries, and signal approaches must all be addressed when Phase II ATC design begins.

- Determined which stations may be approached as "typical" with regard to ATC requirements in facilities design.

ANALYSIS OF PHASE II  
ALIGNMENT & OPERATING PARAMETERS  
INTERIM STATUS REPORT  
JUNE 30, 1989

SUMMARY

This report describes work accomplished in response to the Metro Rail, Phase II, Part A task entitled Automatic Train Control Block Design Analysis. The description includes a brief report on the status of design work, as of the cessation of work on Phase II work tasks, on June 30, 1989, and comments on remaining work to be done after Phase II design resumes. The ATC Block Design Analysis is a necessary task in preparation for Phase II block design to be accomplished, and thus is a necessary step in the preparation of ATC furnish and install contract documents.

The work program description of the ATC Block Design Analysis task is:

Refine and analyze block designs for combined MOS-1 and MOS-2 six car train operation at minimum headway taking into consideration operations in subway and on elevated guideway. Prepare analysis report utilizing ATC Block Design Computer Program.

The work program calls for four products:

1. Alignment & Operating Parameters
2. Interim Status Report
3. Draft Report (Part B Task)
4. Final Report (Part B Task)

Product 1 consisted of working notes used in the preparation of this report. Work on product 3, the draft report, will begin when Phase II design resumes.

This report is formatted with respect to geographical portions of the route, starting at the MOS-1 boundary at Alvarado Street.

MACARTHUR PARK POCKET TRACK

1. The addition of a full-pocket interlocking at this location imposes the requirement for an additional train control room. Several decisions must be made concerning options for emergency operation from a local control panel, maintainer needs, and SCADA interface. For preliminary design the control rooms will be treated as two separate locations.

2. Preliminary operating and block design criteria require trains to operate at 45 mph between Wilshire/Alvarado and Wilshire/Vermont, A5 and A6. Except for turnout moves, civil speed restrictions in this area should not be less than 45 mph.
3. Criteria concerning control and operation of the pocket interlocking must be established. The options of automatic routing, pushbutton back-up routing, call-on routing, and SCADA automatic routing will be considered. Alternatives will be analyzed and selected to fit SCRTD needs as appropriate during the design phase.
4. Distance to the A6 TC&C room is approximately 5,100 ft., which is within design limits.
5. Normal train speed from A5 to A6 will be 45 mph due to curves starting at approximately 292 and 303.
6. Trains traveling from A6 to A5 will be restricted to 45 mph for the early part of the trip; however, once the rear of the train clears the curve at (approximately) 292, it can be commanded to accelerate to 55 mph. The 55 mph command will be given, if the R=2500 curve (at approximately 278) can be designed with at least one inch of superelevation (to allow a 55 mph curve rating) without interfering with the special trackwork.
7. Block design specifications will be supplemented to address system characteristics introduced to Metro Rail in the MOS-2 alignment. In order to optimize operation with a full-pocket interlocking, requirements addressing the approaches to the wayside signals should be added. The signals that control both normal traffic movements, and diverging moves into the pocket track, should have an associated approach block of length consistent with a train approaching with a 9 mph speed command.
8. Where a station platform track constitutes all or part of the approach distance to a wayside signal, train moves into the station with the signal at STOP may have to be made in Restricted Manual Mode; manual berthing must be performed if the ATP subsystem does not permit the train to completely berth while operating in the ATO programmed station stopping profile. If manual berthing is required, preferred operation is to stop the train outside the platform, and then proceed to complete berth in manual mode. At A5 both station platforms are bounded by wayside signals; each scenario must be investigated to determine where manual berthing should be implemented. "



9. Specific directive drawing(s) or marked up civil drawings will be needed for locating conduits and wayside equipment.

WILSHIRE/VERMONT (A6)

1. This is a diverge/converge interlocking. It is the only one in the Metro Rail System to date. Specific directive drawing(s) or marked-up civil drawings will be needed for locating conduits and wayside equipment.
2. Due to curves, maximum train speed between A6 and Wilshire/Normandie (B1) will be 45 mph for both tracks in both directions. Maximum speed for trains traveling from A7 to A6 will be 45 mph due to curve. Trains traveling from A6 to A7 could be given a 55 mph command when the rear of the train vacates the curve at 341; however, given that the distance required to accelerate from 45 mph to 55 mph is entirely 3.75% upgrade, it may be of questionable benefit in shortening runtime.
3. Design headway beyond the diverge/converge points will be 120 seconds per SCRTD acceptance of this feature as a cost reduction in the budget estimate. This applies to the normal direction station-to-station runs from A7 to A6 and B1 to A6 for following trains in the same traffic pattern.
4. Operation of the AL track interlocking (normally-converging) should have approach cleared automatic routing. The automatic routes will be cleared on a first-come, first-requested basis, and alternating routing if both approach zones are occupied.

The block design specifications should require that a block boundary be located the distance in approach to each wayside signal corresponding to a 9 mph SBD, and that additional blocks shall be provided as necessary to attain the optimum intermediate speed approach to a signal at STOP.

5. Operation of the AR track interlocking (normally diverging) should not have automatic routing. RCC is provided train destinations, and can route automatically or manually. To continue operations in the event of RCC or CTS failure, a wayside pushbutton for routing should be provided. It may not be advantageous to require the train to close within 9 mph SBD of the red signal, since this would leave the train only partially within the platform. Options for operation are:
  - a. Locate block boundaries and transmit speed commands such that trains will stop outside of the station platform. If the delay in waiting for the signal to

clear is significant, RCC can authorize stop-and-proceed movement into the station.

- b. The platform definition track circuit can be used as a zero speed block. This will allow the train in ATO to stop part-way in the station, and then berth manually in stop-and-proceed submode of MTO.
6. The approximate distances to adjacent TC&C rooms is 4,700 ft. to B1, and 5,700 ft. to A7; this is satisfactory.

WILSHIRE/NORMANDIE (B1)

1. Train speeds between this station and Wilshire/Western (B2) will not exceed 45 mph due to short distance.
2. The distance to the B2 TC&C room is approximately 2,200 ft. This short distance could allow deletion of this train control location. The distance is short enough that ATC would not require an equipment room; however, minimal cost savings would be realized since an equipment room for communications and ATC interface equipment (PSS and door control equipment) would be needed. Also, maintenance becomes easier as the distance between the ATC room equipment the wayside equipment becomes less.
3. B1 is a typical non-interlocking station; a common directive drawing for all non-interlocking stations is sufficient to illustrate ATC conduit and cableway needs.

WILSHIRE/WESTERN (B2)

1. This will be a temporary terminal location with diamond interlocking. Operation and design principles will be very similar to those of Wilshire/Alvarado for MOS-1. If a tail track can be included behind the station, runtime can be improved 25 seconds or more.
2. If the above-referenced tail track cannot be incorporated, then the use of trip stops, similar to A5 MOS-1 configuration, can permit the transmission of adequate speed commands on approach and into the platform for ATO operation, without requiring a long overrun track behind the platform.
3. A directive drawing should be developed to define the conduit requirements for this diamond interlocking; it may serve as a general directive for other diamond interlockings.
4. For BR Track block design specifications should require an approach block corresponding to 9 mph SBD, and best intermediate speed command approach to a signal at STOP.

VERMONT/BEVERLY (A7)

1. This diamond interlocking is typical with no automatic routing.
2. Maximum train speed on both tracks and in both directions between A7 and A8 will be 55 mph.
3. Distance to the A8 TC&C room is approximately 4,700 ft., which is easily within limits.
4. A typical directive drawing for diamond interlocking locations should be sufficient to show conduit requirements.
5. Block design rules for approach to a signal at STOP through the platform track should be consistent with A5.
6. For AR track block design rules should allow 9 mph SBD approach to signal at STOP from the A6 direction.

VERMONT/SANTA MONICA (A8)

1. This is a typical non-interlocking station. Typical directive drawing will be sufficient to convey ATC conduit requirements.
2. Distance to A9 TC&C room is approximately 2,800 ft., which is substantially within limits; this indicates that only about 6 track circuits total will be required at this TC&C room location.
3. Maximum train speed on both tracks and in both directions between A8 and A9 will probably be 45 mph. Acceleration above the 45 mph speed regulation point would be possible; however, only a very minimal runtime savings on the A8 to A9 route would be achieved, since accelerating on a 4% upgrade would limit the time at higher velocity. Increased braking distance and slowing to 45 mph for a station stop would limit the speed in the A9 to A8 direction.

VERMONT/SUNSET (A9)

1. This is a typical diamond interlocking. A typical directive drawing developed for A7 should suffice for defining conduit needs.
2. The distance to the A10 TC&C room is approximately 5,800 ft.; this presents no problem.
3. Maximum train speed from station A10 to A9 will be restricted to 55 mph because of curve.

4. Maximum train speed from A9 towards A10 could be accelerated beyond 55 mph after curve, although runtime savings would be minimal.

HOLLYWOOD/WESTERN (A10)

1. This is a typical non-interlocking station. A typical directive drawing for conduit should suffice.
2. Distance to All station is about 5,100 ft. which is easily within the limits.
3. Maximum train speed from All to A10 would be 55 mph. There are no curve restrictions to limit to this speed; however, this is due to the short distance, and because part of the acceleration distance is on a 2% upgrade.
4. Maximum train speed from A10 to All could be above 55 mph; acceleration is assisted by 2% downgrade in the area where the limit is 45 mph or above. Actual runtime savings would be a nominal 4 seconds (very approximate).

HOLLYWOOD/VINE (A11)

1. This is a pocket track interlocking location and will need a specific directive drawing to show the conduit and cableway requirements.
2. Distance to A12 TC&C room is approximately 4,200 ft.
3. Operation of the pocket track should be consistent with Wilshire/Alvarado pocket, unless specific requirements of the SCRTD warrant deviation.
4. Maximum train speed on both tracks and in both directions between All and station A12 will be 55 mph.

HOLLYWOOD/HIGHLAND (A12)

1. This is a typical non-interlocking station. A typical directive drawing for conduit requirements will be sufficient.
2. Long distance to A13 station (approximately 16,800 ft.) will require TC&C rooms at the vent shafts at 653 and 709. Cable lengths would be excessive without the additional rooms.
3. Curve at west end of A12, and before the crossover at Universal City (A13), will limit speed slightly for trains

in both directions. However, much of the distance can be traveled at 67 mph.

UNIVERSAL CITY (A13)

1. This is a diamond interlocking, temporary terminal station. A typical directive drawing developed for other diamond interlocking locations should be sufficient for defining the conduit needs.
2. This location could be a temporary terminal for long duration. If so, 320 ft. minimum of tail track should be provided behind the station. This would be sufficient to store a 4 car train. And also, would make a significant runtime differential of 25 seconds or more for each train.
3. Block design specifications should require train to approach signal at STOP to within 9 mph SBD in normal direction.

PHASE II INTERIM STATUS REPORT  
TRACTION POWER  
JUNE 30, 1989

Introduction

This report summarizes Phase II traction power design work accomplished during the work program period ending June 30, 1989 and describes the interim status of the products of that work at the time of the end of the work program.

Scope of Work

The following traction power design activities were assigned under task 1.2.7.A of the Contract Scope:

1. Special studies were planned in two main areas of traction power design:

- a. Emergency Backup Power Analysis - Establish the emergency backup power requirements for the Phase II alignment taking into account critical load demands for systems and facilities equipment.

This study is needed to determine a feasible and cost effective means of powering methane gas ventilation fans in the event of failure of the primary and alternative power supplies from the power utility. Five milestones were planned:

- (1) Phase II Power Requirements
- (2) Examine DWP Alternatives
- (3) Interim Status Report
- (4) Draft Report (Part B Task)
- (5) Final Report (Part B Task)

- b. Traction Power Transit Operating Model Analysis - Perform computer simulation runs (TOM runs) to establish power requirements and refine substation locations and capacity ratings for Phase II alignment.

The purpose of the analysis is to identify the potential number, location, and power capacities of traction power substations for the selected candidate route alignment and profile of Phase II. Four milestones were planned:

- (1) Alignment & Operating Parameters
- (2) Part A Interim Status Report
- (3) Draft Report (Part B Task)
- (4) Final Report (Part B Task)

2. Systems engineering Traction Power support activities were assigned in three areas:

- a. High voltage supply and distribution -- Prepare preliminary design for distribution of D.C. power and installation of 34.5 KV feeders for tunnels and stations.
- b. Support of designs for traction power substations and electric power utility service facilities -- Define electric substation and utility entrance minimum sizes and coordinate incorporation of equipment room requirements into facility design planning.
- c. Support of designs for electric power for passenger station and tunnel facilities including emergency ventilation equipment -- Review facility electric power loads for stations and vent shafts to establish required ratings of auxiliary power transformers.

#### Work Accomplished and Work Products

##### 1. Special Studies:

- a. The design of the emergency backup power supply (EBPS) system for Phase II was advanced to the "Examine DWP Alternatives" stage, but the milestone was not completed. It is recognized however, that power requirements for facilities essential to prolonged operation, in case of an event that would cause loss of primary and secondary utility power, can be directly extrapolated from the MOS-1 EBPS requirements, since both route portions are in subway, and there are no long interstation tunnel segments until the alignment north of the Hollywood Boulevard corridor needs to be considered. (Issues related to both normal and emergency power supply are yet to be addressed with regard to the tunnels through the mountains.)

Regarding EBPS design for the Wilshire and Vermont corridors, we will need to assess, more accurately than we have to date, the distance beyond which 34.5 KV voltage drop from the feed point in the MOS-1 yard becomes excessive. When Phase II design resumes, we will need to address designs considerations similar to those we addressed, with the District engineering staff, for the conceptual design of the MOS-1 EBPS.

- b. A transit operation model (TOM) analysis of traction power requirements was carried out for the Phase II alignment identified as candidate alignment No. 1 modified (CALM) based on extrapolation from results of a TOM analysis of the original starter line.

The analysis indicated that the rectifier rating selected as the standard size for MOS-1 traction power substation (TPSS) is suitable for Phase II, and that by the

"design year" (2000), double rectifier/transformer units will be needed at each Phase II TPSS.

Because the extrapolation method yields conservative results, a more exacting analysis will be performed when Phase II design work resumes, to see if single rectifier/transformers can adequately serve some TPSS locations for a significant initial operating time. A copy of the revised report is attached.

2. Support Activities:

- a. TPSS equipment room space, access and conduit requirements were given to facilities designers.
- b. High voltage supply and distribution designs were advanced to a preliminary level that enabled us to provide to passenger station designers requirements for electric power incoming service facilities, conduits, blockouts, and equipment entrance ways.
- c. Space requirements were coordinated with the facilities design group for the incoming service (DWP) and traction power substation rooms at Wilshire/Vermont, Vermont/Beverly, Vermont/Santa Monica, Hollywood/Highland, and Universal City passenger stations. Preliminary equipment layouts were performed for these rooms to confirm that the planned room configurations and allocated spaces are adequate.
- d. Facilities drawings for construction contracts B331 and B201 were marked up to identify traction power requirements including traction power conduits and other embedments as well as interfaces at panelboards, junction boxes, etc.
- e. Preliminary contact rail designs were performed for Wilshire/Vermont "Y" and MacArthur Park pocket track structure to coordinate conduit stubups and walkway locations.
- f. Design support service inputs were provided for cost estimating, scheduling, and equipment descriptions as required for development of budgets, project schedules, and contract unit descriptions.
- g. Facilities design packages were reviewed and commented on, and responses to the comments were coordinated.
- h. Support was provided for the systemwide equipment procurement study.



METRO RAIL PROJECT  
MOS-2, LPE

TRACTION POWER SUBSTATIONS  
LOCATIONS AND EQUIPMENT SIZES  
REPORT FOR CALM

Prepared for  
Southern California Rapid Transit District

by

Metro Rail Transit Consultants

February 1989

The preparation of this document has been financed in part through a grant from the U.S. Department of Transportation, Urban Mass Transportation Administration, under the Urban Mass Transportation Act of 1964, as amended, the State of California, and the Los Angeles County Transportation Commission.

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## I. EXECUTIVE SUMMARY

This report identifies the potential number and locations of traction power substations for the selected Candidate Alignment No. 1 Modified (CA1M) as shown on Figure 1. The report also evaluates the equipment sizes (ratings) necessary to meet the Design Year and Long-Range Design Standard (LRDS) operating requirements for the combined MOS-1 and MOS-2 segments as defined by DD-002<sup>1</sup>.

The methodology used to determine the spacing, locations, and rating of substations for CA1M consists of the following analytical procedures:

- o The minimum acceptable voltage at trains to provide the necessary performance for the required peak period service headway establishes the average distance between, and optimum location of, the substations.
- o Train size and service headway during peak period operation establishes the maximum load for which the traction power system should be designed.
- o The maximum load on each substation is established from the first and second procedures.
- o The required equipment continuous rating at each substation is derived from the load established by the third procedure.

These procedures have established preliminary locations and equipment ratings for CA1M which are shown in Table 1. These findings are to be confirmed by a detailed system analysis during preliminary engineering and final design phases of the project.

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<sup>1</sup> Design Directive, Accommodation of Patronage Growth, Metro Rail Project, Long-Range Design Standard.

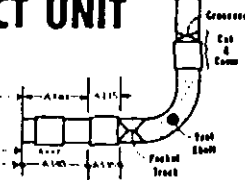
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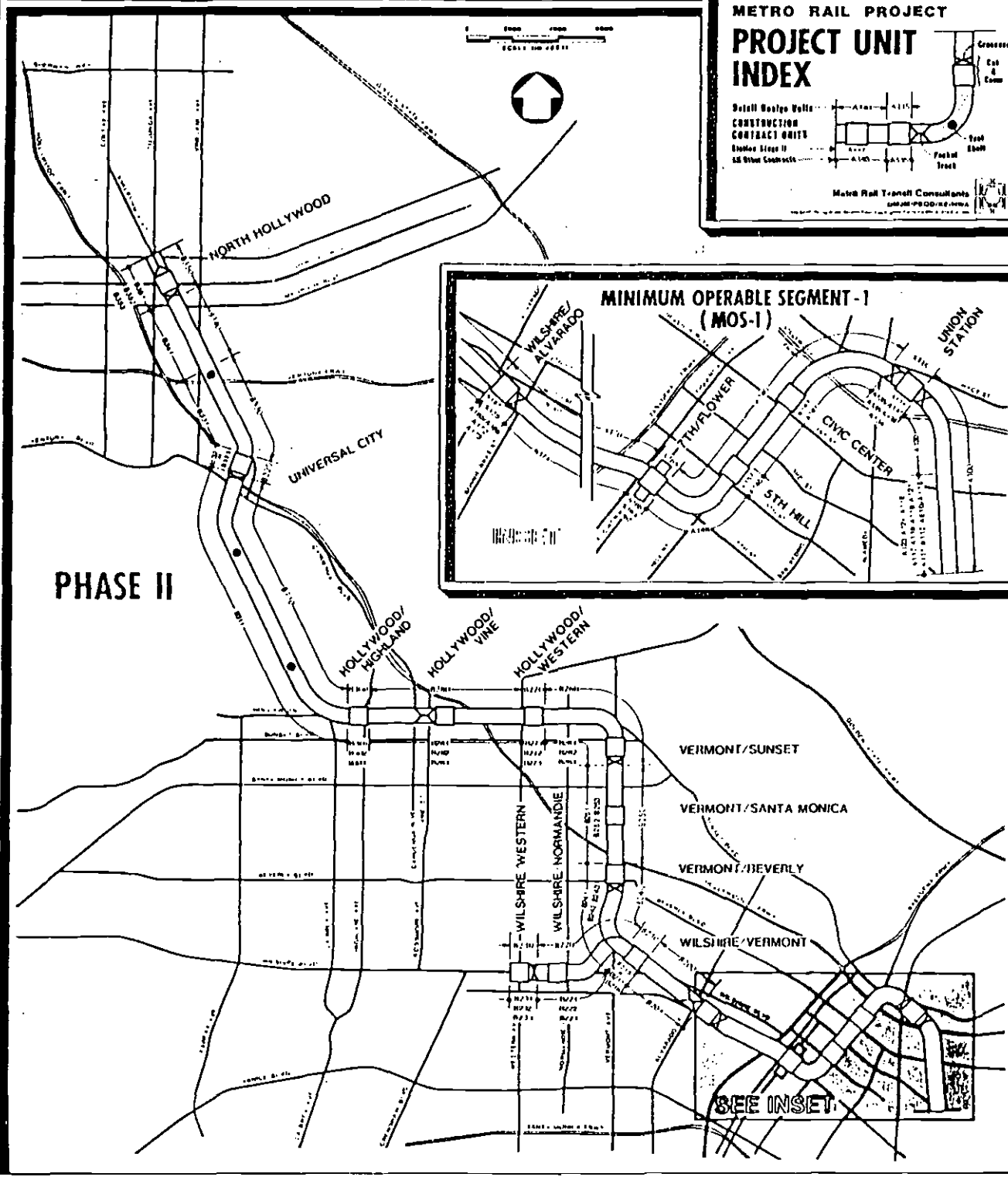
JULY 14, 1988  
as adopted by  
the SCRTO Board

## METRO RAIL PROJECT PROJECT UNIT INDEX

Detail Design Units  
CONSTRUCTION  
CONTRACT UNITS  
Station Stage II  
All Street Crossings



Metro Rail Transit Consultants  
GARDNER WHARF COMPANY



CANDIDATE ALIGNMENT NO. 1 MODIFIED (CA1M)

Figure 1

TABLE 1

CAIM

Substation Locations and Power Rectifier Ratings

<u>Substation Locations</u>	<u>Rectifier Rating (kW)</u>	
	<u>Design Year*</u>	<u>LRDS**</u>
Wilshire/Vermont	2 X 2,500	2 X 2,500
Wilshire/Western	2 X 2,500	2 x 2,500
Vermont/Beverly	2 X 2,500	2 x 2,500
Vermont/Sunset	2 X 2,500	2 x 2,500
Hollywood/Western	2 X 2,500	2 x 2,500
Hollywood/Vine	2 X 2,500	2 x 2,500
Hollywood/Highland	2 X 2,500	2 x 2,500
Vent Shaft	2 X 2,500	2 x 2,500
Universal City	2 X 2,500	2 x 2,500
North Hollywood	2 X 2,500	2 x 2,500

\* Six-car trains at 6-minute headway over branches and 3-minute headway over common alignment

\*\* Six-car trains at 3-minute headway over branches and 2-minute headway over common alignment

## II. BACKGROUND

This report provides the results of the study performed under Limited Preliminary Engineering for the selected Locally Preferred Alignment (LPA), identified as CALM. The study covers a preliminary evaluation of traction power substation locations for this alignment, the load on each substation, and the proposed dc equipment ratings. The evaluation covers the requirements for train service over the combined MOS-1 and LPA segments, for the Design Year, and Long-Range Design Standard (LRDS) as defined by Design Directive DD-002.

## III. CRITERIA AND METHODOLOGY

The determination of substation locations and capacities is based on the Metro Rail Project System Criteria for traction power and on the methodology used in a previous study<sup>1</sup> performed for the original starter line between Union and North Hollywood stations. The evaluation of substation spacing for each candidate alignment is based on the established vehicle performance characteristics, corrosion control requirements, and existing traction power wayside distribution parameters.

The evaluation of the traction power system total capacity and individual substation ratings is based on extrapolation from the results of a study performed by transit operation model (TOM) analysis of the original starter line.<sup>1</sup>

The evaluation of substation equipment sizing (rectifier and dc switchgear ratings) is based on the maximum load demand during peak period service operation and recognized standard ratings for this class of equipment.

## IV. CALM PARAMETERS

The alignment used for the study is shown diagrammatically on Figure 2 and identified as CALM. It extends from Wilshire/Alvarado Station to the LPA terminal stations identified as Wilshire/Western and North Hollywood.

The operating parameters used in performing the study are based on those given in the SEIS/SEIR<sup>2</sup> and DD-002, and are described as follows:

---

<sup>1</sup> Report WBSZ-72-G12, Traction Power System Study, July 1985.

<sup>2</sup> Draft Supplemental Environmental Impact Statement/ Subsequent Environmental Impact Report, Los Angeles Rail Rapid Transit Project, Metro Rail, November 1987.

- o The traction power system design is based on the maximum number of cars per train and shortest headway to be used during peak period service.
- o At the Design Year, six-car trains will operate at a headway of 6-minutes over the two branches and at a headway of 3-minutes over the common alignment between Wilshire/Vermont and Union stations.
- o For the LRDS, six-car trains will operate at a headway of 3-minutes over the two branches, and at a headway of 2-minutes over the common alignment.

V. TRACTION POWER SUBSTATION LOCATIONS

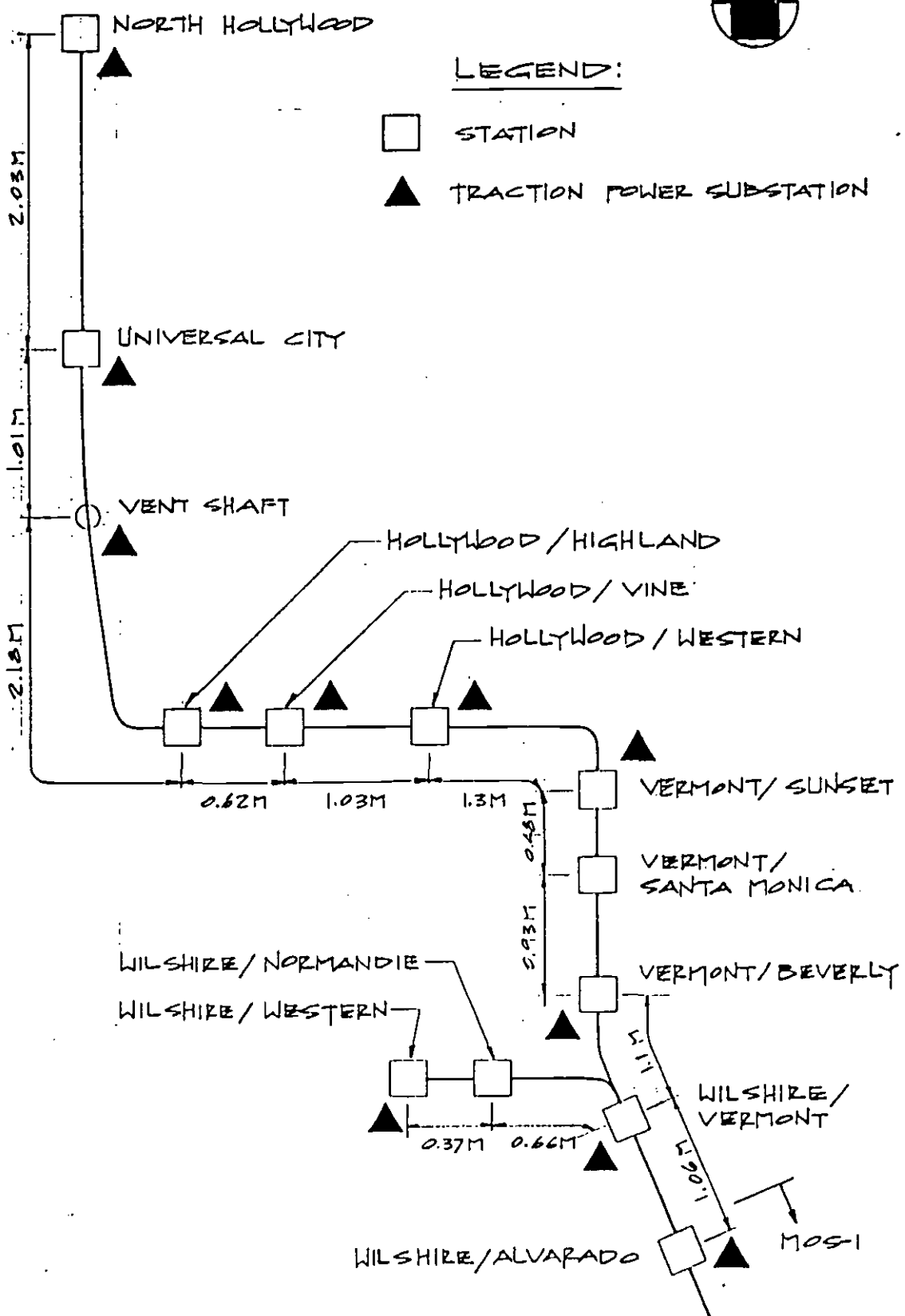
Previous traction power and corrosion control studies performed for the Metro Rail Project have confirmed that for a 750-V dc traction power system, substations should be located at stations, and with an average spacing between substations of one mile. These factors ensure that an average voltage of not less than 650 V is maintained at the vehicle power pick-up shoes during peak period service with all substations in operation. The proposed locations and estimated distances between traction power substations for CALM are shown on Figure 2.

VI. TRACTION POWER SYSTEM LOAD

The evaluation of load on the LPA segment traction power system is based on peak period service requirements, the stated criteria and methodology, and extrapolation of the data obtained from the previous study for the original starter line. Load calculations are attached as Appendix A.

A. Peak Period Service Load

The previous study<sup>1</sup> performed on the original starter line established the traction power load during peak period service. A TOM program simulation was made for six-car trains operating at 2-1/2-minute headways over a distance of 17.87 miles between the Union and North Hollywood terminal stations. The peak period load on the traction power system was established as being in the order of 72,000 kW. This data is derived from Table 3-3A of that study, which is included in Appendix B. The original traction power system incorporated 16 substations, spaced apart at distances varying between 0.6085 and 2.4256 miles.



LPA CA1M  
TRACTION POWER SUBSTATION LOCATIONS

Figure 2



The average load demand on each substation varied between 3,700 kW and 5,200 kW, with an average value of 4600 kW. The average load demand per mile of alignment was:

$$\frac{72,000}{17.87} = \underline{4063 \text{ kW}}$$

#### B. LPA Segment Peak Period Service Load

The preliminary evaluation of peak period service load on the LPA segment traction power system can be extrapolated from the data derived from the previous system study<sup>1</sup> by the following methodology:

- o The calculation of average load demand is based on the formulae used for the previous system study<sup>1</sup> to convert the output data from the TOM computer simulation program 7. These formulae are given in Appendix C.
- o The peak period LRDS load demand per mile on the original starter line traction power system, with six-car trains operating at 2-1/2-minute intervals, can be converted to an "equivalent value" average load per mile of alignment for a single car by means of the referenced formulae. This value of load is used as the basis for establishing the load on the traction power system for the LPA segment.
- o The CALM alignment between Wilshire/Alvarado and the Wilshire/Western and North Hollywood stations comprises approximately 12.82 miles of double-track mainline, with 11 passenger stations and 10 traction power substations. The traction power system capacity is based on peak period service over the two branches.

#### C. Design Year Service Load

- o At the Design Year, peak period service will be performed by six-car trains operating at headways of 3-minutes over the trunk line and 6-minutes over the two branch lines.
- o The traction power system load was calculated from the "equivalent value" and established as 7,184 kW for the trunk line and 28,615 kW for the two branch lines.

- o The average substation load demands are 3,036 kW for trunk substations and 3,179 kW for branch substations.

#### D. LRDS Service Loads

- o The LRDS peak period service will be performed by six-car trains operating at headways of 2-minutes over the trunk line and 3-minutes over the two branch lines.
- o The traction power system loads calculated from the "equivalent value" were 8,798 kW for the trunk line and 40,474 kW for the two branch lines.
- o The average substation load demands are 4,399 kW for trunk substations and 4,497 kW for branch substations.

### VII. LPA SEGMENT TRACTION POWER SUBSTATION EQUIPMENT CAPACITY

The major items of substation equipment, whose capacity is dependent on the load for peak period train operation, are the transformer/rectifier units and dc switchgear. Based on established criteria<sup>1</sup> for traction power system design, all substations shall have provision for two transformer/rectifier units of equal capacity to meet the full system LRDS service load demand and provide partial redundancy.

Each transformer/rectifier unit will have a two-hour overload capability of 163 percent of its continuous rating. This overload capability can be used to meet the peak period service load and abnormal loading conditions caused by a substation outage. This overload capability is not normally used, however, for the evaluation of transformer/rectifier rating for peak period load under normal operating conditions because of the reduction of dc voltage level at the substation bus.

The dc switchgear ratings are based on the overload capability of the selected transformer/rectifier units and on the load imposed by trains operating on each segment of railtrack supplied from the substation.

#### A. Rectifier Transformer Capacity

- o Based on the average load per mile of track, two 2,500 kW transformer rectifier units will have to be installed at all trunk and branch line substations in order to satisfy the peak period service for the Design Year.

- o The two 2,500 kW transformer rectifier units will also support the LRDS peak period service.

B. 750-V Dc Switchgear Rating

- o The switchgear bus and transformer/rectifier unit cathode circuit breaker continuous current rating will match the rectifier two-hour overload rating as follows:

- 2,500 kW transformer/rectifier unit 2-hour overload rating =  $2,500 \times 1.63 = 4,075$  kW

Therefore: Main bus and cathode circuit breaker rating will be not less than:

$$\frac{4075 \times 10^3}{750} = \underline{5433 \text{ A}}$$

- o Feeder circuit breaker current rating will be based on the LRDS service load, with 6-car trains operating at 2-minute headways. From the previous sytem study<sup>1</sup> performed for LRDS service, it was established that the required continuous current rating of the 750V dc feeder circuit breakers was 4,000 A.

END OF REPORT

APPENDIX A



METRO RAIL TRANSIT CONSULTANTS  
OMIM / PBQD / KE / HWA

LOS ANGELES METRO RAIL PROJECT	JOB NO. MOS-2 LPE	SHEET NO. 1 of 3
MOS-2 SEGMENT LPA	DESIGNED BY GMP	DATE 8/21/88
TRACTION POWER SYSTEM CALCULATIONS	APPROVED	

CALCULATION OF AVERAGE KW LOAD PER TRAIN

A. BASIS FOR CALCULATION:

- AVERAGE LOAD PER TRAIN (KW)

$$= N \times \sqrt{\frac{3600}{N \text{ HDWY}}} \times \frac{1}{2} \times \left[ \frac{\sqrt{A+B} + \sqrt{C}}{2} \right] \times V$$

$$= N \times \sqrt{\frac{3600}{N \text{ HDWY}}} \times (\text{CONSTANT } K)^2$$

WHERE:

- N = NUMBER OF CARS PER TRAIN
- HDWY = HEADWAY IN SECONDS
- V = DC VOLTAGE AT SUBSTATION BUS
- A = AVE. (ASQ-H)
- B = MIN. (ASQ-H)
- C = MAX. (ASQ-H)

O. PEAK LOAD ON ORIGINAL STARTER LINE FOR LRDS SERVICE -  
(REF. TABLE 3-3A OF JULY 1985 REPORT)

TOTAL LOAD ON TRACTION POWER SYSTEM (COLUMN 3)  
= 72600 KW

LENGTH OF STARTER LINE ROUTE ~ 17.87 MILES

AVERAGE LOAD/MILE OF ROUTE =  $\frac{72600}{17.87} = \underline{4063 \text{ KW}}$

O. EQUIVALENT LOAD OF ONE CAR/MILE OF ROUTE (KW<sub>C</sub>)

FROM 6 CAR TRAINS AT 2-1/2 MINUTE HEADWAYS

∴ KW<sub>C</sub> =  $4063 / 6 \times \sqrt{\frac{3600}{150}} = \frac{4063}{6 \times 4.9} = \underline{138 \text{ KW}}$

B. PEAK LOAD/MILE ON LPA TRACTION POWER SYSTEM.

B1. DESIGN YEAR

- TRUNK LINE - 6 CAR TRAINS AT 3 MIN HEADWAYS

$$LD/M = 6 \times \sqrt{\frac{3600}{180}} \times 138^* = \underline{3703 \text{ kW/mile}}$$

- BRANCH LINES - 6 CAR TRAINS AT 6 MIN HEADWAYS

$$LD/M = 6 \times \sqrt{\frac{3600}{360}} \times 138 = \underline{2618 \text{ kW/mile}}$$

B2. LRDS

- TRUNK LINE - 6 CAR TRAINS AT 2 MIN HEADWAYS

$$LD/M = 6 \times \sqrt{\frac{3600}{120}} \times 138 = \underline{4535 \text{ kW/mile}}$$

- BRANCH LINES - 6 CAR TRAINS AT 3 MIN HEADWAYS

$$LD/M = 6 \times \sqrt{\frac{3600}{180}} \times 138 = \underline{3703 \text{ kW/mile}}$$

C. AVERAGE LOAD ON LPA TRACTION POWER SUBSTATIONS

(CAIM ROUTE LENGTH IS 12.87 MILES OUT OF WHICH 1.94 MILES ARE FED BY TWO "TRUNK" SUBSTATIONS AND 10.93 ARE FED BY 9 "BRANCH" SUBSTATIONS)

C1. DESIGN YEAR

- TRUNK LINE - 6 CAR TRAINS AT 3 MIN HEADWAYS

$$LD/S = 3703 \times 1.94 / 2 = \underline{3036 \text{ kW}}$$

\* EQUIVALENT LOAD/CAR  $kW_c = 138 \text{ kW}$

Page 3 of 3   
Made by DAN GARCOLANO  
Date 2/10/89  
Checked by \_\_\_\_\_  
Date \_\_\_\_\_

---

◦ BRANCH LINES - 6 CAR TRAINS AT 6 MIN HEADWAYS

$$LD/S = 2618 \times 10.93 / 9 = \underline{3179 \text{ kW}}$$

C2. LRDS

◦ TRUNK LINES - 6 CAR TRAINS AT 2 MIN HEADWAYS

$$LD/S = 4535 \times 1.94 / 2 = \underline{4399 \text{ kW}}$$

◦ BRANCH LINES - 6 CAR TRAINS AT 3 MIN HEADWAYS

$$LD/S = 3703 \times 10.93 / 9 = \underline{4497 \text{ kW}}$$

APPENDIX B

TABLE 3-3A

COMPLETE STARTER LINE CONFIGURATION (2-1/2-MIN HEADWAY)  
NORMAL OPERATING CONDITION  
SUBSTATION AVERAGE LOAD AND TRAIN VOLTAGE

<u>Substation Location</u>	<u>Spacing (Miles)</u>	<u>Rectifier Rating (MW)</u>	<u>Average Load Demand (MW)</u>	<u>Required Minimum Capacity (MW)</u>	<u>Train Minimum Voltage</u>
Union	0.8356	2 x 2.5	3.1	2.1	693
Civic Center	0.4595	2 x 2.5	4.7	3.1	711
5th/Hill	0.5473	0	0	0	669
7th/Flower	1.1178	2 x 2.5	5.0	3.3	699
Wilshire/Alvarado	1.0412	2 x 2.5	4.7	3.1	694
Wilshire/Vermont	0.5981	2 x 2.5	5.1	3.4	706
Wilshire/Normandie	0.4245	0	0	0	669
Wilshire/Western	0.6085	2 x 2.5	5.1	3.4	710
Wilshire/Crenshaw	1.4161	2 x 2.5	4.6	3.1	700
Wilshire/La Brea	0.9463	2 x 2.5	4.7	3.1	692
Wilshire/Fairfax	0.8341	2 x 2.5	4.2	2.8	708
Fairfax/Beverly	1.0654	2 x 2.5	4.4	2.9	703
Fairfax/S. Monica	1.3445	2 x 2.5	4.7	3.1	690
La Brea/Sunset	0.9922	2 x 2.5	4.8	3.2	693
Hollywood/Cahuenga	2.4356	2 x 2.5	5.2	3.5	<u>648</u>
Vent Shaft: Sta. 876+63	1.0125	2 x 2.5	4.1	2.7	689
Universal City	2.1885	2 x 2.5	4.5	3.0	676
North Hollywood		2 x 2.5	3.7	2.5	

\* Average load demand divided by rectifier 2-hr overload capability of 150% rating.

\*\* Minimum voltage at a train operating between adjacent stations.

## APPENDIX C

### FORMULAE FOR SUBSTATION POWER CAPACITY CALCULATIONS

#### 1. Average RMS Current

A. Average RMS I =  
(Amperes/Vehicle)

$$\sqrt{(\text{Ave. (ASQ-H)} \times \frac{(3600)}{\text{HDWY}} + \text{min. (ASQ-H)} \times \frac{(3600)}{\text{HDWY}})}$$

B. Maximum RMS I =  
(Amperes/Vehicle)  $\sqrt{(\text{Max. (ASQ-H)} \times \frac{(3600)}{\text{HDWY}})}$

C. Average I =  $6 \frac{(\text{Ave. RMS I} + \text{Max. RMSI})}{2}$   
(Amperes/Train)

Where:

Ave. (ASQ-H) = Average Substation loading  
(Column 6 of TOM-7 Output)

Min. (ASQ-H) = Minimum Substation loading  
(Column 5 of TOM-7 Output)

Max. (ASQ-H) = Maximum Substation loading  
(Column 7 of TOM-7 Output)

HDWY = Train operating headway, seconds.

#### 2. Average Megawatts

$$\text{Average MW} = \frac{\text{Ave. I} \times \text{Voltage at Substation Bus}}{10^6}$$

$$2 \text{ Hr Loading} = \frac{\text{Ave. MW} \times C}{\text{Transformer - rectifier overload factor}}$$

Where: C = 1.2 for end stations  
C = 1.1 for nonend stations  
Transformer-rectifier overload factor = 1.5



PHASE II INTERIM STATUS REPORT  
COMMUNICATIONS  
JUNE 30, 1989

This report summarizes communications system design work for Phase II of the Metro Rail Transit system accomplished during the work program period ending June 30, 1989. The work tasks were assigned with the understanding that an interim report would be prepared at the conclusion of Part A, after which the work might be interrupted for a period of a few weeks or months. Initially the interim report was scheduled for April 28, 1989; and was subsequently rescheduled for June 30, 1989, and as of that date all Phase II work was halted. This report briefly addresses tasks that will be resumed, using the work products that are in various stages of completion as of June 30, 1989.

Limited Preliminary Engineering (LPE) refers to a work program in which certain Phase II preliminary design work was done just prior to work summarized in this Interim Report.

Work Planned

Six areas of communications systems work were planned:

1. Preliminary Design Specifications

The equipment specifications developed for MOS-1 will be reviewed for applicability and interface with Phase II procurements. The CUDs developed for Phase II will be refined as required. Development of preliminary specifications will be initiated after contract units are identified. Requirements for compatibility with MOS-1 equipment will be listed and an equipment interface listing will be established for each contract.

2. Radio, CCTV and PA Requirements

Incorporate Limited Preliminary Engineering requirements for radio, CCTV, and PA into equipment concepts.

3. Emergency and Access Management

Establish and refine emergency management, intrusion detection and access control system requirements for Phase II station and vent shafts and develop compatible design concepts for integration into MOS-1 voice and SCADA subsystem.

4. Seismic and Gas Detection

Establish and incorporate Phase II seismic and gas detection requirements into preliminary specifications.

5. Redundancy Requirements

Analyze potential requirements of redundant communications during RCC down time.

6. SCADA Integration

Perform analysis of SCADA software and hardware modifications needed for addition of the Phase II system to existing MOS-1 equipment.

Work Not Anticipated

LAPD Radio Requirements

Additional design study became necessary to consider the Los Angeles Police Department radio requirements in the underground portion of the Metro Rail, and to evaluate the design, cost and operational consequences for Phase II.

Work Accomplished and Work Products

1. Preliminary Design Specifications

In the Phase II, Part A activities, work that was begun in the LPE activities was continued, especially in the reviews and analysis of MOS-1 communications designs, for direct or modified applicability into the Phase II designs. The review and analysis extended beyond the design specifications used for procurement of the MOS-1 Communications design, into the design documents submitted by the A640 Contractor. The Contractor's designs have advanced to the review portion of Preliminary Design cycle. Using the design information from these design reviews we have started to implement basic detail designs into the Phase II preliminary work. A systems procurement study has been initiated (discussed in a separate interim report), the results of which will affect the composition of design specifications for some or all of the Phase II Communications subsystems. Communications staff support was continuously required to assist in formulating the procurement study alternatives.

2. Radio, CCTV and PA Requirements

A preliminary radio frequency (RF) power budget was developed to assess the power needed for the radio subsystem. The RF power budget is being used to determine base station repeater locations. Early preliminary design of the Communications subsystems were performed to develop approximate equipment quantities, equipment room space requirements and order of magnitude cost estimates.

### 3. Emergency and Access Management

Conceptual designs were begun, based on review of MOS-1 requirement extrapolations.

The fire detection and suppression requirements have been reviewed by the Fire/Life Safety Committee. Several modifications can be made which may result in fewer monitoring devices, and potentially, some reductions in floor space, wiring, and indications. We are in the process of verifying these potential savings at this time.

### 4. Seismic and Gas Detection

We have begun estimating gas monitoring locations and analyzer units within each passenger station and tunnel section, based on the consideration that all of Phase II locations are classified as "gassy".

Contractor implementation of the MOS-1 gas monitoring equipment has revealed that the central analyzer units, with necessary peripheral equipment, require more TC&C room space than first considered, and that all the apparatus must be powered by UPS power. These factors must be considered in Phase II equipment.

### 5. Redundancy Requirements

Consideration has been given to studying means of providing a totally independent means of communicating from underground Metro Rail locations to the "outside world" in the event of a catastrophe that would knock out the normal and emergency communication routes that go through the RCC. No specific work has begun on that study.

### 6. SCADA Integration

No specific work has been done to integrate the MOS-1 SCADA system across the boundary between it and the Phase II facilities and systems that will be under centralized supervisory control from the RCC. When Phase II design resumes, the task will begin with the assessment of approximately how much parallel capability will be needed at the RCC.

## Unanticipated Work Accomplished

### LAPD Radio Requirements

The Los Angeles Police Department has requesting additional radio frequency capability to be installed in underground portions of Metro Rail system. The design of this radio system addition is still in its conceptual phase. Major impact on floor space in the TC&C rooms and the communi-

cations equipment rooms in the Rail Control Center is anticipated.

The LAPD radio requirements have not been entered into the RF power budget discussed under #2 above.

## PHASE II INTERIM STATUS REPORT

Operations and Maintenance Planning  
June 30, 1989

### I. INTRODUCTION

The O&M discipline provides support to the O&M Committee for Phase II through participation at meetings and as a liaison with MRTC disciplines. Deliverables include a maintainability analysis report, and a maintenance equipment summary report. Due to work performed on the Hollywood Area Pocket Track Study, some work originally planned was deferred.

### II. AWP REQUIREMENTS-PART "A"

The AWP requirements for Part "A" work include the following:

1. Maintainability analysis of line sections and structures report
2. Maintenance equipment summary report

### III. ADDITIONAL WORK NOT ANTICIPATED

At the District's request, MRTC was directed to prepare an analysis to determine the optimum location for a pocket track along Hollywood Boulevard. Concerns expressed from the City of Los Angeles relating to potential negative construction impacts on the surrounding businesses have caused a re-examination of the current pocket track location on the west end of the Hollywood/Vine Station.

### IV. SUMMARY OF WORK AND DELIVERABLES

1. The major work product of the O&M Staff for Part A was preparation of the pocket track study.

Numerous working sessions were held with District representatives to discuss the operational, maintenance, business, construction, traffic and cost impacts related to locating the pocket track.

The Hollywood area pocket track report was prepared in final draft form and distributed for SCRTD to prepare the conclusion and recommendation in April, 1989.

District input on the conclusion and recommendation section was received at MRTC the week of June 19. MRTC has issued the final report.

2. Maintainability Analysis of Line and Structures

The maintainability analysis of line sections and structures has been initiated. Work to date has been identifying various maintenance procedures (e.g. custodial room/closet location access, line sump pump pit clean-out) that can be more easily accommodated through minor design features. Facility design has not yet progressed to the point at allowing completion of the analysis.

Much of the work to date has focused on the discussions and coordination of issues with in-house architects and mechanical discipline staff.

3. Maintenance Equipment Summary Report

The maintenance equipment summary report will be prepared during Part B activities. Budget limitations precluded further Part A work.

V. REVIEW OF WORK BY OTHERS

The O&M discipline has been involved in the review and comment of the following Phase II contracts:

- o Phase II Conceptual Design Review of station Contracts B240, B250, B270, B280, and B300
- o B201 Line Section Alvarado to Vermont Station (In-Progress)
- o B330 Line Section-Universal City to North Hollywood (In Progress)
- o B331 Line Section-Universal City to North Hollywood (In-Progress)

PHASE II INTERIM STATUS REPORT

Safety, Assurance and Security  
June 30, 1989

I. INTRODUCTION

This report encompasses the verification of emergency exiting capability of the following stations and the determination of the adequacy of the existing DWP water distribution system to supply the required flow/pressure to the fire suppression systems of the following station and line section contracts.

A. RED LINE STATIONS/LINE SECTIONS

1. Wilshire/Alvarado - Wilshire/Vermont Line Section, with Pocket Track (B201)
2. Wilshire/Vermont Station (B211/B215)
3. Wilshire/Vermont - Vermont/Beverly Line Section and Vermont/Beverly Station, with Crossover (B241)
4. Vermont/Beverly - Vermont/Sunset Line Section and Vermont/Santa Monica Station (B251)
5. Vermont/Sunset - Hollywood/Western Line Section and Vermont/Sunset Station, with Crossover (B261)
6. Hollywood/Western Station (B271)
7. Hollywood/Western - Hollywood/Highland Line Section and Hollywood/Vine Station, with Pocket Track (B281)
8. Hollywood/Highland Station (B301)
9. Universal City Station - Ventura Freeway Line Section, with Vent Structure (B331)

B. ORANGE LINE STATIONS/LINE SECTIONS

1. Wilshire/Vermont - Wilshire/Western Line Section and Wilshire/Normandie Station (B221)
2. Wilshire/Western Station, with Crossover and Tailtrack (B231)

## II. ANNUAL WORK PLAN REQUIREMENTS - PART "A"

### A. EMERGENCY EXITING

Review conceptual station designs to verify conformance to emergency exiting criteria requirements, using approved projected patronage data.

### B. WATER SUPPLY ANALYSIS

Perform preliminary hydraulic calculations for station and line section fire suppression systems to determine flow/pressure demands. Review DWP Service Analysis Reports to verify adequacy of existing DWP water distribution system to meet required demands.

## III. ADDITIONAL WORK THAT WAS NOT ANTICIPATED

### A. MULTIPLE STATION ENTRANCE CONFIGURATIONS

Due to the complexity of Wilshire/Vermont Station siting considerations, multiple station layouts and entrance configurations were proposed by the architectural staff. Layouts of each proposal were reviewed to determine potential impact on emergency exiting times and travel distances.

### B. DETERMINE CONCEPTUAL STAIR/ESCALATOR CAPACITY REQUIREMENTS

Using verbal information and generic sketches provided by architectural design staff, an order of magnitude number of required emergency exit lanes was determined for six (6) stations based on patronage data in D-001, Revision 1.

## IV. SUMMARY OF WORK AND DELIVERABLES

### A. WORK ORIGINATED BY SAFETY, ASSURANCE & SECURITY

1. Computer generated exiting calculations and letter reports were developed for seven (7) stations based on input provided by design staff. This information was submitted to design personnel. See Attachments A, A.1, B and C.
2. Through coordination with LAFD representatives, MRTC obtained a DWP Service Analysis Report (flow/pressure report) for the proposed point of connection to the DWP water distribution system at the B331 vent structure.



3. Computer generated hydraulic calculations were developed for the B331 tunnel fire suppression system. Calculations were based on the wet standpipe system design depicted in the prefinal (85%) design review submittal. System flow/pressure demand was compared to existing DWP capability and a letter report was issued indicating compliance with criteria requirements. See Attachment D.

#### B. REVIEW OF WORK BY OTHERS

1. As a basis for performing preliminary emergency exiting calculations for the Wilshire/Vermont Stations, preliminary architectural drawings and sketches showing proposed station layout, platform plans, ancillary plans, and station longitudinal sections were reviewed. These drawings were provided by architectural personnel. Approximately three (3) different station layouts/entrance configurations were reviewed.

Preliminary calculations for the remaining six (6) Red Line stations covered by this report were based on a verbal description of a generic station, provided by architectural staff. The Phase II final conceptual design drawings distributed for review in May 1989 were not of sufficient detail to perform emergency exiting calculations, therefore none were developed

Projected patronage data used for all preliminary emergency exiting calculations performed for Phase II was that contained in DD-001, Revision 1.

2. As a basis for performing tunnel wet standpipe system hydraulic calculations for Contract B331, the mechanical drawings issued as a part of the prefinal (85%) design review submittal were reviewed. Additionally, the DWP Service Analysis Report obtained through LAFD was reviewed in determining the capability of the tunnel wet standpipe to perform as required.

The design of utilities for the remaining Red Line stations/line sections covered by this report did not progress to a point sufficient to determine the proposed points of connection to the DWP water distribution system. Due to this fact, no work on the task of determining the adequacy of the existing water distribution system was performed.

#### IV. PLANS FOR FUTURE WORK

##### A. EMERGENCY EXITING

Preliminary emergency exiting calculations will be developed based on the final design 30% review submittal for the seven (7) Red Line stations covered by this report. Recommendations will be developed to address areas of non-compliance, if any, and to otherwise enhance station emergency exiting. Final emergency exiting calculations will be based on the final design 85% review submittals.

Upon resurrection of design activities for the two (2) Orange Line stations included in this report, the appropriate emergency exiting calculations previously developed and included in the September 1984 Metro Rail Project Station Exiting Calculations Report will be reviewed and refined to reflect revised patronage data from DD-001, Revision 1 and any proposed revisions to station configurations. Final emergency exiting calculations for these stations will be based on the final design 85% review submittals.

##### B. WATER SUPPLY

Once utilities design for the Red Line stations and line sections is developed to the degree that points of connection to the DWP water distribution system can be determined, DWP Service Analysis Reports for the appropriate locations will be requested through the project LAFD representatives. Station and line section vertical alignment and fire suppression system schematic drawings will be reviewed. Hydraulic calculations will be developed to determine flow/pressure demands at each proposed point of connection. These demands will be compared to the DWP system capability to determine compliance with criteria requirements. A final report will be issued based on the final design 60% review submittal.



**Rolf Jensen & Associates, Inc.**

FIRE PROTECTION ENGINEERS • BUILDING CODE CONSULTANTS

**RECEIVED**

**MAR 27 1989**

March 23, 1989

**FAX D.C.C.**  
**(213) 622-4670**

Mr. Malcolm Ingram  
Metro Rail Transit Consultants  
548 South Spring Street, Seventh Floor  
Los Angeles, California 90013

WILSHIRE/VERMONT EXIT CALCULATIONS

Malcolm:

Enclosed are the preliminary exit calculations for the Wilshire/Vermont Station with the maximum egress times as follows:

<u>Location of Discounted Escalator</u>	<u>Egress Time (minutes)</u>
Lower Platform (.LPE)	4.929
Upper Platform (.UPE)	4.883
Concourse (.CE)	4.954

All egress times are under 6 minutes and comply with the Fire/Life Safety Criteria.

We have the following comments on the review of the drawings.

<u>Drawing Number</u>	<u>Comment</u>
SKA-12	Auxiliary Power & Battery Room and Air Supply Room have openings directly into the emergency exit passageway which are not permitted by Section 3312(a) of the UBC. A vestibule has been previously accepted as an alternate by the Fire/Life Safety Committee for other stations.

Please give me a call if you have questions.

Sincerely,

Christopher L. Vollman, P.E.

CLV:jmp - H3275 Wilshire/Vermont

## EMERGENCY EXITING CALCULATIONS

SHT 1 OF 4WILVERDY.LPE

STATION: WILSHIRE /VERMONT      DATE: 3/22/89      BY: EHS  
 NET PLATFORM AREA: UPPER =      9010 LOWER =      12384  
 OCCUPANT LOAD CALCULATION      DESIGN YR

PEAK HOUR		A.M.	P.M.
LINK LOADS:	INBOUND	11420	4862
	OUTBOUND	5662	9766
ENTRAINING LOADS:	INBOUND	2863	1999
	OUTBOUND	806	1130

PEAK 15 MINUTE LOADS [=PEAK HR. LOAD x 1.15 / 4]

LINK LOADS:	INBOUND	3284	1398
	OUTBOUND	1628	2808
ENTRAINING LOADS:	INBOUND	824	575
	OUTBOUND	232	325
PEAK HEADWAY	INBOUND	3.0	3.0
	OUTBOUND	3.0	3.0

CALCULATED TRAIN LOAD [=PEAK 15 MIN. LINK LOAD / (NO. OF HEADWAYS)]  
 ----- [NO. OF HEADWAYS = INTEGER(15 MIN. / HEADWAY)]

	A.M.	P.M.
INBOUND	657	280
OUTBOUND	326	562

[USE NOT<1320]

PEAK ENTRAINING LOAD [=PEAK 15 MIN. LOAD x (4 x HEADWAY / 15 MIN.)]

	A.M.	P.M.
INBOUND	660	460
OUTBOUND	186	260

TOTAL OCCUPANT LOAD [=CALCULATED TRAIN LOAD + PEAK ENTRAINING LOAD]

	A.M.	P.M.
UPPER PLATFORM	1506	1580
LOWER PLATFORM	1980	1780

DENSITY [=NET PLATFORM AREA / PEAK ENTRAINING LOAD]

	A.M.	P.M.
(NOT < 4 SQ.FT. / PERSON) UPR	48.44	34.65
LWR	18.76	26.92

STATION OCCUPANT LOAD [=LOWER + UPPER]      3486      3360

TOTAL OCCUPANT LOAD FOR CALCULATIONS      3486

EMERGENCY EXIT CAPACITY

SHT 2 OF 4 WILVERDY.LPE

STATION: WILSHIRE /VERMONT

DATE: 3/22/89

BY: EHS

EXIT LANES AND CAPACITY PROVIDED

ELEMENT DIRECTION NUMBER x LANES x CAPACITY = PPM  
/ LANE

Lower Platform to Upper Platform

Stairs	-up	4	3	35	=	420
	-down	0	0	40	=	0
Escalators	-up *	3	2	35	=	210
	-down	0	0	40	=	0
Emer.	-up	4	4	35	=	560
Stairs	-down	0	0	40	=	0
* Discounted						
		34	Total			1190

Upper Platform to Concourse

Stairs	-up	4	3	35		420
	-down	0	0	40		0
Escalators	-up	4	2	35		280
	-down	0	0	40		0
Emer.	-up	2	2	35		140
Stairs	-down	0	0	40		0
			24 Total			840

Through Fare Barriers (South)

Turnstiles		4	1	25	=	100
Fare Gates		0	1	50	=	0
Service Gates		1	2	50	=	100
Emergency Gates		2	2	50	=	200
			Total			400

Through Fare Barriers (North)

Turnstiles		4	1	25		100
Fare Gates		0	1	50		0
Service Gates		1	2	50		100
Emergency Gates		2	2	50		200
			Total			400

Fare Barriers to Safe Area (South)

Stairs	-up	2	3	35	=	210
	-down	0	0	40	=	0
Escalators	-up	2	2	35	=	140
	-down	0	0	40	=	0
Emer.	-up	0	0	35	=	0
Stairs	-down	0	0	40	=	0
			Total			350

Fare Barriers to Safe Area (North)

Stairs	-up	2	3	35		210
	-down	0	0	40		0
Escalators	-up	2	2	35		140
	-down	0	0	40		0
Emer.	-up	0	0	35		0
Stairs	-down	0	0	40		0
			Total			350

EMERGENCY EXIT WIDTH TEST

Minimum Platform Exit Width

(=Net Platform Area / (75sq.Ft. per Person x 50 Persons per Ft.))  
 Minimum Width Required : Up.= 25.74 Lower= 35.38  
 Width Provided: Up.= 44.00 Lower= 62.33 (NOT REQUIRED)

FARE BARRIER TEST

	Array	South	North
Capacity of Fare Gates and Turnstiles		100	100
Percent of Total Capacity		25.00	25.00

EMERGENCY EXIT CAPACITY TESTS

SHT 3 OF 4WILVERDY.LPE

STATION: WILSHIRE /VERMONT

DATE: 3/22/89

BY: EHS

Test 1

Evacuate Total Occupant Load from Platform(s) in 4 minutes or less.

W1(waiting time at platform exits) = Occupant Load / Exit Capacity

W1U = 1246 / 840 = 1.483 Minutes  
 W1L = 1980 / 1190 = 1.664 Minutes

Test 2

Evacuate Total Occupant Load from the most remote point on the platform to a point of safety in 6 minutes or less.

Walking Time for longest exit route

T = T1 + T2 + T3 + T4 + T5 + T6

	Feet	/ FPM	=	Minutes
T1 (on lower platform)	90	200		0.450
T2 (platform to platform)-up	30	50		0.600
T3 (on upper platform)	54	200		0.270
T4 (platform to concourse)-up	19	50		0.380
T5 (on concourse to safe area)	241	200		1.205
T6 (concourse to grade)-up	40.5	50		0.810

T = 3.715

Additional Waiting Time at Platform Exits

(W1L - T1) = 1.664 - 0.450 = 1.214 Minutes  
 (W1U - W1L) = 1.483 - 1.664 = 0.000 Minutes

Additional Waiting Time at Fare Barrier

Occupant Load at Concourse = Total Occupant Load (Lower Platform) - Lower Platform Emergency Stair 4 Minute Capacity + Total Occupant Load (Upper Platform) - Upper Platform Emergency Stair 4 Minute Capacity

1980 - (4 x 560) + 1506 - (4 x 140) = 946 Patrons

W2 = Concourse Occupant Load / Gate Capacity

W2 = 946 / 800 = 1.183 Minutes  
 (W2 - W1) = 1.183 - 1.664 = 0.000 Minutes

Check for Unbalanced Flow

North: 473 / 400 = 1.183  
 South: 473 / 400 = 1.183

Additional Waiting Time at Concourse Exits

W3 = Concourse Occupant Load / Exit Capacity

W3 = 946 / 700 = 1.351 Minutes  
 (W3 - W1) = 1.351 - 1.664 = 0.000 Minutes

North: 473 / 350 = 1.351  
 South: 473 / 350 = 1.351

Total Exit Time [T + (W1 - T1) + (W1U - W1L) + (W2 - W1) + (W3 - W1)]

TOTAL = 4.929 MINUTES (NOT > 6 MINUTES)

## EMERGENCY EXITING CALCULATIONS

SHT 4 OF 4WILVERDY.LPE

STATION: WILSHIRE /VERMONT DATE: 3/22/89 BY: EHS

CRITICAL CONSIDERATION - Lower Platform Occupants Use Emer. Stairs Only.  
 Test 1

Evacuate Total Occupant Load from Platform in 4 minutes or less.

$$W1U = 1580 / 840 = 1.881 \text{ Minutes}$$

Test 2

Evacuate Total Occupant Load from the most remote point on the platform to a point of safety in 6 minutes or less.

Waiting Time at Platform Exits

$$(W1U-T3) = 1.881 - 0.270 = 1.611 \text{ Minutes}$$

Waiting Time at Fare Barrier

Occupant Load at Concourse = Total Occupant Load (upper) - Upper Platform Emergency Stair 4 Minute Capacity

$$1580 - (4 \times 140) = 1020 \text{ Patrons}$$

$$\text{North: } W2U = 510 / 400 = 1.275 \text{ Minutes}$$

$$\text{South: } W2U = 510 / 400 = 1.275 \text{ Minutes}$$

$$W2U - W1U = 1.275 - 1.881 = 0.600 \text{ Minutes}$$

Waiting Time at Concourse Exits

$$\text{North: } W3U = 510 / 350 = 1.457 \text{ Minutes}$$

$$\text{South: } W3U = 510 / 350 = 1.457 \text{ Minutes}$$

$$W3U - W1U = 1.457 - 1.881 = 0.000 \text{ Minutes}$$

Total Exit Time  $[(T3+T4+T5+T6)+(W1U-T3)+(W2U-W1U)+(W3U-W1U)]$

Upper Platform

$$\text{Total} = 4.276 \text{ Minutes [Not } > 6 \text{ Minutes]}$$

Total Exit Time  $[\text{Occupant Load} / \text{Emer. Stair Capacity}]$

Lower Platform

$$\text{Total} = 1980 / 560 = 3.536 \text{ Minutes}$$

## EMERGENCY EXITING CALCULATIONS

SHT 1 OF 4WILVERDY.UPE

STATION: WILSHIRE /VERMONT  
 NET PLATFORM AREA: UPPER =  
 OCCUPANT LOAD CALCULATION

DATE: 3/22/89  
 9010 LOWER =

BY: EHS  
 12384  
 DESIGN YR

PEAK HOUR		A.M.	P.M.
LINK LOADS:	INBOUND	11420	4862
	OUTBOUND	5662	9766
ENTRAINING LOADS:	INBOUND	2863	1999
	OUTBOUND	806	1130

PEAK 15 MINUTE LOADS [=PEAK HR. LOAD x 1.15 / 4]

LINK LOADS:	INBOUND	3284	1398
	OUTBOUND	1628	2808
ENTRAINING LOADS:	INBOUND	824	575
	OUTBOUND	232	325
PEAK HEADWAY	INBOUND	3.0	3.0
	OUTBOUND	3.0	3.0

CALCULATED TRAIN LOAD [=PEAK 15 MIN. LINK LOAD / (NO. OF HEADWAYS)]

		A.M.	P.M.
	INBOUND	657	280
	OUTBOUND	326	562

[USE NOT <1320]

PEAK ENTRAINING LOAD [=PEAK 15 MIN. LOAD x (4 x HEADWAY / 15 MIN.)]

		A.M.	P.M.
	INBOUND	660	460
	OUTBOUND	186	260

TOTAL OCCUPANT LOAD [=CALCULATED TRAIN LOAD + PEAK ENTRAINING LOAD]

		A.M.	P.M.
	UPPER PLATFORM	1506	1580
	LOWER PLATFORM	1980	1780

DENSITY [=NET PLATFORM AREA / PEAK ENTRAINING LOAD]

		A.M.	P.M.
(NOT < 4 SQ.FT. / PERSON)	UPR	48.44	34.65
	LWR	18.76	26.92

STATION OCCUPANT LOAD [=LOWER + UPPER] 3486 3360

TOTAL OCCUPANT LOAD FOR CALCULATIONS 3486



EMERGENCY EXIT CAPACITY

SHT 2 OF 4WILVERDY.UPE

STATION: WILSHIRE /VERMONT DATE: 3/22/89 BY: EHS

EXIT LANES AND CAPACITY PROVIDED

ELEMENT DIRECTION NUMBER x LANES x CAPACITY = PPM / LANE

Lower Platform to Upper Platform

Stairs	-up	4	3	35	=	420
	-down	0	0	40	=	0
Escalators	-up	4	2	35	=	280
	-down	0	0	40	=	0
Emer.	-up	4	4	35	=	560
Stairs	-down	0	0	40	=	0

36 Total 1260

Through Fare Barriers (South)

Turnstiles		4	1	25	=	100
Fare Gates		0	1	50	=	0
Service Gates		1	2	50	=	100
Emergency Gates		2	2	50	=	200

Total 400

Fare Barriers to Safe Area (South)

Stairs	-up	2	3	35	=	210
	-down	0	0	40	=	0
Escalators	-up	2	2	35	=	140
	-down	0	0	40	=	0
Emer.	-up	0	0	35	=	0
Stairs	-down	0	0	40	=	0

Total 350

Upper Platform to Concourse

Stairs	-up	4	3	35	420
	-down	0	0	40	0
Escalators	-up	3	2	35	210
	-down	0	0	40	0
Emer.	-up	2	2	35	140
Stairs	-down	0	0	40	0

\* Discounted 22 Total 770

Through Fare Barriers (North)

Turnstiles		4	1	25	100
Fare Gates		0	1	50	0
Service Gates		1	2	50	100
Emergency Gates		2	2	50	200

Total 400

Fare Barriers to Safe Area (North)

Stairs	-up	2	3	35	210
	-down	0	0	40	0
Escalators	-up	2	2	35	140
	-down	0	0	40	0
Emer.	-up	0	0	35	0
Stairs	-down	0	0	40	0

Total 350

EMERGENCY EXIT WIDTH TEST

Minimum Platform Exit Width

[=Net Platform Area / (75Sq.Ft. per Person x 50 Persons per Ft.)]  
 Minimum Width Required : Up.= 25.74 Lower= 35.38  
 Width Provided: Up.= 40.33 Lower= 66.00 (NOT REQUIRED)

FARE BARRIER TEST

Capacity of Fare Gates and Turnstiles	Array	South	North
Percent of Total Capacity		100	100
		25.00	25.00

EMERGENCY EXIT CAPACITY TESTS

SHT 3 OF 4WILVERDY.UPE

STATION: WILSHIRE /VERMONT

DATE: 3/22/89

BY: EHS

Test 1

Evacuate Total Occupant Load from Platform(s) in 4 minutes or less.

W1(waiting time at platform exits) = Occupant Load / Exit Capacity

W1U = 1246 / 770 = 1.618 Minutes  
 W1L = 1980 / 1260 = 1.571 Minutes

Test 2

Evacuate Total Occupant Load from the most remote point on the platform to a point of safety in 6 minutes or less.

Walking Time for longest exit route

T = T1 + T2 + T3 + T4 + T5 + T6

	Feet	/ FPM	=	Minutes
T1 (on lower platform)	90	200		0.450
T2 (platform to platform)-up	30	50		0.600
T3 (on upper platform)	54	200		0.270
T4 (platform to concourse)-up	19	50		0.380
T5 (on concourse to safe area)	241	200		1.205
T6 (concourse to grade)-up	40.5	50		0.810

T = 3.715

Additional Waiting Time at Platform Exits

(W1L - T1) = 1.571 - 0.450 = 1.121 Minutes  
 (W1U - W1L) = 1.618 - 1.571 = 0.047 Minutes

Additional Waiting Time at Fare Barrier

Occupant Load at Concourse = Total Occupant Load (Lower Platform) - Lower Platform Emergency Stair 4 Minute Capacity + Total Occupant Load (Upper Platform) - Upper Platform Emergency Stair 4 Minute Capacity

1980 - (4 x 560) + 1506 - (4 x 140) = 946 Patrons

W2 = Concourse Occupant Load / Gate Capacity

W2 = 946 / 800 = 1.183 Minutes  
 (W2 - W1) = 1.183 - 1.618 = 0.000 Minutes

Check for Unbalanced Flow

North: 473 / 400 = 1.183  
 South: 473 / 400 = 1.183

Additional Waiting Time at Concourse Exits

W3 = Concourse Occupant Load / Exit Capacity

W3 = 946 / 700 = 1.351 Minutes  
 (W3 - W1) = 1.351 - 1.618 = 0.000 Minutes

North: 473 / 350 = 1.351  
 South: 473 / 350 = 1.351

Total Exit Time [T + (W1 - T1) + (W1U - W1L) + (W2 - W1) + (W3 - W1)]

TOTAL = 4.883 MINUTES [NOT > 6 MINUTES]

## EMERGENCY EXITING CALCULATIONS

SHT 4 OF 4 WILVERDY.UPE

STATION: WILSHIRE / VERMONT DATE: 3/22/89 BY: EHS

CRITICAL CONSIDERATION - Lower Platform Occupants Use Emer. Stairs Only.  
 Test 1

Evacuate Total Occupant Load from Platform in 4 minutes or less.

W1U = 1580 / 770 = 2.052 Minutes

Test 2

Evacuate Total Occupant Load from the most remote point on the platform to a point of safety in 6 minutes or less.

Waiting Time at Platform Exits

(W1U-T3) = 2.052 - 0.270 = 1.782 Minutes

Waiting Time at Fare Barrier

Occupant Load at Concourse = Total Occupant Load (upper) - Upper Platform Emergency Stair 4 Minute Capacity

1580 - (4 x 140) = 1020 Patrons

North: W2U = 510 / 400 = 1.275 Minutes

South: W2U = 510 / 400 = 1.275 Minutes

W2U - W1U = 1.275 - 2.052 = 0.000 Minutes

Waiting Time at Concourse Exits

North: W3U = 510 / 350 = 1.457 Minutes

South: W3U = 510 / 350 = 1.457 Minutes

W3U - W1U = 1.457 - 2.052 = 0.000 Minutes

Total Exit Time [(T3+T4+T5+T6)+(W1U-T3)+(W2U-W1U)+(W3U-W1U)]

Upper Platform

Total = 4.447 Minutes [Not > 6 Minutes]

Total Exit Time [=Occupant Load / Emer. Stair Capacity]

Lower Platform

Total = 1980 / 560 = 3.536 Minutes

## EMERGENCY EXITING CALCULATIONS

SHT 1 OF 4WILVERDY.CE

STATION: WILSHIRE /VERMONT      DATE: 3/22/89      BY: EHS  
 NET PLATFORM AREA: UPPER =      9010 LOWER =      12384  
 OCCUPANT LOAD CALCULATION      DESIGN YR

PEAK HOUR		A.M.	P.M.
LINK LOADS:	INBOUND	11420	4862
	OUTBOUND	5662	9766
ENTRAINING LOADS:	INBOUND	2863	1999
	OUTBOUND	806	1130

PEAK 15 MINUTE LOADS [=PEAK HR. LOAD x 1.15 / 4]

LINK LOADS:	INBOUND	3284	1398
	OUTBOUND	1628	2808
ENTRAINING LOADS:	INBOUND	824	575
	OUTBOUND	232	325
PEAK HEADWAY	INBOUND	3.0	3.0
	OUTBOUND	3.0	3.0

CALCULATED TRAIN LOAD [=PEAK 15 MIN. LINK LOAD / (NO. OF HEADWAYS)]

	A.M.	P.M.
INBOUND	657	280
OUTBOUND	326	562

[USE NOT<1320]

PEAK ENTRAINING LOAD [=PEAK 15 MIN. LOAD x (4 x HEADWAY / 15 MIN.)]

	A.M.	P.M.
INBOUND	660	460
OUTBOUND	186	260

TOTAL OCCUPANT LOAD [=CALCULATED TRAIN LOAD + PEAK ENTRAINING LOAD]

	A.M.	P.M.
UPPER PLATFORM	1506	1580
LOWER PLATFORM	1980	1780

DENSITY [=NET PLATFORM AREA / PEAK ENTRAINING LOAD]

	A.M.	P.M.
(NOT < 4 SQ.FT. / PERSON) UPR	48.44	34.65
LWR	18.76	26.92

STATION OCCUPANT LOAD [=LOWER + UPPER]      3486      3360

TOTAL OCCUPANT LOAD FOR CALCULATIONS      3486

EMERGENCY EXIT CAPACITY

SHT 2 OF 4WILVERDY.CE

STATION: WILSHIRE /VERMONT

DATE: 3/22/89

BY: EHS

EXIT LANES AND CAPACITY PROVIDED

ELEMENT DIRECTION NUMBER x LANES x CAPACITY = PPM  
/ LANE

Lower Platform to Upper Platform

Stairs	-up	4	3	35	=	420
	-down	0	0	40	=	0
Escalators	-up	4	2	35	=	280
	-down	0	0	40	=	0
Emer.	-up	4	4	35	=	560
Stairs	-down	0	0	40	=	0
			36	Total		1260

Through Fare Barriers (South)

Turnstiles		4	1	25	=	100
Fare Gates		0	1	50	=	0
Service Gates		1	2	50	=	100
Emergency Gates		2	2	50	=	200
				Total		400

Fare Barriers to Safe Area (South)

Stairs	-up	2	3	35	=	210
	-down	0	0	40	=	0
Escalators	-up *	1	2	35	=	70
	-down	0	0	40	=	0
Emer.	-up	0	0	35	=	0
Stairs	-down	0	0	40	=	0
				Total		280

\* Discounted

Upper Platform to Concourse

Stairs	-up	4	3	35	=	420
	-down	0	0	40	=	0
Escalators	-up	4	2	35	=	280
	-down	0	0	40	=	0
Emer.	-up	2	2	35	=	140
Stairs	-down	0	0	40	=	0
			24	Total		840

Through Fare Barriers (North)

Turnstiles		4	1	25	=	100
Fare Gates		0	1	50	=	0
Service Gates		1	2	50	=	100
Emergency Gates		2	2	50	=	200
				Total		400

Fare Barriers to Safe Area (North)

Stairs	-up	2	3	35	=	210
	-down	0	0	40	=	0
Escalators	-up	2	2	35	=	140
	-down	0	0	40	=	0
Emer.	-up	0	0	35	=	0
Stairs	-down	0	0	40	=	0
				Total		350

EMERGENCY EXIT WIDTH TEST

Minimum Platform Exit Width

[=Net Platform Area / (75Sq.Ft. per Person x 50 Persons per Ft.)]  
 Minimum Width Required : Up.= 25.74 Lower= 35.38  
 Width Provided: Up.= 44.00 Lower= 66.00 [NOT<REQUIRED]

FARE BARRIER TEST

	Arrav	South	North
Capacity of Fare Gates and Turnstiles		100	100
Percent of Total Capacity		25.00	25.00

EMERGENCY EXIT CAPACITY TESTS

SHT 3 OF 4WILVERDY.CE

STATION: WILSHIRE /VERMONT

DATE: 3/22/89

BY: EHS

Test 1

Evacuate Total Occupant Load from Platform(s) in 4 minutes or less.

W1 (waiting time at platform exits) = Occupant Load / Exit Capacity

W1U = 1246 / 840 = 1.483 Minutes  
 W1L = 1980 / 1260 = 1.571 Minutes

Test 2

Evacuate Total Occupant Load from the most remote point on the platform to a point of safety in 6 minutes or less.

Walking Time for longest exit route

T = T1 + T2 + T3 + T4 + T5 + T6

	Feet	/ FPM	=	Minutes
T1 (on lower platform)	90	200	=	0.450
T2 (platform to platform)-up	30	50	=	0.600
T3 (on upper platform)	54	200	=	0.270
T4 (platform to concourse)-up	19	50	=	0.380
T5 (on concourse to safe area)	241	200	=	1.205
T6 (concourse to grade)-up	40.5	50	=	0.810

T = 3.715

Additional Waiting Time at Platform Exits

(W1L - T1) = 1.571 - 0.450 = 1.121 Minutes  
 (W1U - W1L) = 1.483 - 1.571 = 0.000 Minutes

Additional Waiting Time at Fare Barrier

Occupant Load at Concourse = Total Occupant Load (Lower Platform) - Lower Platform Emergency Stair 4 Minute Capacity + Total Occupant Load (Upper Platform) - Upper Platform Emergency Stair 4 Minute Capacity

1980 - (4 x 560) + 1506 - (4 x 140) = 946 Patrons

W2 = Concourse Occupant Load / Gate Capacity

W2 = 946 / 800 = 1.183 Minutes  
 (W2 - W1) = 1.183 - 1.571 = 0.000 Minutes

Check for Unbalanced Flow

North: 473 / 400 = 1.183  
 South: 473 / 400 = 1.183

Additional Waiting Time at Concourse Exits

W3 = Concourse Occupant Load / Exit Capacity

W3 = 946 / 630 = 1.502 Minutes  
 (W3 - W1) = 1.689 - 1.571 = 0.118 Minutes

North: 473 / 350 = 1.351  
 South: 473 / 280 = 1.689

Total Exit Time [ = T + (W1 - T1) + (W1U - W1L) + (W2 - W1) + (W3 - W1) ]

TOTAL = 4.954 MINUTES (NOT > 6 MINUTES)

## EMERGENCY EXITING CALCULATIONS

SHT 4 OF 4WILVERDY.CE

STATION: WILSHIRE /VERMONT DATE: 3/22/89 BY: EHS

CRITICAL CONSIDERATION - Lower Platform Occupants Use Emer. Stairs Only.  
 Test 1

Evacuate Total Occupant Load from Platform in 4 minutes or less.

$$W1U = 1580 / 840 = 1.881 \text{ Minutes}$$

## Test 2

Evacuate Total Occupant Load from the most remote point on the platform to a point of safety in 6 minutes or less.

## Waiting Time at Platform Exits

$$(W1U-T3) = 1.881 - 0.270 = 1.611 \text{ Minutes}$$

## Waiting Time at Fare Barrier

Occupant Load at Concourse = Total Occupant Load (Upper) - Upper Platform Emergency Stair 4 Minute Capacity

$$1580 - (4 \times 140) = 1020 \text{ Patrons}$$

$$\text{North: } W2U = 510 / 400 = 1.275 \text{ Minutes}$$

$$\text{South: } W2U = 510 / 400 = 1.275 \text{ Minutes}$$

$$W2U - W1U = 1.275 - 1.881 = 0.000 \text{ Minutes}$$

## Waiting Time at Concourse Exits

$$\text{North: } W3U = 510 / 350 = 1.457 \text{ Minutes}$$

$$\text{South: } W3U = 510 / 280 = 1.821 \text{ Minutes}$$

$$W3U - W1U = 1.821 - 1.881 = 0.000 \text{ Minutes}$$

$$\text{Total Exit Time } [(T3+T4+T5+T6) + (W1U-T3) + (W2U-W1U) + (W3U-W1U)]$$

Upper Platform

$$\text{Total} = 4.276 \text{ Minutes [Not } > 6 \text{ Minutes]}$$

$$\text{Total Exit Time } [= \text{Occupant Load} / \text{Emer. Stair Capacity}]$$

Lower Platform

$$\text{Total} = 1980 / 560 = 3.536 \text{ Minutes}$$



**Rolf Jensen & Associates, Inc.**

FIRE PROTECTION ENGINEERS • BUILDING CODE CONSULTANTS

June 21, 1989

FEDERAL EXPRESS  
(213) 612-7000

Mr. Malcolm Ingram  
Metro Rail Transit Consultants  
548 South Spring Street, 7th Floor  
Los Angeles, California 90013

WILSHIRE/VERMONT  
PRELIMINARY ARCHITECTURAL REVIEW

Malcolm:

We have reviewed the drawings and sketches which accompanied your speed letter of June 9, 1989.

Updated exit calculations are attached. In all three sets, one escalator was discounted. Maximum egress times are as follows:

<u>Discounted Escalator at</u>	<u>Egress Time, Minutes</u>
Lower Platform (.LPE)	6.378
Upper Platform (.UPE)	6.891
Concourse (.CE)	6.338

All of the total exit times exceed 6 minutes. The exit capacity does not meet the Fire/Life Safety Criteria. Upper platform exiting capacity needs to be increased.

We have the following comments from our review of the drawings:

<u>Ref No.</u>	<u>Drawing</u>	<u>Comment</u>
1	SKA-02 SKA-07	Guardrails are needed opposite doors from ancillary spaces to trainway.
2	SKA-12 SKA-15	In Aux. Power Room batteries should be separated from other equipment for ventilation purposes.
3	SKA-13	Ventilation and generator exhaust may be a problem. Another shaft may be needed to serve the emergency generator room.



<u>Ref No.</u>	<u>Drawing</u>	<u>Comment</u>
4	SKA-25	The lower platform is more than 80 feet below finished grade. Protected level separation or other special protection features will be needed to comply with Fire/Life Safety Criteria 2.2.2.3.

If you have a question or would like to discuss our comments, please call.

Sincerely,



David R. Fiedler, P.E.

DRF:jmp - Wilshire/Vermont

Enclosure

5381C

## EMERGENCY EXITING CALCULATIONS

SHT 1 OF 4 ~~WYTCAS/EP~~

STATION: ~~XXXXXXXXXX~~ DATE: ~~8/21/89~~ BY: CAF  
 NET PLATFORM AREA: UPPER = 9501 LOWER = 14624  
 OCCUPANT LOAD CALCULATION DESIGN YR

PEAK HOUR		A.M.	P.M.
LINK LOADS:	INBOUND	11420	4862
	OUTBOUND	5662	9766
ENTRAINING LOADS:	INBOUND	2863	1999
	OUTBOUND	806	1130
PEAK 15 MINUTE LOADS (=PEAK HR. LOAD x 1.15 / 4)			
LINK LOADS:	INBOUND	3284	1398
	OUTBOUND	1628	2608
ENTRAINING LOADS:	INBOUND	624	575
	OUTBOUND	232	325
PEAK HEADWAY	INBOUND	3.0	3.0
	OUTBOUND	3.0	3.0

CALCULATED TRAIN LOAD (=PEAK 15 MIN. LINK LOAD / (NO. OF HEADWAYS))  
 ----- (NO. OF HEADWAYS = INTEGER(15 MIN. / HEADWAY))

	A.M.	P.M.
INBOUND	657	280
OUTBOUND	326	562

(USE NOT<1320)

PEAK ENTRAINING LOAD (=PEAK 15 MIN. LOAD x (4 x HEADWAY / 15 MIN.))  
 -----

	A.M.	P.M.
INBOUND	660	460
OUTBOUND	186	260

TOTAL OCCUPANT LOAD (=CALCULATED TRAIN LOAD + PEAK ENTRAINING LOAD)  
 -----

	A.M.	P.M.
UPPER PLATFORM	1506	1560
LOWER PLATFORM	1460	1730

DENSITY (=NET PLATFORM AREA / PEAK ENTRAINING LOAD)  
 -----

	A.M.	P.M.
(NOT < 4 SQ.FT. / PERSON) UPR	51.08	35.54
LWR	22.16	31.79

STATION OCCUPANT LOAD (=LOWER + UPPER) 3486 3360

TOTAL OCCUPANT LOAD FOR CALCULATIONS 3486

EMERGENCY EXIT CAPACITY

SHT 2 OF 4 WVDYCAF.LPE

STATION: WILSHIRE /VERMONT DATE: 6/21/89 By: CAF

EXIT LANES AND CAPACITY PROVIDED

ELEMENT DIRECTION NUMBER x LANES x CAPACITY = PPM  
/ LANE

Lower Platform to Upper Platform

Stairs	-up	2	3	35	=	210
	-down	0	0	40	=	0
Escalators	-up *	3	2	35	=	210
	-down	0	0	40	=	0
Emer.	-up	4	4	35	=	560
Stairs	-down	0	0	40	=	0

\* Discounted

28 Total 980

Through Fare Barriers (South)

Turnstiles		4	1	25	=	100
Fare Gates		0	1	50	=	0
Service Gates		0	2	50	=	0
Emergency Gates		1	2	50	=	100

Total 200

Fare Barriers to Safe Area (South)

Stairs	-up	2	3	35	=	210
	-down	0	0	40	=	0
Escalators	-up	2	2	35	=	140
	-down	0	0	40	=	0
Emer.	-up	0	0	35	=	0
Stairs	-down	0	0	40	=	0

Total 350

Upper Platform to Concourse

Stairs	-up	2	3	35	210
	-down	0	0	40	0
Escalators	-up	2	2	35	140
	-down	0	0	40	0
Emer.	-up	2	2	35	140
Stairs	-down	0	0	40	0

14 Total 490

Through Fare Barriers (North)

Turnstiles		4	1	25	100
Fare Gates		0	1	50	0
Service Gates		0	2	50	0
Emergency Gates		1	2	50	100

Total 200

Fare Barriers to Safe Area (North)

Stairs	-up	2	3	35	210
	-down	0	0	40	0
Escalators	-up	2	2	35	140
	-down	0	0	40	0
Emer.	-up	0	0	35	0
Stairs	-down	0	0	40	0

Total 350

EMERGENCY EXIT WIDTH TEST

Minimum Platform Exit Width

(=Net Platform Area / (780.Ft. per Person x 50 Persons per Ft.))

Minimum Width Required: Up.= 27.15 Lower= 41.78

Width Provided: Up.= 25.67 Lower= 51.33 [NOT REQUIRED]

FARE BARRIER TEST

	Array	South	North
Capacity of Fare Gates and Turnstiles		100	100
Percent of Total Capacity		50.00	50.00

EMERGENCY EXIT CAPACITY TESTS

SHT 3 OF 4 WVDYCAF.LPE

STATION: WILSHIRE /VERMONT DATE: 6/21/69 BY: CAF

Test 1

Evacuate Total Occupant Load from Platform(s) in 4 minutes or less.

W1(waiting time at platform exits) = Occupant Load / Exit Capacity

W1U = 1506 / 490 = 3.073 Minutes  
 W1L = 1980 / 980 = 2.020 Minutes

Test 2

Evacuate Total Occupant Load from the most remote point on the platform to a point of safety in 6 minutes or less.

Walking Time for longest exit route

T = T1 + T2 + T3 + T4 + T5 + T6

	Feet	/ FPM	=	Minutes
T1 (on lower platform)	96	200	=	0.480
T2 (platform to platform)-up	50	50	=	0.600
T3 (on upper platform)	142	200	=	0.710
T4 (platform to concourse)-up	21	50	=	0.420
T5 (on concourse to safe area)	195	200	=	0.975
T6 (concourse to grade)-up	50	50	=	0.600

T = 3.785

Additional Waiting Time at Platform Exits

(W1L - T1) = 2.020 - 0.480 = 1.540 Minutes  
 (W1U - W1L) = 3.073 - 2.020 = 1.053 Minutes

Additional Waiting Time at Fare Barrier

Occupant Load at Concourse = Total Occupant Load (Lower Platform) - Lower Platform Emergency Stair 4 Minute Capacity  
 - Total Occupant Load (Upper Platform) - Upper Platform Emergency Stair 4 Minute Capacity

1980 - 14 x 560 + 1506 - 14 x 140 = 946 Patrons

W2 = Concourse Occupant Load / Gate Capacity

W2 = 946 / 400 = 2.365 Minutes  
 (W2 - W1) = 2.365 - 3.073 = 0.000 Minutes

Check for Unbalanced Flow

North: 473 / 200 = 2.365  
 South: 473 / 200 = 2.365

Additional Waiting Time at Concourse Exits

W3 = Concourse Occupant Load / Exit Capacity

W3 = 946 / 700 = 1.351 Minutes  
 (W3 - W1) = 1.351 - 3.073 = 0.000 Minutes

North: 473 / 350 = 1.351  
 South: 473 / 350 = 1.351

Total Exit Time (T + (W1 - T1) + (W1U - W1L) + (W2 - W1) + (W3 - W1))

TOTAL = 2.378 MINUTES (NOT > 5 MINUTES)

## EMERGENCY EXITING CALCULATIONS

SHT 4 OF 4 WVDYCAF.LPE

STATION: WILSHIRE /VERMONT DATE: 6/21/89 BY: CAF

SPECIAL CONSIDERATION - Lower Platform Occupants Use Emer. Stairs Only.  
Test 1

Evacuate Total Occupant Load from Platform in 4 minutes or less.

$$W1U = 1580 / 490 = 3.224 \text{ Minutes}$$

Test 2

Evacuate Total Occupant Load from the most remote point on the platform to a point of safety in 6 minutes or less.

Waiting Time at Platform Exits

$$(W1U-T3) = 3.224 - 0.710 = 2.514 \text{ Minutes}$$

Waiting Time at Fare Barrier

Occupant Load at Concourse = Total Occupant Load (upper) - Upper Platform Emergency Stair 4 Minute Capacity

$$1580 - (4 \times 140) = 1020 \text{ Patrons}$$

$$\text{North: } W2U = 510 / 200 = 2.550 \text{ Minutes}$$

$$\text{South: } W2U = 510 / 200 = 2.550 \text{ Minutes}$$

$$U - W1U = 2.550 - 3.224 = 0.000 \text{ Minutes}$$

Waiting Time at Concourse Exits

$$\text{North: } W3U = 510 / 350 = 1.457 \text{ Minutes}$$

$$\text{South: } W3U = 510 / 350 = 1.457 \text{ Minutes}$$

$$W3U - W1U = 1.457 - 3.224 = 0.000 \text{ Minutes}$$

Total Exit Time (=T3+T4+T5+T6)-(W1U-T3)+(W2U-W1U)+(W3U-W1U)

Upper Platform

$$\text{Total} = 5.219 \text{ Minutes (Not } > 6 \text{ Minutes)}$$

Total Exit Time [=Occupant Load / Emer. Stair Capacity]

Lower Platform

$$\text{Total} = 1980 / 560 = 3.536 \text{ Minutes}$$

## EMERGENCY EXITING CALCULATIONS

SHT 1 OF 4 ~~WVTCAP-108~~

STATION: ~~XXXXXXXXXX~~ DATE: 6/21/88 BY: CBF  
 NET PLATFORM AREA: UPPER = 9501 LOWER = 14824  
 OCCUPANT LOAD CALCULATION DESIGN YR

PEAK HOUR A.M. F.M.

LINK LOADS: INBOUND 11420 4862  
 OUTBOUND 5662 9766

ENTRAINING LOADS: INBOUND 2863 1999  
 OUTBOUND 806 1130

PEAK 15 MINUTE LOADS [=PEAK HR. LOAD x 1.15 / 4]

LINK LOADS: INBOUND 3284 1398  
 OUTBOUND 1628 2808

ENTRAINING LOADS: INBOUND 324 575  
 OUTBOUND 232 325

PEAK HEADWAY INBOUND 3.0 3.0  
 OUTBOUND 3.0 3.0

CALCULATED TRAIN LOAD [=PEAK 15 MIN. LINK LOAD / (NO. OF HEADWAYS)]  
 (NO. OF HEADWAYS = INTEGER (15 MIN. / HEADWAY))

A.M. F.M.  
 INBOUND 657 280  
 OUTBOUND 326 562

(USE NOT < 1320)

PEAK ENTRAINING LOAD [=PEAK 15 MIN. LOAD x (4 x HEADWAY / 15 MIN.)]

A.M. F.M.  
 INBOUND 560 460  
 OUTBOUND 186 260

TOTAL OCCUPANT LOAD [=CALCULATED TRAIN LOAD + PEAK ENTRAINING LOAD]

A.M. F.M.  
 UPPER PLATFORM 1506 1560  
 LOWER PLATFORM 1980 1760

DENSITY [=NET PLATFORM AREA / PEAK ENTRAINING LOAD]

(NOT < 4 SQ.FT. / PERSON) A.M. F.M.  
 UPR 51.08 35.54  
 LWR 22.16 31.79

STATION OCCUPANT LOAD [=LOWER + UPPER] 3486 3360

TOTAL OCCUPANT LOAD FOR CALCULATIONS 3486

EMERGENCY EXIT CAPACITY

SHT 2 OF 4 WVDYCAF.UPE

STATION: WILSHIRE /VERMONT DATE: 6/21/89 BY: CAF

EXIT LANES AND CAPACITY PROVIDED

ELEMENT DIRECTION NUMBER x LANES x CAPACITY / LANE = PPM

Lower Platform to Upper Platform

Stairs	-up	2	3	35	=	210
	-down	0	0	40	=	0
Escalators	-up	4	2	35	=	280
	-down	0	0	40	=	0
Emer.	-up	4	4	35	=	560
Stairs	-down	0	0	40	=	0

30 Total 1050

Through Fare Barriers (South)

Turnstiles		4	1	25	=	100
Fare Gates		0	1	50	=	0
Service Gates		0	2	50	=	0
Emergency Gates		1	2	50	=	100

Total 200

Fare Barriers to Safe Area (South)

Stairs	-up	2	3	35	=	210
	-down	0	0	40	=	0
Escalators	-up	2	2	35	=	140
	-down	0	0	40	=	0
Emer.	-up	0	0	35	=	0
Stairs	-down	0	0	40	=	0

Total 350

Upper Platform to Concourse

Stairs	-up	2	3	35	=	210
	-down	0	0	40	=	0
Escalators	-up *	1	2	35	=	70
	-down	0	0	40	=	0
Emer.	-up	2	2	35	=	140
Stairs	-down	0	0	40	=	0

\* Discounted 12 Total 420

Through Fare Barriers (North)

Turnstiles		4	1	25	=	100
Fare Gates		0	1	50	=	0
Service Gates		0	2	50	=	0
Emergency Gates		1	2	50	=	100

Total 200

Fare Barriers to Safe Area (North)

Stairs	-up	2	3	35	=	210
	-down	0	0	40	=	0
Escalators	-up	2	2	35	=	140
	-down	0	0	40	=	0
Emer.	-up	0	0	35	=	0
Stairs	-down	0	0	40	=	0

Total 350

EMERGENCY EXIT WIDTH TEST

Minimum Platform Exit Width

(=Net Platform Area / (75sq.Ft. per Person x 50 Persons per Ft.))

Minimum Width Required : Up.= 27.15 Lower= 41.76

Width Provided: Up.= 22.00 Lower= 55.00 (NOT REQUIRES)

FARE BARRIER TEST

	Area	South	North
Capacity of Fare Gates and Turnstiles		100	100
Percent of Total Capacity		50.00	50.00

EMERGENCY EXIT CAPACITY TESTS

SHT 3 OF 4 WVDYCAF.UPE

STATION: WILSHIRE /VERMONT

DATE: 6/21/89

BY: CAF

Test 1

Evacuate Total Occupant Load from Platform(s) in 4 minutes or less.

W1(waiting time at platform exits) = Occupant Load / Exit Capacity

W1U = 1506 / 420 = 3.586 Minutes  
 W1L = 1980 / 1050 = 1.886 Minutes

Test 2

Evacuate Total Occupant Load from the most remote point on the platform to a point of safety in 6 minutes or less.

Walking Time for longest exit route

T = T1 + T2 + T3 + T4 + T5 + T6

	Feet	/ FPM	=	Minutes
T1 (on lower platform)	96	200	=	0.480
T2 (platform to platform)-up	30	50	=	0.600
T3 (on upper platform)	142	200	=	0.710
T4 (platform to concourse)-up	21	50	=	0.420
T5 (on concourse to safe area)	195	200	=	0.975
T6 (concourse to grade)-up	30	50	=	0.600

T = 3.785

Additional Waiting Time at Platform Exits

(W1L - T1) = 1.886 - 0.480 = 1.406 Minutes  
 (W1U - W1L) = 3.586 - 1.886 = 1.700 Minutes

Additional Waiting Time at Fare Barrier

Occupant Load at Concourse = Total Occupant Load (Lower Platform) - Lower Platform Emergency Stair 4 Minute Capacity + Total Occupant Load (Upper Platform) - Upper Platform Emergency Stair 4 Minute Capacity

1980 - (4 x 550) + 1506 - (4 x 140) = 1194 Patrons

W2 = Concourse Occupant Load / Gate Capacity

W2 = 946 / 400 = 2.365 Minutes  
 (W2 - W1) = 2.365 - 3.586 = 0.000 Minutes

Check for Unbalanced Flow

North: 473 / 200 = 2.365  
 South: 473 / 200 = 2.365

Additional Waiting Time at Concourse Exits

W3 = Concourse Occupant Load / Exit Capacity

W3 = 946 / 700 = 1.351 Minutes  
 (W3 - W1) = 1.351 - 3.586 = 0.000 Minutes

North: 473 / 350 = 1.351  
 South: 473 / 350 = 1.351

Total Exit Time [(T)+(W1-T1)+(W1U-W1L)+(W2-W1)+(W3-W1)]

TOTAL = 2.891 MINUTES (NOT > 6 MINUTES)



## EMERGENCY EXITING CALCULATIONS

SHT 4 OF 4 WVOYCAF.UPE

STATION: WILSHIRE /VERMONT DATE: 6/21/89 BY: CAF

SPECIAL CONSIDERATION - Lower Platform Occupants Use Emer. Stairs Only.  
Test 1

Evacuate Total Occupant Load from Platform in 4 minutes or less.

W1U = 1580 / 420 = 3.762 Minutes

## Test 2

Evacuate Total Occupant Load from the most remote point on the platform to a point of safety in 6 minutes or less.

Waiting Time at Platform Exits

(W1U-T3) = 3.762 - 0.710 = 3.052 Minutes

Waiting Time at Fare Barrier

Occupant Load at Concourse = Total Occupant Load (upper) - Upper Platform Emergency Stair 4 Minute Capacity

1580 - (4 x 140) = 1020 Patrons

North: W2U = 510 / 200 = 2.550 Minutes

South: W2U = 510 / 200 = 2.550 Minutes

J - W1U = 2.550 - 3.762 = 0.000 Minutes

Waiting Time at Concourse Exits

North: W3U = 510 / 350 = 1.457 Minutes

South: W3U = 510 / 350 = 1.457 Minutes

W3U - W1U = 1.457 - 3.762 = 0.000 Minutes

Total Exit Time (=T3+T4+T5+T6)+(W1U-T3)+(W2U-W1U)+(W3U-W1U)

Upper Platform

Total = 5.757 Minutes (Not &gt; 6 Minutes)

Total Exit Time (=Occupant Load / Emer. Stair Capacity)

Lower Platform

Total = 1980 / 560 = 3.536 Minutes

## EMERGENCY EXITING CALCULATIONS

SHT 1 OF 4

STATION: [REDACTED] DATE: [REDACTED] BY: CAF  
 NET PLATFORM AREA: UPPER = 9501 LOWER = 14524  
 OCCUPANT LOAD CALCULATION DESIGN YR

PEAK HOUR A.M. P.M.

LINK LOADS: INBOUND 11420 4852  
 OUTBOUND 5662 9766

ENTRAINING LOADS: INBOUND 2363 1999  
 OUTBOUND 806 1130

PEAK 15 MINUTE LOADS [=PEAK HR. LOAD x 1.15 / 4]

LINK LOADS: INBOUND 3284 1398  
 OUTBOUND 1628 2808

ENTRAINING LOADS: INBOUND 824 575  
 OUTBOUND 232 325

PEAK HEADWAY INBOUND 3.0 3.0  
 OUTBOUND 3.0 3.0

CALCULATED TRAIN LOAD [=PEAK 15 MIN. LINK LOAD / (NO. OF HEADWAYS)]  
 [NO. OF HEADWAYS = INTEGER(15 MIN. / HEADWAY)]

A.M. P.M.  
 INBOUND 657 280  
 OUTBOUND 326 362

(USE NOT(1320))

PEAK ENTRAINING LOAD [=PEAK 15 MIN. LOAD x (4 x HEADWAY / 15 MIN.)]

A.M. P.M.  
 INBOUND 460 460  
 OUTBOUND 136 260

TOTAL OCCUPANT LOAD [=CALCULATED TRAIN LOAD + PEAK ENTRAINING LOAD]

A.M. P.M.  
 UPPER PLATFORM 1506 1560  
 LOWER PLATFORM 1980 1760

DENSITY [=NET PLATFORM AREA / PEAK ENTRAINING LOAD]

(NOT < 4 SQ.FT. / PERSON) A.M. P.M.  
 UPR 51.08 26.54  
 LWR 22.16 31.79

STATION OCCUPANT LOAD [=LOWER + UPPER] 3486 3360

TOTAL OCCUPANT LOAD FOR CALCULATIONS 3486

EMERGENCY EXIT CAPACITY

SHT 2 OF 4 WVDYCAF.CE

STATION: WILSHIRE /VERMONT DATE: 6/21/89 BY: CAF

EXIT LANES AND CAPACITY PROVIDED

ELEMENT DIRECTION NUMBER x LANES x CAPACITY = FPM  
/ LANE

Lower Platform to Upper Platform

Stairs	-up	2	3	35	=	210
	-down	0	0	40	=	0
Escalators	-up	4	2	35	=	280
	-down	0	0	40	=	0
Emer.	-up	4	4	35	=	560
Stairs	-down	0	0	40	=	0

30 Total 1950

Through Fare Barriers (South)

Turnstiles		4	1	25	=	100
Fare Gates		0	1	50	=	0
Service Gates		0	2	50	=	0
Emergency Gates		1	2	50	=	100

Total 200

Fare Barriers to Safe Area (South)

Stairs	-up	2	3	35	=	210
	-down	0	0	40	=	0
Escalators	-up *	1	2	35	=	70
	-down	0	0	40	=	0
Emer.	-up	0	0	35	=	0
Stairs	-down	0	0	40	=	0

\* Discounted

Total 280

Upper Platform to Concourse

Stairs	-up	2	3	35	=	210
	-down	0	0	40	=	0
Escalators	-up	2	2	35	=	140
	-down	0	0	40	=	0
Emer.	-up	2	2	35	=	140
Stairs	-down	0	0	40	=	0

14 Total 490

Through Fare Barriers (North)

Turnstiles		4	1	25	=	100
Fare Gates		0	1	50	=	0
Service Gates		0	2	50	=	0
Emergency Gates		1	2	50	=	100

Total 200

Fare Barriers to Safe Area (North)

Stairs	-up	2	3	35	=	210
	-down	0	0	40	=	0
Escalators	-up	2	2	35	=	140
	-down	0	0	40	=	0
Emer.	-up	0	0	35	=	0
Stairs	-down	0	0	40	=	0

Total 350

EMERGENCY EXIT WIDTH TEST

Minimum Platform Exit Width

(=Net Platform Area / (750.Ft. per Person x 50 Persons per Ft.))  
 Minimum Width Required : Up.= 27.15 Lower= 41.78  
 Width Provided: Up.= 25.67 Lower= 55.00 (NOT REQUIRED)

FARE BARRIER TEST

	Arrav	South	North
Capacity of Fare Gates and Turnstiles		100	100
Percent of Total Capacity		30.00	50.00

EMERGENCY EXIT CAPACITY TESTS

SHT 3 OF 4 WILVERDY.CE

STATION: WILSHIRE /VERMONT

DATE: 6/21/89

BY: CAF

Test 1

Evacuate Total Occupant Load from Platform(s) in 4 minutes or less.

$W1$  (waiting time at platform exits) = Occupant Load / Exit Capacity

W1U = 1506 / 490 = 3.073 Minutes  
 W1L = 1980 / 1050 = 1.886 Minutes

Test 2

Evacuate Total Occupant Load from the most remote point on the platform to a point of safety in 6 minutes or less.

Walking Time for longest exit route

$T = T1 + T2 + T3 + T4 + T5 + T6$

	Feet	/ FPM	=	Minutes
T1 (on lower platform)	90	200		0.450
T2 (platform to platform)-up	30	50		0.600
T3 (on upper platform)	54	200		0.270
T4 (platform to concourse)-up	19	50		0.380
T5 (on concourse to safe area)	241	200		1.205
T6 (concourse to grade)-up	49.5	50		0.990

T = 3.715

Additional Waiting Time at Platform Exits

(W1L - T1) = 1.886 - 0.450 = 1.436 Minutes  
 (W1U - W1L) = 3.073 - 1.886 = 1.186 Minutes

Additional Waiting Time at Fare Barrier

Occupant Load at Concourse = Total Occupant Load (Lower Platform) - Lower Platform Emergency Stair 4 Minute Capacity + Total Occupant Load (Upper Platform) - Upper Platform Emergency Stair 4 Minute Capacity

1980 - (4 x 560) + 1506 - (4 x 140) = 946 Patrons

W2 = Concourse Occupant Load / Gate Capacity

W2 = 946 / 400 = 2.365 minutes  
 (W2 - W1) = 2.365 - 3.073 = 0.000 Minutes

Check for Unbalanced Flow

North: 473 / 200 = 2.365  
 South: 473 / 200 = 2.365

Additional Waiting Time at Concourse Exits

W3 = Concourse Occupant Load / Exit Capacity

W3 = 946 / 630 = 1.502 Minutes  
 (W3 - W1) = 1.502 - 3.073 = 0.000 Minutes

North: 473 / 350 = 1.351  
 South: 473 / 280 = 1.689

Total Exit Time (=T+(W1-T1)+(W1U-W1L)+(W2-W1)+(W3-W1))

TOTAL = 6.338 MINUTES (NOT > 6 MINUTES)

EMERGENCY EXITING CALCULATIONS

SHT 4 OF 4WILVERDY.CE

STATION: WILSHIRE /VERMONT DATE: 6/21/89 BY: CAF

SPECIAL CONSIDERATION - Lower Platform Occupants Use Emer. Stairs Only.  
 Test 1

Evacuate Total Occupant Load from Platform in 4 minutes or less.

$$W1U = 1580 / 490 = 3.224 \text{ Minutes}$$

Test 2

Evacuate Total Occupant Load from the most remote point on the platform to a point of safety in 6 minutes or less.

Waiting Time at Platform Exits

$$(W1U-T3) = 3.224 - 0.270 = 2.954 \text{ Minutes}$$

Waiting Time at Fare Barrier

Occupant Load at Concourse = Total Occupant Load (upper) - Upper Platform Emergency Stair 4 Minute Capacity

$$1580 - 14 \times 140 = 1020 \text{ Patrons}$$

$$\text{North: } W2U = 510 / 200 = 2.550 \text{ Minutes}$$

$$\text{South: } W2U = 510 / 200 = 2.550 \text{ Minutes}$$

$$1 - W1U = 2.550 - 3.224 = 0.000 \text{ Minutes}$$

Waiting Time at Concourse Exits

$$\text{North: } W3U = 510 / 350 = 1.457 \text{ Minutes}$$

$$\text{South: } W3U = 510 / 280 = 1.821 \text{ Minutes}$$

$$W3U - W1U = 1.821 - 3.224 = 0.000 \text{ Minutes}$$

Total Exit Time (=T3+T4+T5+T6)+(W1U-T3)+(W2U-W1U)+(W3U-W1U)

Upper Platform

$$\text{Total} = 5.619 \text{ Minutes (Not } > 6 \text{ Minutes)}$$

Total Exit Time (=Occupant Load / Emer. Stair Capacity)

Lower Platform

$$\text{Total} = 1980 / 560 = 3.536 \text{ Minutes}$$



**Rolf Jensen & Associates, Inc.**

FIRE PROTECTION ENGINEERS • BUILDING CODE CONSULTANTS

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MAR 15 1989

March 14, 1989

FEDERAL EXPRESS **ECC**  
(213) 612-7000

Mr. Malcolm Ingram  
Metro Rail Transit Consultants  
548 South Spring Street, 7th Floor  
Los Angeles, CA 90013

**RECEIVED BY, MRTC**

MAR 16 1989

**SAFETY & ASSURANCE**

PRELIMINARY EXIT  
CALCULATIONS

Malcolm:

Copies of preliminary exit calculations for Hollywood/Highland and Hollywood/Vine are enclosed. As we discussed, the Wilshire/Alvarado Station configuration was used.

Each station will have the following circulation elements:

Platform to Mezzanine  
2 3-unit emergency stairs  
2 3-unit normal stairs  
2 2-unit escalators

Mezzanine to Grade  
2 3-unit normal stairs  
2 2-unit escalators

These calculations will be updated as the preliminary plans become available. If you have a question, please call.

Sincerely,

David R. Fiedler, P.E.

DRF: jmp - H3275.01

Enclosure

## EMERGENCY EXITING CALCULATIONS

SHT 1 OF 3

STATION: Hollywood/Vine      DATE: 3/14/69      BY: DFF  
 NET PLATFORM AREA = 11632

OCCUPANT LOAD CALCULATION      DESIGN YR

PEAK HOUR		A.M.	P.M.
LINK LOADS:	INBOUND	5469	1527
	OUTBOUND	2255	4477
ENTRAINING LOADS:	INBOUND	718	797
	OUTBOUND	148	324

PEAK 15 MINUTE LOADS (=PEAK HR. LOAD x 1.15) 41

LINK LOADS:		A.M.	P.M.
LINK LOADS:	INBOUND	1573	449
	OUTBOUND	549	1238
ENTRAINING LOADS:	INBOUND	207	230
	OUTBOUND	43	94
HEADWAYS:	INBOUND	6.0	6.0
	OUTBOUND	6.0	6.0

CALCULATED TRAIN LOAD (=PEAK 15 MIN. LINK LOAD / (NO. OF HEADWAYS))  
 ----- (NO. OF HEADWAYS = INTEGER (15 MIN. / HEADWAY))

	A.M.	P.M.
INBOUND	797	229
OUTBOUND	325	644
TOTAL	1122	864
USE (NOT < 1320)	1320	1320

PEAK ENTRAINING LOAD (=PEAK 15 MIN. LOAD x (4 x HEADWAY / 15 MIN.))

	A.M.	P.M.
INBOUND	216	244
OUTBOUND	64	140
TOTAL	274	384

DENSITY (=NET PLATFORM AREA / PEAK ENTRAINING LOAD)

	A.M.	P.M.
(NOT < 4 SQ. FT. / PERSON)	31.10	24.03

TOTAL OCCUPANT LOAD (=CALCULATED TRAIN LOAD + PEAK ENTRAINING LOAD)

	A.M.	P.M.
TOTAL	1694	1204

Lesser of

AREA / 4 or Entraining Load + Train Load	1694	1204
TOTAL OCCUPANT LOAD FOR CALCULATIONS	1694	1204

Waiting time at platform exits = Occupant Load / Exit Capacity

W1 = 1804 / 550 = 3.221 Minutes

Test 2

Evacuate Total Occupant Load from the most remote point on the platform to a point of safety in 6 minutes or less.

Walking Time for longest exit route

T = T1 + T2 + T3 + T4 + T5

	Feet	FFM	=	Minutes
T1 (platform)	124	300		0.620
T2 (platform to concourse)-up	18.5	50		0.530
T3 (on concourse)	261	200		1.305
T4 (concourse to grade)-up	18.5	50		0.370
T5 (grade)	0	200		0.000
			T =	2.625

Additional Waiting Time at Platform Exits

(W1 - T) = 3.221 - 0.620 = 2.601 Minutes

Additional Waiting Time at Fare Barrier

Occupant Load at Concourse = Total Occupant Load - Emergency Stair 4  
Minute Capacity

1804 - 14 x 210 = 924 Patrons

CHECK FOR UNBALANCED FLOW

W2 = Concourse Occupant Load / Gate Capacity

W2 = 924 / 550 = 1.483 Minutes

(W2 - W1) = 1.510 - 3.221 = 0.000 Minutes

Arrow	East	West
Patrons	653	302
Capacity	450	200
Time	1.473	1.510

Additional Waiting Time at Concourse Exits

W3 = Concourse Occupant Load / Exit Capacity

W3 = 924 / 260 = 3.443 Minutes

(W3 - W1) = 3.443 - 3.221 = 0.221 Minutes

Total Exit Time = (T + (W1 - T)) + (W2 - W1) + (W3 - W1)

TOTAL = 5.448 MINUTES (NOT > 6 MINUTES)



EXIT LANES AND CAPACITY PROVIDED

ELEMENT	DIRECTION	NUMBER	x LANES	x CAPACITY / LANE	=	PPM			
Platform to Concourse									
Stairs	-up	2	3	35	=	210			
	-down	0	0	40	=	0			
Escalators	-up	2	2	35	=	140		discounted	
	-down	0	0	40	=	0			
Emer.	-up	2	3	35	=	210			
Stairs	-down	0	0	40	=	0			
			16	Total		560			
Through Fare Barriers									
Turnstiles		10	1	25	=	250	6	4	
Fare Gates		0	1	50	=	0	0	0	
Service Gates		1	2	50	=	100	1	0	
Emergency Gates		3	2	50	=	300	2	1	
				Total		550			
Fare Barriers to Safe Area									
Stairs	-up	2	3	35	=	210			
	-down	0	0	40	=	0			
Escalators	-up *	1	2	35	=	70		* - one escalator discounted	
	-down	0	0	40	=	0			
Emer.	-up	2	3	35	=	0			
Stairs	-down	0	0	40	=	0			
				Total		250			

FARE BARRIER TEST

	Array	East	West	PPM
Capacity of Fare Gates and Turnstiles	150	100	100	PPM
Percent of Total Capacity	37.5%	50.0%	50.0%	(NOT 50%)

EMERGENCY EXIT WIDTH TEST

Minimum Platform Exit Width

(Net Platform Area / (79 Sq. Ft. per Person x 50 Persons per Ft.))  
 Minimum Width Required = 33.24  
 Width Provided = 29.33 (NOT < REQUIRED)

EMERGENCY EXIT CAPACITY TESTS

SHT 2 OF 3

## EMERGENCY EXITING CALCULATIONS

SHT 1 OF 3

STATION: Hollywood/Highland      DATE: 3/14/89      BY: DRF  
 NET PLATFORM AREA = 11632

OCCUPANT LOAD CALCULATION      DESIGN /R

PEAK HOUR		A.M.	P.M.
LINK LOADS:	INBOUND	5674	1228
	OUTBOUND	1546	4115
ENTRAINING LOADS:	INBOUND	854	614
	OUTBOUND	263	761

PEAK 15 MINUTE LOADS (=PEAK HR. LOAD x 1.15 / 4)

LINK LOADS:		A.M.	P.M.
LINK LOADS:	INBOUND	1633	354
	OUTBOUND	445	1184
ENTRAINING LOADS:	INBOUND	255	177
	OUTBOUND	76	219
HEADWAYS:	INBOUND	6.0	6.0
	OUTBOUND	6.0	6.0

CALCULATED TRAIN LOAD [=PEAK 15 MIN. LINK LOAD x (NO. OF HEADWAYS)]  
 (NO. OF HEADWAYS = INTEGER(15 MIN. / HEADWAY))

	A.M.	P.M.
INBOUND	917	177
OUTBOUND	228	552
TOTAL	1145	729
USE (NOT > 1320)	1320	1320

PEAK ENTRAINING LOAD [=PEAK 15 MIN. LOAD x (4 x HEADWAY / 15 MIN.)]

	A.M.	P.M.
INBOUND	381	265
OUTBOUND	114	323
TOTAL	495	588

DENSITY [=NET PLATFORM AREA / PEAK ENTRAINING LOAD]

	A.M.	P.M.
(NOT < 4 SQ.FT. / PERSON)	23.50	19.61

TOTAL OCCUPANT LOAD [=CALCULATED TRAIN LOAD + PEAK ENTRAINING LOAD]

	A.M.	P.M.
TOTAL	1815	1913

Lesser of

AREA/4 or Entraining Load + Train Load	1315	1913
TOTAL OCCUPANT LOAD FOR CALCULATIONS	1315	1913



EMERGENCY EXIT CAPACITY TESTS

PAGE 3 OF 3

STATION: Hollywood/Highland      DATE: 3/14/89      BY: BRP  
 Test 1

Evacuate Total Occupant Load from Platform(s) in 4 minutes or less.

$W1$  (waiting time at platform exits) = Occupancy Load / Exit Capacity

$W1 = 1913 / 550 = 3.416$  Minutes

Test 2

Evacuate Total Occupant Load from the most remote point on the platform to a point of safety in 6 minutes or less.

Walking Time for longest exit route

$T = T1 + T2 + T3 + T4 + T5$

	Feet	/ FPM	=	Minutes
T1 (platform)	124	200	=	0.620
T2 (platform to concourse)-up	16.5	50	=	0.330
T3 (on concourse)	361	200	=	1.805
T4 (concourse to grade)-up	18.5	50	=	0.370
T5 (grade)	0	200	=	0.000
<b>T =</b>				<b>2.625</b>

Additional Waiting Time at Platform Exits

$(W1 - T1) = 3.416 - 0.620 = 2.796$  Minutes

Additional Waiting Time at Fare Barrier

Occupant Load at Concourse = Total Occupant Load - Emergency Stair 4 Minute Capacity

$1913 - 144 = 1769$  Patrons

CHECK FOR UNBALANCED FLOW

	East	West
Area		
Patrons	735	316
Capacity	450	200
Time	1.640	1.580

$W2$  = Concourse Occupant Load / Gate Capacity

$W2 = 1073 / 650 = 1.651$  Minutes

$(W2 - W1) = 1.651 - 3.416 = -1.765$  Minutes

Additional Waiting Time at Concourse Exits

$W3$  = Concourse Occupant Load / Exit Capacity

$W3 = 1073 / 280 = 3.832$  Minutes

$(W3 - W1) = 3.832 - 3.416 = 0.416$  Minutes

Total Exit Time  $T = (W1 - T1) + (W2 - W1) + (W3 - W1)$

TOTAL = 5.837 MINUTES (NOT > 6 MINUTES)

## EMERGENCY EXITING CALCULATIONS

SHT : OF 3

STATION: Hollywood/Vine

DATE: 3/14/89

BY: DRF

NET PLATFORM AREA = 11632

## OCCUPANT LOAD CALCULATION

DESIGN YR

PEAK HOUR		A.M.	P.M.
LINK LOADS:	INBOUND	5469	1527
	OUTBOUND	2255	4477
ENTRAINING LOADS:	INBOUND	718	797
	OUTBOUND	148	324
PEAK 15 MINUTE LOADS (=PEAK HR. LOAD x 1.15 / 4)			
LINK LOADS:	INBOUND	573	440
	OUTBOUND	547	1788
ENTRAINING LOADS:	INBOUND	207	270
	OUTBOUND	43	94
HEADWAYS:	INBOUND	6.0	6.0
	OUTBOUND	6.0	6.0

CALCULATED TRAIN LOAD (=PEAK 15 MIN. LINK LOAD / (NO. OF HEADWAYS))

----- (NO. OF HEADWAYS = INTEGER (15 MIN. / HEADWAYS))

	A.M.	P.M.
INBOUND	757	320
OUTBOUND	325	644
TOTAL	1082	964
USE (NOT 100%)	1000	1000

PEAK ENTRAINING LOAD (=PEAK 15 MIN. LOAD x (4 x HEADWAY / 15 MIN.))

	A.M.	P.M.
INBOUND	718	344
OUTBOUND	84	140
TOTAL	774	484

DENSITY (=NET PLATFORM AREA / PEAK ENTRAINING LOAD)

	A.M.	P.M.
(NOT < 4 SQ.FT. / PERSON)	31.10	24.00

TOTAL OCCUPANT LOAD (=CALCULATED TRAIN LOAD + PEAK ENTRAINING LOAD)

	A.M.	P.M.
TOTAL	1574	1804

Lesser of

AREA / 4 or Entraining Load + Train Load	1574	1804
TOTAL OCCUPANT LOAD FOR CALCULATIONS	904	

Waiting time at platform exits = occupancy load / exit capacity

W1 = 1804 / 490 = 3.682 Minutes

Test 2

Evacuate Total Occupant Load from the most remote point on the platform to a point of safety in 6 minutes or less.

Walking Time for longest exit route

T = T1 + T2 + T3 + T4 + T5

	Feet	/ FPM	=	Minutes
T1 (platform)	124	200		0.620
T2 (platform to concourse)-up	16.5	50		0.330
T3 (on concourse)	261	200		1.305
T4 (concourse to grade)-up	18.5	50		0.370
T5 (grade)	0	200		0.000
T =				2.625

Additional Waiting Time at Platform Exits

(W1 - T1) = 3.682 - 0.620 = 3.062 Minutes

Additional Waiting Time at Fare Barrier

Occupant Load at Concourse = Total Occupant Load - Emergency Stair 4 Minute Capacity

1804 - (4 x 210) = 984 Patrons

W2 = Concourse Occupant Load / Gate Capacity

W2 = 984 / 650 = 1.514 Minutes

(W2 - W1) = 1.514 - 3.682 = -2.168 Minutes

CHECK FOR UNBALANCED FLOW

Array	East	West
Patrons	620	345
Capacity	450	200
Time	1.378	1.725

Additional Waiting Time at Concourse Exits

W3 = Concourse Occupant Load / Exit Capacity

W3 = 984 / 350 = 2.811 Minutes

(W3 - W1) = 2.811 - 3.682 = -0.871 Minutes

Total Exit Time = (T + (W1 - T1) + (W2 - W1) + (W3 - W1))

TOTAL = 5.687 MINUTES (NOT > 6 MINUTES)

## EMERGENCY EXITING CALCULATIONS

SHT 1 OF 3

STATION: Sunset / Vermont      DATE: 3/17/89      BY: DRF  
 NET PLATFORM AREA = 11832

OCCUPANT LOAD CALCULATION      DESIGN YR

PEAK HOUR		A.M.	P.M.
LINK LOADS:	INBOUND	6316	2112
	OUTBOUND	2090	4906
ENTRAINING LOADS:	INBOUND	719	396
	OUTBOUND	279	287

PEAK 15 MINUTE LOADS (=PEAK HR. LOAD x 1.15 / 4)

LINK LOADS:		A.M.	P.M.
LINK LOADS:	INBOUND	1816	608
	OUTBOUND	601	1411
ENTRAINING LOADS:	INBOUND	207	114
	OUTBOUND	81	83
HEADWAYS:	INBOUND	6.0	6.0
	OUTBOUND	6.0	6.0

CALCULATED TRAIN LOAD (=PEAK 15 MIN. LINK LOAD / (NO. OF HEADWAYS))

		A.M.	P.M.
LINK LOADS:	INBOUND	306	304
	OUTBOUND	201	706
TOTAL		1209	1010
USE (NOT < 1020)		1020	1020

PEAK ENTRAINING LOAD (=PEAK 15 MIN. LOAD x (4 x HEADWAY / 15 MIN.))

		A.M.	P.M.
LINK LOADS:	INBOUND	310	171
	OUTBOUND	121	104
TOTAL		431	275

DENSITY (=NET PLATFORM AREA / PEAK ENTRAINING LOAD)

	A.M.	P.M.
(NOT < 4 SQ.FT. / PERSON)	26.99	39.43

TOTAL OCCUPANT LOAD (=CALCULATED TRAIN LOAD + PEAK ENTRAINING LOAD)

	A.M.	P.M.
TOTAL	1751	1615
Lesser of		
AREA/A or Entraining Load + Train Load	1751	1615
TOTAL OCCUPANT LOAD PER CALCULATIONS	1751	

EMERGENCY EXIT CAPACITY

SHT 2 OF 3

STATION: Sunset / Vermont

DATE: 3/17/89

SY: ORF

EXIT LANES AND CAPACITY PROVIDED

ELEMENT DIRECTION NUMBER x LANES x CAPACITY = PFM  
/ LANE

Platform to Concourse

Stairs -up 2 3 35 = 210  
 -down 0 0 40 = 0  
 Escalators-up 1 2 35 = 70 \* - one escalator discounted  
 -down 0 0 40 = 0  
 Emer. -up 2 3 35 = 210  
 Stairs -down 0 0 40 = 0

14 Total 495

Through Fare Barriers

Turnstiles 10 1 25 = 250  
 Fare Gates 0 1 50 = 0  
 Service Gates 1 2 50 = 100  
 Emergency Gates 3 2 50 = 300

East West

6 4  
 0 0  
 1 0  
 2 1

Total 650

Fare Barriers to Safe Areas

Stairs -up 2 3 35 = 210  
 -down 0 0 40 = 0  
 Escalators-up 2 2 35 = 140  
 -down 0 0 40 = 0  
 Emer. -up 0 0 35 = 0  
 Stairs -down 0 0 40 = 0

Total 390

FARE BARRIER TEST

	Array	East	West	PFM
Capacity of Fare Gates and Turnstiles		150	100	
Percent of Total Capacity		33.33	50.00	(NOT 50%)

EMERGENCY EXIT WIDTH TEST

Minimum Platform Exit Width

(Level Platform Area / (75Sq.Ft. per Person x 50 Persons per Ft.))  
 Minimum Width Required = 33.24  
 Width Provided = 25.87 (NOT REQUIRED)



EMERGENCY EXIT CAPACITY TESTS

PAGE 3 OF 3

STATION: Sunset / Vermont DATE: 3/17/89 BY: JRF  
 Test 1

Evacuate Total Occupant Load from Platform(s) in 4 minutes or less.

W1 (waiting time at platform exits) = Occupancy Load / Exit Capacity

W1 = 1751 / 490 = 3.573 Minutes

Test 2

Evacuate Total Occupant Load from the most remote point on the platform to a point of safety in 6 minutes or less.

Walking Time for longest exit route

T = T1 + T2 + T3 + T4 + T5

	Feet	/ FPM	=	Minutes
T1 (platform)	121	200	=	0.620
T2 (platform to concourse)-up	16.5	50	=	0.330
T3 (on concourse)	261	200	=	1.305
T4 (concourse to grade)-up	18.5	50	=	0.370
T5 (grade)	0	200	=	0.000
T =				2.625

Additional Waiting Time at Platform Exits

(W1 - T1) = 3.573 - 0.620 = 2.953 Minutes

Additional Waiting Time at Fare Barrier

Occupant Load at Concourse = Total Occupant Load - Emergency Stair 4 Minute Capacity

1751 - 4 x 210 = 911 Patrons

CHECK FOR UNBALANCED FLOW

	East	West
Arrival Patrons	586	326
Capacity	450	200
Time	1.307	1.630

W2 = Concourse Occupant Load / Gate Capacity

W2 = 911 / 650 = 1.402 Minutes

(W2 - W1) = 1.630 - 3.573 = 0.000 Minutes

Additional Waiting Time at Concourse Exits

W3 = Concourse Occupant Load / Exit Capacity

W3 = 911 / 350 = 2.603 Minutes

(W3 - W1) = 2.603 - 3.573 = 0.000 Minutes

Total Exit Time (=T+(W1-T1)+(W2-W1)+(W3-W1))

TOTAL = 3.573 MINUTES (NOT 6 MINUTES)

## EMERGENCY EXITING CALCULATIONS

SHT 1 OF 3

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STATION: Vermont / Beverly      DATE: 3/16/89      BY: DRF  
 NET PLATFORM AREA = 11632

OCCUPANT LOAD CALCULATION      DESIGN YR

PEAK HOUR		A.M.	P.M.
LINK LOADS:	INBOUND	8057	2698
	OUTBOUND	2217	5746
ENTRAINING LOADS:	INBOUND	954	321
	OUTBOUND	300	401

PEAK 15 MINUTE LOADS (=PEAK HR. LOAD x 1.15 / 4)

LINK LOADS:		A.M.	P.M.
LINK LOADS:	INBOUND	3317	775
	OUTBOUND	638	1652
ENTRAINING LOADS:	INBOUND	275	93
	OUTBOUND	87	116
HEADWAYS:	INBOUND	6.0	5.0
	OUTBOUND	6.0	6.0

CALCULATED TRAIN LOAD (=PEAK 15 MIN. LINK LOAD / (NO. OF HEADWAYS))  
 ----- (NO. OF HEADWAYS = INTEGER (15 MIN. / HEADWAY))

	A.M.	P.M.
INBOUND	1159	788
OUTBOUND	319	806
TOTAL	1478	1214
USE (NOT < 1320)	1478	1020

PEAK ENTRAINING LOAD (=PEAK 15 MIN. LOAD x (4 x HEADWAY / 15 MIN.))

	A.M.	P.M.
INBOUND	411	109
OUTBOUND	130	173
TOTAL	541	312

DENSITY (=NET PLATFORM AREA / PEAK ENTRAINING LOAD)

	A.M.	P.M.
(NOT < 4 SQ.FT. / PERSON)	21.50	37.28

TOTAL OCCUPANT LOAD (=CALCULATED TRAIN LOAD + PEAK ENTRAINING LOAD)

	A.M.	P.M.
TOTAL	2019	1632

Lesser of

AREA/4 or Entraining Load + Train Load	2019	1632
TOTAL OCCUPANT LOAD FOR CALCULATIONS	2019	



EMERGENCY EXIT CAPACITY TESTS

SHT 3 OF 3

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STATION: Vermont / Beverly      DATE: 3/16/89      BY: DRF  
Test 1

Evacuate Total Occupant Load from Platform(s) in 4 minutes or less.

$W1$  (waiting time at platform exits) = Occupancy Load / Exit Capacity

$W1 = 2019 / 630 = 3.205$  Minutes

Test 2

Evacuate Total Occupant Load from the most remote point on the platform to a point of safety in 6 minutes or less.

Walking Time for longest exit route

$T = T1 + T2 + T3 + T4 + T5$

	Feet	f FPM	=	Minutes
T1 (platform)	124	200		0.620
T2 (platform to concourse)-up	16.5	50		0.330
T3 (on concourse)	261	200		1.305
T4 (concourse to grade)-up	18.5	50		0.370
T5 (grade)	0	200		0.000
$T =$				2.625

Additional Waiting Time at Platform Exits

$(W1 - T1) = 3.205 - 0.620 = 2.585$  Minutes

Additional Waiting Time at Fare Barrier

Occupant Load at Concourse = Total Occupant Load - Emergency Stair 4  
 Minute Capacity

$2019 - 14 \times 260 = 899$  Patrons

$W2 =$  Concourse Occupant Load / Gate Capacity

$W2 = 899 / 650 = 1.383$  Minutes

$(W2 - W1) = 1.405 - 3.205 = 0.000$  Minutes

CHECK FOR UNBALANCED FLOW

	East	West
Arrival Patrons	67	261
Capacity	450	200
Time	1.376	1.405

Additional Waiting Time at Concourse Exits

$W3 =$  Concourse Occupant Load / Exit Capacity

$W3 = 899 / 280 = 3.211$  Minutes

$(W3 - W1) = 3.211 - 3.205 = 0.006$  Minutes

Total Exit Time  $(T + (W1 - T1) + (W2 - W1) + (W3 - W1))$

TOTAL = 3.316 MINUTES (NOT > 6 MINUTES)

## EMERGENCY EXITING CALCULATIONS

SHT 1 OF 3

STATION: Vermont / Beverly      DATE: 3/14/89      BY: DRF  
 NET PLATFORM AREA = 11632

## OCCUPANT LOAD CALCULATION

DESIGN YR

PEAK HOUR		A.M.	P.M.
LINK LOADS:	INBOUND	8057	2679
	OUTBOUND	3217	5746
ENTRAINING LOADS:	INBOUND	954	321
	OUTBOUND	300	401

## PEAK 15 MINUTE LOADS (=PEAK HR. LOAD x 0.15 / 4)

LINK LOADS:		A.M.	P.M.
LINK LOADS:	INBOUND	2317	776
	OUTBOUND	638	1652
ENTRAINING LOADS:	INBOUND	275	93
	OUTBOUND	87	116
HEADWAYS:	INBOUND	6.0	6.0
	OUTBOUND	6.0	6.0

## CALCULATED TRAIN LOAD (=PEAK 15 MIN. LINK LOAD / (NO. OF HEADWAYS))

NO. OF HEADWAYS = (INTEGER) (15 MIN. / HEADWAY)

	A.M.	P.M.
INBOUND	1159	339
OUTBOUND	319	825
TOTAL	1478	1214
USE (NOT < 1000)	1478	1320

## PEAK ENTRAINING LOAD (=PEAK 15 MIN. LOAD x 4 x HEADWAY / 15 MIN.)

	A.M.	P.M.
INBOUND	411	179
OUTBOUND	179	173
TOTAL	541	312

## DENSITY (=NET PLATFORM AREA / PEAK ENTRAINING LOAD)

	A.M.	P.M.
(NOT < 4 SQ.FT. / PERSON)	21.50	37.28

## TOTAL OCCUPANT LOAD (=CALCULATED TRAIN LOAD + PEAK ENTRAINING LOAD)

	A.M.	P.M.
TOTAL	2019	1632

Lesser of

AREA/4 or Entraining Load + Train Load	2019	1632
TOTAL OCCUPANT LOAD FOR CALCULATIONS	2019	

EMERGENCY EXIT CAPACITY

SHT 2 OF 3

STATION: Vermont / Beverly      DATE: 3/14/89      BY: DRF

EXIT LANES AND CAPACITY PROVIDED

ELEMENT	DIRECTION	NUMBER	x LANES	x CAPACITY / LANE	=	PPM			
Platform to Concourse									
Stairs	-up	2	3	35	=	210			
	-down	0	0	40	=	0			
Escalators	-up *	1	2	35	=	70	*	- one escalator discounted	
	-down	0	0	40	=	0			
Emer.	-up	2	3	35	=	210			
Stairs	-down	0	0	40	=	0			
			14	Total		490			

Through Fare Barriers						East	West	
Turnstiles		10	1	25	=	250	6	4
Fare Gates		0	1	50	=	0	0	0
Service Gates		1	2	50	=	100	1	0
Emergency Gates		3	2	50	=	300	2	1
				Total		650		

Fare Barriers to Safe Area									
Stairs	-up	2	3	35	=	210			
	-down	0	0	40	=	0			
Escalators	-up	2	2	35	=	140			
	-down	0	0	40	=	0			
Emer.	-up	0	3	35	=	0			
Stairs	-down	0	0	40	=	0			
				Total		350			

FARE BARRIER TEST

	Array	East	West	
Capacity of Fare Gates and Turnstiles		150	100	PPM
Percent of Total Capacity		33.33	50.00	(NOT GOOD)

EMERGENCY EXIT WIDTH TEST

Minimum Platform Exit Width

[ = Net Platform Area / (7Sq.Ft. per Person x 50 Persons per Ft.) ]  
 Minimum Width Required = 33.24  
 Width Provided = 25.67 (NOT REQUIRED)

EMERGENCY EXIT CAPACITY TESTS

PAGE 3 OF 3

STATION: Vermont / Beverly      DATE: 3/14/89      BY: DRF  
 Test 1

Evacuate Total Occupant Load from Platform(s) in 4 minutes or less.

W1 (waiting time at platform exits) = Occupancy Load / Exit Capacity

W1 = 2019 / 490 = 4.120 Minutes

Test 2

Evacuate Total Occupant Load from the most remote point on the platform to a point of safety in 6 minutes or less.

Walking Time for longest exit route

T = T1 + T2 + T3 + T4 + T5

	Feet	/ FPM	=	Minutes
T1 (platform)	124	200		0.620
T2 (platform to concourse)-up	16.5	50		0.330
T3 (on concourse)	261	200		1.305
T4 (concourse to grade)-up	16.5	50		0.330
T5 (grade)	0	200		0.000
T =				2.625

Additional Waiting Time at Platform Exits

(W1 - T1) = 4.120 - 0.620 = 3.500 Minutes

Additional Waiting Time at Fare Barrier

Occupant Load at Concourse = Total Occupant Load - Emergency Stair 4  
 Minute Capacity

2019 - (4 x 210) = 1179 Patrons

W2 = Concourse Occupant Load / Gate Capacity

W2 = 1179 / 650 = 1.814 Minutes  
 (W2 - W1) = 2.110 - 4.120 = 0.000 Minutes

CHECK FOR UNBALANCED FLOW

	East	West
Arrival Patrons	758	422
Capacity	450	300
Time	1.684	2.110

Additional Waiting Time at Concourse Exits

W3 = Concourse Occupant Load / Exit Capacity

W3 = 1179 / 350 = 3.369 Minutes  
 (W3 - W1) = 3.369 - 4.120 = 0.000 Minutes

Total Exit Time [(T - (W1 - T1)) + (W2 - W1) + (W3 - W1)]

TOTAL = 6.125 MINUTES (NOT 6 MINUTES)

## EMERGENCY EXITING CALCULATIONS

SHT 1 OF 3

STATION: Vermont / Beverly DATE: 3/16/89 BY: DRF

NET PLATFORM AREA = 11632

## OCCUPANT LOAD CALCULATION

DESIGN YR

PEAK HOUR		A.M.	P.M.
LINK LOADS:	INBOUND	8057	2678
	OUTBOUND	2217	3746
ENTRAINING LOADS:	INBOUND	954	321
	OUTBOUND	300	401

PEAK 15 MINUTE LOADS (=PEAK HR. LOAD x 1.15 / 4)

LINK LOADS:		A.M.	P.M.
LINK LOADS:	INBOUND	2317	776
	OUTBOUND	639	1652
ENTRAINING LOADS:	INBOUND	275	93
	OUTBOUND	87	116
HEADWAYS:	INBOUND	6.0	6.0
	OUTBOUND	6.0	6.0

CALCULATED TRAIN LOAD (=PEAK 15 MIN. LINK LOAD / (NO. OF HEADWAYS))  
 ----- (NO. OF HEADWAYS = INTEGER(15 MIN. / HEADWAY))

	A.M.	P.M.
INBOUND	1159	368
OUTBOUND	719	826
TOTAL	1473	1214
USE (NOT > 1000)	1473	1000

PEAK ENTRAINING LOAD (=PEAK 15 MIN. LOAD x (4 x HEADWAY / 15 MIN.))

	A.M.	P.M.
INBOUND	411	179
OUTBOUND	300	173
TOTAL	541	311

DENSITY (=NET PLATFORM AREA / PEAK ENTRAINING LOAD)

	A.M.	P.M.
(NOT < 4 SQ. FT. / PERSON)	21.59	37.28

TOTAL OCCUPANT LOAD (=CALCULATED TRAIN LOAD + PEAK ENTRAINING LOAD)

	A.M.	P.M.
TOTAL	2019	1601

Lesser of

AREA/4 or Entaining Load + Train Load	2019	1632
TOTAL OCCUPANT LOAD FOR CALCULATIONS	2019	





EMERGENCY EXIT CAPACITY TESTS

SHT 3 OF 3

STATION: Vermont / Beverly

DATE: 3/16/99

BY: DRF

Test 1

Evacuate Total Occupant Load from Platform(s) in 4 minutes or less.

W1 (waiting time at platform exits) = Occupancy Load / Exit Capacity

W1 = 2019 / 550 = 3.605 Minutes

Test 2

Evacuate Total Occupant Load from the most remote point on the platform to a point of safety in 6 minutes or less.

Walking Time for longest exit route

T = T1 + T2 + T3 + T4 + T5

	Feet	/ FPM	=	Minutes
T1 (platform)	124	200	=	0.620
T2 (platform to concourse)-up	16.5	50	=	0.330
T3 (on concourse)	251	200	=	1.305
T4 (concourse to grade)-up	16.5	50	=	0.330
T5 (grade)	0	200	=	0.000
T =				2.625

Additional waiting Time at Platform Exits

W1 + T = 3.605 + 0.620 = 4.225 Minutes

Additional Waiting Time at Fare Barrier

Occupant Load at Concourse = Total Occupant Load - Emergency Stair + Minute Capacity

1179 - 110 = 1069 Patrons

LINE	CONCOURSE FLOW
Arrow	East West
Patrons	311 267
Capacity	450 300
Time	1.580 1.345

W2 = Concourse Occupant Load / Gate Capacity

W2 = 1179 / 650 = 1.814 Minutes

W2 + W1 = 1.814 + 3.605 = 5.419 Minutes

Additional Waiting Time at Concourse Exits

W3 = Concourse Occupant Load / Exit Capacity

W3 = 1179 / 280 = 4.211 Minutes

W3 + W2 = 4.211 + 3.605 = 7.816 Minutes

Total Exit Time = (W1+W2+T1)+(W2+W1)+(W3+W2)

TOTAL = 5.419 MINUTES (WGT = 6 MINUTES)

## EMERGENCY EXITING CALCULATIONS

SHT 1 OF 3

STATION: Hollywood/Western      DATE: 3/14/89      BY: JAF  
 NET PLATFORM AREA = 11632

OCCUPANT LOAD CALCULATION      DESIGN YR

PEAK HOUR		A.M.	P.M.
LINK LOADS:	INBOUND	5786	2167
	OUTBOUND	2612	4721
ENTRAINING LOADS:	INBOUND	362	348
	OUTBOUND	503	376

PEAK 15 MINUTE LOADS (=PEAK HR. LOAD x 1.15 / 4)

LINK LOADS:		A.M.	P.M.
INBOUND	1664	624	
OUTBOUND	579	1353	
ENTRAINING LOADS:	INBOUND	248	101
	OUTBOUND	145	114
HEADWAYS:	INBOUND	6.0	6.0
	OUTBOUND	6.0	6.0

CALCULATED TRAIN LOAD (=PEAK 15 MIN. LINK LOAD / (NO. OF HEADWAYS))

----- (NO. OF HEADWAYS = INTEGER(15 MIN. / HEADWAY))		A.M.	P.M.
INBOUND		332	312
OUTBOUND		290	579
TOTAL		1122	891
USE (NOT > 1320)		1320	1320

PEAK ENTRAINING LOAD (=PEAK 15 MIN. LOAD x 14 x HEADWAY / 15 (MIN.))

-----		A.M.	P.M.
INBOUND		371	181
OUTBOUND		217	171
TOTAL		588	352

DENSITY (=NET PLATFORM AREA / PEAK ENTRAINING LOAD)

-----		A.M.	P.M.
(NOT < 4 SQ.FT. / PERSON)		19.78	26.12

TOTAL OCCUPANT LOAD (=CALCULATED TRAIN LOAD + PEAK ENTRAINING LOAD)

-----		A.M.	P.M.
TOTAL		1908	1642

Lesser of:

AREA/A or Entraining Load + Train Load	1908	1642
TOTAL OCCUPANT LOAD FOR CALCULATIONS	1908	

EMERGENCY EXIT CAPACITY

SHT 2 OF 3

STATION: Hollywood/Western DATE: 3/14/89 BY: DRF

EXIT LANES AND CAPACITY PROVIDED

ELEMENT DIRECTION NUMBER x LANES x CAPACITY = PPM  
/ LANE

Platform to Concourse

Stairs	-up	2	3	35	=	210	
	-down	0	0	40	=	0	
Escalators	-up	2	2	35	=	140	scouted
	-down	0	0	40	=	0	
Emer.	-up	2	3	35	=	210	
Stairs	-down	0	0	40	=	0	

16 Total 560

Through Fare Barriers

Turnstiles	10	1	25	=	250	6	4
Fare Gates	0	1	50	=	0	0	0
Service Gates	1	2	50	=	100	1	0
Emergency Gates	0	2	50	=	300	2	1

Total 650

Fare Barriers to Safe Area

Stairs	-up	2	3	35	=	210	
	-down	0	0	40	=	0	
Escalators	-up *	1	2	35	=	70	* - one escalator discounted
	-down	0	0	40	=	0	
Emer.	-up	0	0	35	=	0	
Stairs	-down	0	0	40	=	0	

Total 280

FARE BARRIER TEST

	Array	East	West	
Capacity of Fare Gates and Turnstiles		150	100	PPM
Percent of Total Capacity		33.33	50.00	(NOT >50%)

EMERGENCY EXIT WIDTH TEST

Minimum Platform Exit Width

(=Net Platform Area / (75Sq. Ft. per Person x 50 Persons per Ft.))  
 Minimum Width Required = 37.34  
 Width Provided = 29.37 (NOT < REQUIRED)

EMERGENCY EXIT CAPACITY TESTS

SHT 3 OF 3

STATION: Hollywood/Western

DATE: 3/14/89

By: DRF

Test 1

Evacuate Total Occupant Load from Platform(s) in 4 minutes or less.

W1 (waiting time at platform exits) = Occupancy Load / Exit Capacity

$$W1 = 1908 / 560 = 3.407 \text{ Minutes}$$

Test 2

Evacuate Total Occupant Load from the most remote point on the platform to a point of safety in 6 minutes or less.

Walking Time for longest exit route

$$T = T1 + T2 + T3 + T4 + T5$$

	Feet	/ FPM	=	Minutes
T1 (platform)	114	200		0.570
T2 (platform to concourse)-up	15.5	50		0.310
T3 (on concourse)	261	200		1.305
T4 (concourse to grade)-up	19.5	50		0.390
T5 (grade)	0	200		0.000
T =				2.625

Additional Waiting Time at Platform Exits

$$W1 - T1 = 3.407 - 0.570 = 2.737 \text{ Minutes}$$

Additional Waiting Time at Fare Barrier

Occupant Load at Concourse = Total Occupant Load - Emergency Stair 4 Minute Capacity

$$1908 - (4 \times 210) = 1068 \text{ Patrons}$$

W2 = Concourse Occupant Load / Gate Capacity

$$W2 = 1068 / 650 = 1.643 \text{ Minutes}$$

$$(W2 - W1) = 1.643 - 3.407 = 0.000 \text{ Minutes}$$

CHECK FOR UNBALANCED FLOW

	East	West
Arrival Patrons	935	974
Capacity	450	500
Time	1.333	1.970

Additional Waiting Time at Concourse Exits

W3 = Concourse Occupant Load / Exit Capacity

$$W3 = 1068 / 280 = 3.814 \text{ Minutes}$$

$$(W3 - W1) = 3.814 - 3.407 = 0.407 \text{ Minutes}$$

Total Exit Time = (T + W1 - T1) + (W2 - W1) + (W3 - W1)

$$\text{TOTAL} = 5.319 \text{ MINUTES (NOT > 6 MINUTES)}$$

## EMERGENCY EXITING CALCULATIONS

SHT : OF 3

STATION: Hollywood/Western      DATE: 3/14/89      BY: DRF  
 NET PLATFORM AREA = 11632

## OCCUPANT LOAD CALCULATION      DESIGN YR

PEAK HOUR		A.M.	P.M.
LINK LOADS:	INBOUND	5798	2187
	OUTBOUND	2612	4721
ENTRAINING LOADS:	INBOUND	362	348
	OUTBOUND	503	396

## PEAK 15 MINUTE LOADS (=PEAK HR. LOAD x 1.15 / 4)

LINK LOADS:		A.M.	P.M.
LINK LOADS:	INBOUND	1664	924
	OUTBOUND	377	1358
ENTRAINING LOADS:	INBOUND	248	101
	OUTBOUND	145	114
HEADWAYS:	INBOUND	8.0	8.0
	OUTBOUND	8.0	8.0

 CALCULATED TRAIN LOAD (=PEAK 15 MIN. LINK LOAD / (NO. OF HEADWAYS))  
 ----- (NO. OF HEADWAYS = INTEGER (15 MIN. / HEADWAY))

	A.M.	P.M.
INBOUND	332	312
OUTBOUND	290	679
TOTAL	1122	991
USE (NOT < 1320)	1320	1320

## PEAK ENTRAINING LOAD (=PEAK 15 MIN. LOAD x 14 x HEADWAY / 15 MIN.)

	A.M.	P.M.
INBOUND	271	151
OUTBOUND	217	171
TOTAL	588	722

## DENSITY (=NET PLATFORM AREA / PEAK ENTRAINING LOAD)

	A.M.	P.M.
(NOT < 4 SQ.FT. / PERSON)	19.79	38.12

## TOTAL OCCUPANT LOAD (=CALCULATED TRAIN LOAD + PEAK ENTRAINING LOAD)

	A.M.	P.M.
TOTAL	1908	1642

Lesser of

AREA/4 or Entraining Load + Train Load	1908	1642
TOTAL OCCUPANT LOAD FOR CALCULATIONS	1908	

EMERGENCY EXIT CAPACITY

SHT 2 OF 3

STATION: Hollywood/Western

DATE: 3/14/89

By: ERF

EXIT LANES AND CAPACITY PROVIDED

ELEMENT	DIRECTION	NUMBER	x LANES	x CAPACITY / LANE	=	PPM		
<u>Platform to Concourse</u>								
Stairs	-up	2	3	35	=	210		
	-down	0	0	40	=	0		
Escalators	-up *	1	2	35	=	70	* - one escalator discounted	
	-down	0	0	40	=	0		
Emer.	-up	2	3	35	=	210		
Stairs	-down	0	0	40	=	0		
			14	Total		490		
<u>Through Fare Barriers</u>								
							East	West
Turnstiles		10	1	25	=	250	6	4
Fare Gates		0	1	50	=	0	0	0
Service Gates		1	2	50	=	100	1	0
Emergency Gates		3	2	50	=	300	2	1
				Total		650		
<u>Fare Barriers to Safe Area</u>								
Stairs	-up	2	3	35	=	210		
	-down	0	0	40	=	0		
Escalators	-up	2	2	35	=	140		
	-down	0	0	40	=	0		
Emer.	-up	0	0	35	=	0		
Stairs	-down	0	0	40	=	0		
				Total		350		

FARE BARRIER TEST

	Array	East	West	
Capacity of Fare Gates and Turnstiles		150	100	PPM
Percent of Total Capacity		33.33	50.00	(NOT 100%)

EMERGENCY EXIT WIDTH TEST

Minimum Platform Exit Width

(=Net Platform Area / (75sq. Ft. per Person x 50 Persons per Ft.))	
Minimum Width Required =	33.24
Width Provided =	25.87 (NOT SUFFICIENT)

EMERGENCY EXIT CAPACITY TESTS

SHT 3 OF 3

STATION: Hollywood/Western      DATE: 3/14/89      BY: DRF  
 -----  
 Test 1  
 -----

Evacuate Total Occupant Load from Platform(s) in 4 minutes or less.

W1 (waiting time at platform exits) = Occupancy Load / Exit Capacity

W1 = 1908 / 490 = 3.894 Minutes

Test 2  
 -----

Evacuate Total Occupant Load from the most remote point on the platform to a point of safety in 6 minutes or less.

Walking Time for longest exit route  
 -----

T = T1 + T2 + T3 + T4 + T5

	Feet	/ FPM	=	Minutes
T1 (platform)	124	200		0.620
T2 (platform to concourse)-up	16.5	50		0.330
T3 (on concourse)	261	200		1.305
T4 (concourse to grade)-up	19.5	50		0.370
T5 (grade)	0	200		0.000
			T =	2.625

Additional Waiting Time at Platform Exits  
 -----

(W1 - T1) = 3.894 - 0.620 = 3.274 Minutes

Additional Waiting Time at Fare Barrier  
 -----

Occupant Load at Concourse = Total Occupant Load - Emergency Stair 4  
 Minute Capacity

1908 - (4 x 210) = 1068 Patrons

W2 = Concourse Occupant Load / Gate Capacity

W2 = 1068 / 650 = 1.643 Minutes

(W2 - W1) = 1.910 - 3.894 = 0.000 Minutes

CHECK FOR UNBALANCED FLOW

Array	East	West
Patrons	687	382
Capacity	450	200
Time	1.527	1.710

Additional Waiting Time at Concourse Exits  
 -----

W3 = Concourse Occupant Load / Exit Capacity

W3 = 1068 / 350 = 3.051 Minutes

(W3 - W1) = 3.051 - 3.894 = 0.000 Minutes

Total Exit Time (T + (W1 - T1) + (W2 - W1) + (W3 - W1))

TOTAL = 3.894 MINUTES (NOT > 6 MINUTES)





**Rolf Jensen & Associates, Inc.**

FIRE PROTECTION ENGINEERS • BUILDING CODE CONSULTANTS

MAY 22 1989

D.C.C.

May 19, 1989

RECEIVED BY MRTC

MAY 22 1989

SAFETY & ASSURANCE

Mr. Malcolm Ingram  
Metro Rail Transit Consultants  
548 South Spring Street, 7th Floor  
Los Angeles, California 90013

B331 PRE-FINAL DESIGN REVIEW

Malcolm:

This is to confirm our telephone conversation of May 19, 1989.

We have prepared hydraulic calculations for the wet standpipe system shown on the drawings. The pipe sizes shown are adequate to produce the required 500 gpm flow. A copy of the calculations is attached.

If you have any questions, please contact us.

Sincerely,

David R. Fiedler, P.E.

DRF:klr - H3275

Attachments

5229C

SPRINKLER SYSTEM HYDRAULIC ANALYSIS

Date: May 18, 1989

A:\B331WSP2

JOB TITLE: H3275.01 B331 STANDPIPE  
WATER SUPPLY DATA

SOURCE NODE TAG	STATIC PRESS. (PSI)	RESID. PRESS. (PSI)	FLOW @ (GPM)	AVAIL. PRESS. @ (PSI)	TOTAL DEMAND (GPM)	REQ'D PRESS. (PSI)
1	69.0	20.0	2235.0	65.9	500.1	35.1

AGGREGATE FLOW ANALYSIS:

TOTAL FLOW AT SOURCE	500.1 GPM
TOTAL HOSE STREAM ALLOWANCE AT SOURCE	0.0 GPM
OTHER HOSE STREAM ALLOWANCES	0.0 GPM
TOTAL DISCHARGE FROM ACTIVE SPRINKLERS	500.1 GPM

NODE ANALYSIS DATA

NODE TAG	ELEVATION (FT)	NODE TYPE	PRESSURE (PSI)	DISCHARGE (GPM)
2	514.0	- - - -	65.4	- - -
3	509.0	- - - -	67.2	- - -
4	509.0	- - - -	67.1	- - -
5	506.0	- - - -	68.4	- - -
6	509.0	- - - -	67.0	- - -
7	509.0	- - - -	67.0	- - -
8	509.0	- - - -	67.1	- - -
9	506.0	- - - -	68.4	- - -
10	500.0	- - - -	70.7	- - -
11	500.0	- - - -	70.0	- - -
12	499.0	- - - -	69.8	- - -
13	503.0	- - - -	67.7	- - -
14	506.0	- - - -	66.3	- - -
15	506.0	- - - -	66.2	- - -
16	498.0	- - - -	69.3	- - -
17	497.0	- - - -	69.1	- - -
18	497.0	- - - -	68.4	- - -
18A	495.0	- - - -	68.4	- - -
18B	501.0	- - - -	66.3	- - -
19	495.0	- - - -	68.1	- - -
20	495.0	- - - -	67.9	- - -
21	495.0	- - - -	67.3	- - -
22	499.0	- - - -	65.2	- - -
23	502.0	- - - -	63.4	- - -
24	502.0	- - - -	63.4	- - -
25	494.0	- - - -	66.8	- - -
26	493.0	- - - -	66.4	- - -
27	492.0	- - - -	66.0	- - -
27A	492.0	- - - -	65.4	- - -
27B	497.0	- - - -	63.3	- - -
28	491.0	- - - -	65.5	- - -
29	490.0	K=31.00	65.0	250.0
30	490.0	K=31.00	65.0	250.0
31	495.0	- - - -	62.9	- - -

## SPRINKLER SYSTEM HYDRAULIC ANALYSIS

Page 2

JOB TITLE: H3275.01 B331 STANDPIPE  
 NODE ANALYSIS DATA (cont'd)

NODE TAG	ELEVATION (FT)	NODE TYPE	PRESSURE (PSI)	DISCHARGE (GPM)
32	498.0	- - - -	61.7	- - -
33	498.0	- - - -	61.7	- - -
34	496.0	- - - -	62.6	- - -
35	494.0	- - - -	63.4	- - -
36	495.0	- - - -	63.0	- - -
37	497.0	- - - -	62.1	- - -
38	502.0	- - - -	60.0	- - -
39	496.0	- - - -	61.7	- - -
40	500.0	- - - -	60.8	- - -
41	502.0	- - - -	60.0	- - -
42	502.0	- - - -	60.0	- - -
43	500.0	- - - -	60.8	- - -
44	498.0	- - - -	61.7	- - -
45	497.0	- - - -	62.1	- - -
46	502.0	- - - -	60.0	- - -
47	496.0	- - - -	62.6	- - -
48	473.0	- - - -	63.9	- - -
49	491.0	- - - -	64.7	- - -
50	498.0	- - - -	61.7	- - -
51	498.0	- - - -	61.7	- - -
52	498.0	- - - -	61.7	- - -
53	495.0	- - - -	63.0	- - -
54	490.0	- - - -	65.5	- - -
55	491.0	- - - -	65.6	- - -
56	491.0	- - - -	66.1	- - -
57	492.0	- - - -	66.0	- - -
58	497.0	- - - -	63.8	- - -
59	492.0	- - - -	66.3	- - -
60	493.0	- - - -	66.4	- - -
61	494.0	- - - -	66.5	- - -
62	502.0	- - - -	63.4	- - -
63	502.0	- - - -	63.5	- - -
64	499.0	- - - -	64.8	- - -
65	495.0	- - - -	66.9	- - -
66	495.0	- - - -	67.6	- - -
67	496.0	- - - -	67.9	- - -
68	496.0	- - - -	68.3	- - -
69	501.0	- - - -	66.1	- - -
70	497.0	- - - -	63.2	- - -
71	497.0	- - - -	63.9	- - -
72	478.0	- - - -	69.2	- - -
73	506.0	- - - -	66.2	- - -
74	506.0	- - - -	66.3	- - -
75	503.0	- - - -	67.6	- - -
76	499.0	- - - -	69.7	- - -
77	500.0	- - - -	69.9	- - -
78	500.0	- - - -	70.6	- - -
1	587.0	SOURCE	35.1	500.1

SPRINKLER SYSTEM HYDRAULIC ANALYSIS

JOB TITLE: H3275.01 8331 STANDPIPE  
PIPE DATA

PIPE TAG	END	ELEV.	NOZ.	PT	DISC.	Q(GPM)	DIA(IN)	LENGTH	FRESS.
NODES	(FT)	(K)	(PSI)	(GPM)	VEL(FFS)	HW(C)	(FT)	SUM.	
						F.L./FT		(PSI)	
Pipe: 1									
1	587.0	SRCE	35.1	(N/A)	3.1	120	FTG	458.00	PE 31.6
2	514.0	0.0	65.4	0.0	0.002	TL	573.00	FV 0.0	
Pipe: 2									
2	514.0	0.0	65.4	0.0	5.6	120	FTG	E	PE 2.2
3	509.0	0.0	67.2	0.0	0.010	TL	34.00	FV 0.2	
Pipe: 3									
3	509.0	0.0	67.2	0.0	2.8	120	FTG	T	PE 0.0
4	509.0	0.0	67.1	0.0	0.003	TL	34.00	FV 0.0	
Pipe: 4									
4	509.0	0.0	67.1	0.0	0.0	120	FTG	T	PE 1.3
5	506.0	0.0	68.4	0.0	0.000	TL	34.00	FV 0.0	
Pipe: 5									
4	509.0	0.0	67.1	0.0	2.8	120	FTG	T	PE 0.0
6	509.0	0.0	67.0	0.0	0.003	TL	34.00	FV 0.0	
Pipe: 6									
6	509.0	0.0	67.0	0.0	2.8	120	FTG	E	PE 3.9
10	500.0	0.0	70.7	0.0	0.003	TL	84.00	FV 0.0	
Pipe: 7									
6	509.0	0.0	67.0	0.0	0.0	120	FTG	----	PE 0.0
7	509.0	0.0	67.0	0.0	0.000	TL	8.00	FV 0.0	
Pipe: 8									
10	500.0	0.0	70.7	0.0	2.8	120	FTG	----	PE 0.0
11	500.0	0.0	70.0	0.0	0.003	TL	250.00	FV 0.0	
Pipe: 9									
11	500.0	0.0	70.0	0.0	2.8	120	FTG	----	PE 0.4
12	499.0	0.0	69.8	0.0	0.003	TL	250.00	FV 0.0	
Pipe: 10									
12	499.0	0.0	69.8	0.0	2.8	120	FTG	E	PE 1.7
13	503.0	0.0	67.7	0.0	0.003	TL	137.00	FV 0.0	
Pipe: 11									
13	503.0	0.0	67.7	0.0	2.8	120	FTG	T	PE 1.3
14	506.0	0.0	66.3	0.0	0.003	TL	34.00	FV 0.0	
Pipe: 12									
14	506.0	0.0	66.3	0.0	2.7	120	FTG	T	PE 0.0
15	506.0	0.0	66.2	0.0	0.003	TL	34.00	FV 0.0	
Pipe: 13									
14	506.0	0.0	66.3	0.0	0.0	120	FTG	----	PE 0.0
74	506.0	0.0	66.3	0.0	0.000	TL	4.00	FV 0.0	

SPRINKLER SYSTEM HYDRAULIC ANALYSIS

JOB TITLE: H3275.01 B331 STANDPIPE  
 PIPE DATA (cont'd)

PIPE TAG	END	ELEV.	NOZ.	PT	DISC.	Q (GPM)	DIA (IN)	LENGTH	PRESS.
NODES	(FT)	(K)	(PSI)	(GPM)		VEL (FPS)	HW (C)	(FT)	SUM.
							F.L./FT		(PSI)
	Pipe: 14					245.3	6.065	PL 125.00	FF 0.4
15	506.0	0.0	66.2	0.0	2.7	120	FTG E	FE 3.5	
16	493.0	0.0	69.3	0.0		0.003	TL 139.00	FV 0.0	
	Pipe: 15					245.3	6.065	PL 250.00	FF 0.7
16	498.0	0.0	69.3	0.0	2.7	120	FTG ----	FE 0.4	
17	497.0	0.0	69.1	0.0		0.003	TL 250.00	FV 0.0	
	Pipe: 16					245.3	6.065	PL 250.00	FF 0.7
17	497.0	0.0	69.1	0.0	2.7	120	FTG ----	FE 0.0	
18	497.0	0.0	68.4	0.0		0.003	TL 250.00	FV 0.0	
	Pipe: 17					245.3	6.065	PL 125.00	FF 0.4
18	497.0	0.0	68.4	0.0	2.7	120	FTG T	FE 0.4	
18A	495.0	0.0	68.4	0.0		0.003	TL 155.00	FV 0.0	
	Pipe: 18					0.0	4.026	PL 6.00	FF 0.0
18A	496.0	0.0	68.4	0.0	0.0	120	FTG ----	FE 2.2	
18B	501.0	0.0	66.3	0.0		0.000	TL 6.00	FV 0.0	
	Pipe: 19					245.3	6.065	PL 120.00	FF 0.3
18A	496.0	0.0	68.4	0.0	2.7	120	FTG ----	FE 0.0	
19	496.0	0.0	68.1	0.0		0.003	TL 120.00	FV 0.0	
	Pipe: 20					245.3	6.065	PL 250.00	FF 0.7
19	496.0	0.0	68.1	0.0	2.7	120	FTG ----	FE 0.4	
20	495.0	0.0	67.9	0.0		0.003	TL 250.00	FV 0.0	
	Pipe: 21					245.3	6.065	PL 250.00	FF 0.7
20	495.0	0.0	67.9	0.0	2.7	120	FTG ----	FE 0.0	
21	495.0	0.0	67.3	0.0		0.003	TL 250.00	FV 0.0	
	Pipe: 21A					245.3	6.065	PL 125.00	FF 0.4
21	495.0	0.0	67.3	0.0	2.7	120	FTG E	FE 1.7	
22	499.0	0.0	65.2	0.0		0.003	TL 139.00	FV 0.0	
	Pipe: 22					245.3	6.065	PL 125.00	FF 0.4
22	499.0	0.0	65.2	0.0	2.7	120	FTG T	FE 1.3	
23	502.0	0.0	63.4	0.0		0.003	TL 155.00	FV 0.0	
	Pipe: 23					245.3	6.065	PL 4.00	FF 0.0
23	502.0	0.0	63.4	0.0	2.7	120	FTG T	FE 0.0	
24	502.0	0.0	63.4	0.0		0.003	TL 34.00	FV 0.0	
	Pipe: 24					235.3	6.065	PL 8.00	FF 0.0
24	502.0	0.0	63.4	0.0	0.4	120	FTG ----	FE 0.0	
62	502.0	0.0	63.4	0.0		0.000	TL 8.00	FV 0.0	
	Pipe: 25					280.6	6.065	PL 4.00	FF 0.0
24	502.0	0.0	63.4	0.0	3.1	120	FTG E	FE 3.5	
25	494.0	0.0	66.8	0.0		0.003	TL 18.00	FV 0.0	

JOB TITLE: H3275.01 B331 STANDPIPE  
 PIPE DATA (cont'd)

PIPE TAG	END	ELEV.	NOZ.	FT	DISC.	Q (GFM)	DIA (IN)	LENGTH	FRESS.
NODES	(FT)	(K)	(PSI)	(GFM)	VEL (FPS)	HW (C)	(FT)	SUM.	
						F.L./FT		(PSI)	
	Pipe: 26					280.6	6.065	PL 250.00	PF 0.8
25	494.0	0.0	65.8	0.0	3.1	120	FTG ----	PE 0.4	
26	493.0	0.0	65.4	0.0		0.003	TL 250.00	PV 0.0	
	Pipe: 27					280.6	6.065	PL 250.00	PF 0.8
26	493.0	0.0	65.4	0.0	3.1	120	FTG ----	PE 0.4	
27	492.0	0.0	65.0	0.0		0.003	TL 250.00	PV 0.0	
	Pipe: 28					280.6	6.065	PL 125.00	PF 0.5
27	492.0	0.0	65.0	0.0	3.1	120	FTG T	PE 0.0	
27A	492.0	0.0	65.4	0.0		0.003	TL 155.00	PV 0.0	
	Pipe: 29					0.0	4.026	PL 6.00	PF 0.0
27A	492.0	0.0	65.4	0.0	0.0	120	FTG ----	PE 2.2	
27B	497.0	0.0	63.3	0.0		0.000	TL 6.00	PV 0.0	
	Pipe: 30					280.6	6.065	PL 125.00	PF 0.4
27A	492.0	0.0	65.4	0.0	3.1	120	FTG ----	PE 0.4	
28	491.0	0.0	65.5	0.0		0.003	TL 125.00	PV 0.0	
	Pipe: 31					280.6	6.065	PL 250.00	PF 0.8
28	491.0	0.0	65.5	0.0	3.1	120	FTG ----	PE 0.4	
29	490.0	31.0	65.0	250.0		0.003	TL 250.00	PV 0.0	
	Pipe: 32					30.6	6.065	PL 250.00	PF 0.0
29	490.0	31.0	65.0	250.0	0.3	120	FTG ----	PE 0.0	
30	490.0	31.0	65.0	250.0		0.000	TL 250.00	PV 0.0	
	Pipe: 33					-219.4	6.065	PL 4.00	PF 0.0
30	490.0	31.0	65.0	250.0	2.4	120	FTG E	PE 2.2	
31	495.0	0.0	62.9	0.0		0.002	TL 18.00	PV 0.0	
	Pipe: 34					-219.4	6.065	PL 4.00	PF 0.0
31	495.0	0.0	62.9	0.0	2.4	120	FTG T	PE 1.3	
32	498.0	0.0	61.7	0.0		0.002	TL 34.00	PV 0.0	
	Pipe: 35					-87.6	6.065	PL 4.00	PF 0.0
32	496.0	0.0	61.7	0.0	1.0	120	FTG T	PE 0.0	
33	498.0	0.0	61.7	0.0		0.000	TL 34.00	PV 0.0	
	Pipe: 36					-3.6	6.065	PL 125.00	PF 0.0
33	498.0	0.0	61.7	0.0	0.0	120	FTG E	PE 0.9	
34	495.0	0.0	62.6	0.0		0.000	TL 139.00	PV 0.0	
	Pipe: 37					-86.1	6.065	PL 8.00	PF 0.0
33	498.0	0.0	61.7	0.0	1.0	120	FTG ----	PE 0.0	
50	498.0	0.0	61.7	0.0		0.000	TL 8.00	PV 0.0	
	Pipe: 38					-3.6	6.065	PL 125.00	PF 0.0
34	496.0	0.0	62.6	0.0	0.0	120	FTG ----	PE 0.9	
35	494.0	0.0	63.4	0.0		0.000	TL 125.00	PV 0.0	

SPRINKLER SYSTEM HYDRAULIC ANALYSIS

JOB TITLE: H3275.01 B331 STANDPIPE  
 PIPE DATA (cont'd)

PIPE TAG	END	ELEV.	NOZ.	FT	DISC.	Q (GPM)	VEL (FPS)	DIA (IN)	HW (C)	LENGTH	FRESS.	SUM.
NODES	(FT)	(K)	(PSI)	(GPM)				F.L./FT		(FT)	(PSI)	(PSI)
	Pipe: 39					-3.6		6.065	FL	250.00	PF	0.0
35	494.0	0.0	63.4	0.0	0.0			120	FTG	----	PE	0.4
36	495.0	0.0	63.0	0.0	0.0			0.000	TL	250.00	PV	0.0
	Pipe: 40					-3.6		6.065	FL	125.00	PF	0.0
36	495.0	0.0	63.0	0.0	0.0			120	FTG	T	PE	0.9
37	497.0	0.0	62.1	0.0	0.0			0.000	TL	155.00	PV	0.0
	Pipe: 41					0.0		4.026	FL	6.00	PF	0.0
37	497.0	0.0	62.1	0.0	0.0			120	FTG	----	PE	2.2
38	502.0	0.0	60.0	0.0	0.0			0.000	TL	6.00	PV	0.0
	Pipe: 42					-3.6		6.065	FL	125.00	PF	0.0
37	497.0	0.0	62.1	0.0	0.0			120	FTG	----	PE	0.4
39	498.0	0.0	61.7	0.0	0.0			0.000	TL	125.00	PV	0.0
	Pipe: 43					-3.6		6.065	FL	250.00	PF	0.0
37	498.0	0.0	61.7	0.0	0.0			120	FTG	----	PE	0.9
40	500.0	0.0	60.8	0.0	0.0			0.000	TL	250.00	PV	0.0
	Pipe: 44					-3.6		6.065	FL	250.00	PF	0.0
40	500.0	0.0	60.8	0.0	0.0			120	FTG	----	PE	0.9
41	502.0	0.0	60.0	0.0	0.0			0.000	TL	250.00	PV	0.0
	Pipe: 45					-3.6		6.065	FL	250.00	PF	0.0
41	502.0	0.0	60.0	0.0	0.0			120	FTG	25	PE	0.0
42	502.0	0.0	60.0	0.0	0.0			0.000	TL	278.00	PV	0.0
	Pipe: 46					-3.6		6.065	FL	250.00	PF	0.0
42	502.0	0.0	60.0	0.0	0.0			120	FTG	----	PE	0.9
43	500.0	0.0	60.8	0.0	0.0			0.000	TL	250.00	PV	0.0
	Pipe: 47					-3.6		6.065	FL	250.00	PF	0.0
43	500.0	0.0	60.8	0.0	0.0			120	FTG	----	PE	0.9
44	498.0	0.0	61.7	0.0	0.0			0.000	TL	250.00	PV	0.0
	Pipe: 48					-3.6		6.065	FL	125.00	PF	0.0
44	498.0	0.0	61.7	0.0	0.0			120	FTG	T	PE	0.4
45	497.0	0.0	62.1	0.0	0.0			0.000	TL	155.00	PV	0.0
	Pipe: 49					0.0		4.026	FL	6.00	PF	0.0
45	497.0	0.0	62.1	0.0	0.0			120	FTG	----	PE	2.2
46	502.0	0.0	60.0	0.0	0.0			0.000	TL	6.00	PV	0.0
	Pipe: 50					-3.6		6.065	FL	125.00	PF	0.0
45	497.0	0.0	62.1	0.0	0.0			120	FTG	----	PE	0.4
47	496.0	0.0	62.6	0.0	0.0			0.000	TL	125.00	PV	0.0
	Pipe: 51					-3.6		6.065	FL	250.00	PF	0.0
47	496.0	0.0	62.6	0.0	0.0			120	FTG	----	PE	1.3
48	493.0	0.0	63.9	0.0	0.0			0.000	TL	250.00	PV	0.0

SPRINKLER SYSTEM HYDRAULIC ANALYSIS

JOB TITLE: H3275.01 B331 STANDPIPE  
 PIPE DATA (cont'd)

PIPE TAG	Q (GPM)	DIA (IN)	LENGTH	PRESS.
END	VEL (FPS)	HW (C)	(FT)	SUM.
NODES	(K)	F.L./FT	(PSI)	(PSI)
Pipe: 52	-3.6	6.065	PL 125.00	PF 0.0
48	493.0	0.0	63.9 0.0 0.0 120 FTG ----	PE 0.9
49	491.0	0.0	64.7 0.0 0.0 0.000 TL 125.00	PV 0.0
Pipe: 53	-3.6	6.065	PL 125.00	PF 0.0
49	491.0	0.0	64.7 0.0 0.0 120 FTG ET	PE 3.0
50	498.0	0.0	61.7 0.0 0.0 0.000 TL 169.00	PV 0.0
Pipe: 54	-89.6	6.065	PL 4.00	PF 0.0
50	498.0	0.0	61.7 0.0 1.0 120 FTG T	PE 0.0
51	498.0	0.0	61.7 0.0 0.0 0.000 TL 34.00	PV 0.0
Pipe: 55	-129.8	6.065	PL 4.00	PF 0.0
32	498.0	0.0	61.7 0.0 1.4 120 FTG T	PE 0.0
52	498.0	0.0	61.7 0.0 0.0 0.000 TL 34.00	PV 0.0
Pipe: 56	-129.8	6.065	PL 4.00	PF 0.0
52	498.0	0.0	61.7 0.0 1.4 120 FTG ----	PE 0.0
51	498.0	0.0	61.7 0.0 0.0 0.000 TL 4.00	PV 0.0
Pipe: 57	-219.4	6.065	PL 4.00	PF 0.0
51	498.0	0.0	61.7 0.0 2.4 120 FTG ----	PE 1.3
53	495.0	0.0	63.0 0.0 0.0 0.002 TL 4.00	PV 0.0
Pipe: 58	-219.4	6.065	PL 125.00	PF 0.3
53	495.0	0.0	63.0 0.0 2.4 120 FTG E	PE 2.2
54	490.0	0.0	65.5 0.0 0.0 0.002 TL 139.00	PV 0.0
Pipe: 59	-219.4	6.065	PL 250.00	PF 0.5
54	490.0	0.0	65.5 0.0 2.4 120 FTG ----	PE 0.4
55	491.0	0.0	65.6 0.0 0.0 0.002 TL 250.00	PV 0.0
Pipe: 60	-219.4	6.065	PL 250.00	PF 0.5
55	491.0	0.0	65.6 0.0 2.4 120 FTG ----	PE 0.0
56	491.0	0.0	66.1 0.0 0.0 0.002 TL 250.00	PV 0.0
Pipe: 61	-219.4	6.065	PL 125.00	PF 0.3
56	491.0	0.0	66.1 0.0 2.4 120 FTG T	PE 0.4
57	492.0	0.0	66.0 0.0 0.0 0.002 TL 155.00	PV 0.0
Pipe: 62	0.0	4.026	PL 6.00	PF 0.0
57	492.0	0.0	66.0 0.0 0.0 120 FTG ----	PE 2.2
58	497.0	0.0	63.8 0.0 0.0 0.000 TL 6.00	PV 0.0
Pipe: 63	-219.4	6.065	PL 125.00	PF 0.3
57	492.0	0.0	66.0 0.0 2.4 120 FTG ----	PE 0.0
59	492.0	0.0	66.3 0.0 0.0 0.002 TL 125.00	PV 0.0
Pipe: 64	-219.4	6.065	PL 250.00	PF 0.5
59	492.0	0.0	66.3 0.0 2.4 120 FTG ----	PE 0.4
60	493.0	0.0	66.4 0.0 0.0 0.002 TL 250.00	PV 0.0



SPRINKLER SYSTEM HYDRAULIC ANALYSIS

JOB TITLE: H3275.01 B331 STANDPIPE  
 PIPE DATA (cont'd)

PIPE TAG	END	ELEV.	NOZ.	PT	DISC.	Q (GPM)	DIA (IN)	LENGTH	PRESS.
NODES	(FT)	(K)	(PSI)	(GPM)	VEL (FPS)	HW (C)	(FT)	SUM.	
						F.L./FT		(FSI)	
Pipe: 65						-219.4	6.065	FL 250.00	PF 0.5
60	493.0	0.0	66.4	0.0	2.4	120	FTG ----	FE 0.4	
61	494.0	0.0	66.5	0.0		0.002	TL 250.00	FV 0.0	
Pipe: 66						-219.4	6.065	FL 125.00	PF 0.4
61	494.0	0.0	66.5	0.0	2.4	120	FTG ET	PE 3.5	
62	502.0	0.0	63.4	0.0		0.002	TL 169.00	FV 0.0	
Pipe: 67						-254.8	6.065	FL 4.00	PF 0.0
62	500.0	0.0	63.4	0.0	2.8	120	FTG T	PE 0.0	
63	502.0	0.0	63.5	0.0		0.003	TL 34.00	FV 0.0	
Pipe: 68						-254.8	6.065	FL 4.00	PF 0.0
63	502.0	0.0	63.5	0.0	2.8	120	FTG ----	PE 1.3	
64	499.0	0.0	64.8	0.0		0.003	TL 4.00	FV 0.0	
Pipe: 69						-254.8	6.065	FL 125.00	PF 0.4
64	499.0	0.0	64.8	0.0	2.8	120	FTG E	PE 1.7	
65	495.0	0.0	66.9	0.0		0.003	TL 139.00	FV 0.0	
Pipe: 70						-254.8	6.065	FL 250.00	PF 0.7
65	495.0	0.0	66.9	0.0	2.8	120	FTG ----	PE 0.0	
66	495.0	0.0	67.6	0.0		0.003	TL 250.00	FV 0.0	
Pipe: 71						-254.8	6.065	FL 250.00	PF 0.7
66	495.0	0.0	67.6	0.0	2.8	120	FTG ----	PE 0.4	
67	496.0	0.0	67.9	0.0		0.003	TL 250.00	FV 0.0	
Pipe: 72						-254.8	6.065	FL 120.00	PF 0.4
67	495.0	0.0	67.9	0.0	2.8	120	FTG T	PE 0.0	
68	496.0	0.0	68.3	0.0		0.003	TL 150.00	FV 0.0	
Pipe: 73						0.0	4.026	FL 6.00	PF 0.0
68	496.0	0.0	68.3	0.0	0.0	120	FTG ----	PE 2.2	
69	501.0	0.0	66.1	0.0		0.000	TL 6.00	FV 0.0	
Pipe: 74						-254.8	6.065	FL 125.00	PF 0.4
68	496.0	0.0	68.3	0.0	2.8	120	FTG ----	PE 0.4	
70	497.0	0.0	68.2	0.0		0.003	TL 125.00	FV 0.0	
Pipe: 75						-254.8	6.065	FL 250.00	PF 0.7
70	497.0	0.0	68.2	0.0	2.8	120	FTG ----	PE 0.0	
71	497.0	0.0	68.9	0.0		0.003	TL 250.00	FV 0.0	
Pipe: 76						-254.8	6.065	FL 250.00	PF 0.7
71	497.0	0.0	68.9	0.0	2.8	120	FTG ----	PE 0.4	
72	498.0	0.0	69.2	0.0		0.003	TL 250.00	FV 0.0	
Pipe: 77						-254.8	6.065	FL 125.00	PF 0.5
72	498.0	0.0	69.2	0.0	2.8	120	FTG ET	PE 3.5	
73	506.0	0.0	66.2	0.0		0.003	TL 169.00	FV 0.0	

SPRINKLER SYSTEM HYDRAULIC ANALYSIS

JOB TITLE: H3275.01 B331 STANDPIPE  
 PIPE DATA (cont'd)

PIPE TAG	END	ELEV.	NOZ.	PT	DISC.	Q(GPM)	DIA(IN)	LENGTH	PRESS.	SUM.
NODES	(FT)	(K)	(PSI)	(GPM)		VEL(FPS)	HW(C)	(FT)	(PSI)	
							F.L./FT			
	Pipe: 78									
73	506.0	0.0	66.2	0.0	0.0	-254.8	6.065	FL 4.00	FF	0.0
74	506.0	0.0	66.3	0.0	0.0	2.8	120	FTG T	PE	0.0
	Pipe: 79									
74	506.0	0.0	66.3	0.0	0.0	-250.0	6.065	FL 4.00	FF	0.0
75	503.0	0.0	67.6	0.0	0.0	2.8	120	FTG ----	PE	1.3
	Pipe: 80									
75	503.0	0.0	67.6	0.0	0.0	-250.0	6.065	FL 125.00	FF	0.4
76	499.0	0.0	69.7	0.0	0.0	2.8	120	FTG E	PE	1.7
	Pipe: 81									
76	499.0	0.0	69.7	0.0	0.0	-250.0	6.065	FL 250.00	FF	0.7
77	500.0	0.0	69.9	0.0	0.0	2.8	120	FTG ----	PE	0.4
	Pipe: 82									
77	500.0	0.0	69.9	0.0	0.0	-250.0	6.065	FL 250.00	FF	0.7
78	500.0	0.0	70.6	0.0	0.0	2.8	120	FTG ----	PE	0.0
	Pipe: 83									
78	500.0	0.0	70.6	0.0	0.0	-250.0	6.065	FL 70.00	FF	0.3
7	509.0	0.0	67.0	0.0	0.0	2.8	120	FTG ET	PE	3.9
	Pipe: 84									
7	509.0	0.0	67.0	0.0	0.0	-250.0	6.065	FL 4.00	FF	0.0
8	509.0	0.0	67.1	0.0	0.0	2.8	120	FTG T	PE	0.0
	Pipe: 85									
8	509.0	0.0	67.1	0.0	0.0	-250.0	6.065	FL 4.00	FF	0.0
3	509.0	0.0	67.2	0.0	0.0	2.8	120	FTG T	PE	0.0
	Pipe: 86									
8	509.0	0.0	67.1	0.0	0.0	0.0	6.065	FL 4.00	FF	0.0
9	503.0	0.0	68.4	0.0	0.0	0.0	120	FTG ----	PE	1.3
	Pipe: 87									
9	503.0	0.0	68.4	0.0	0.0	0.0	6.065	FL 4.00	FF	0.0

NOTES:

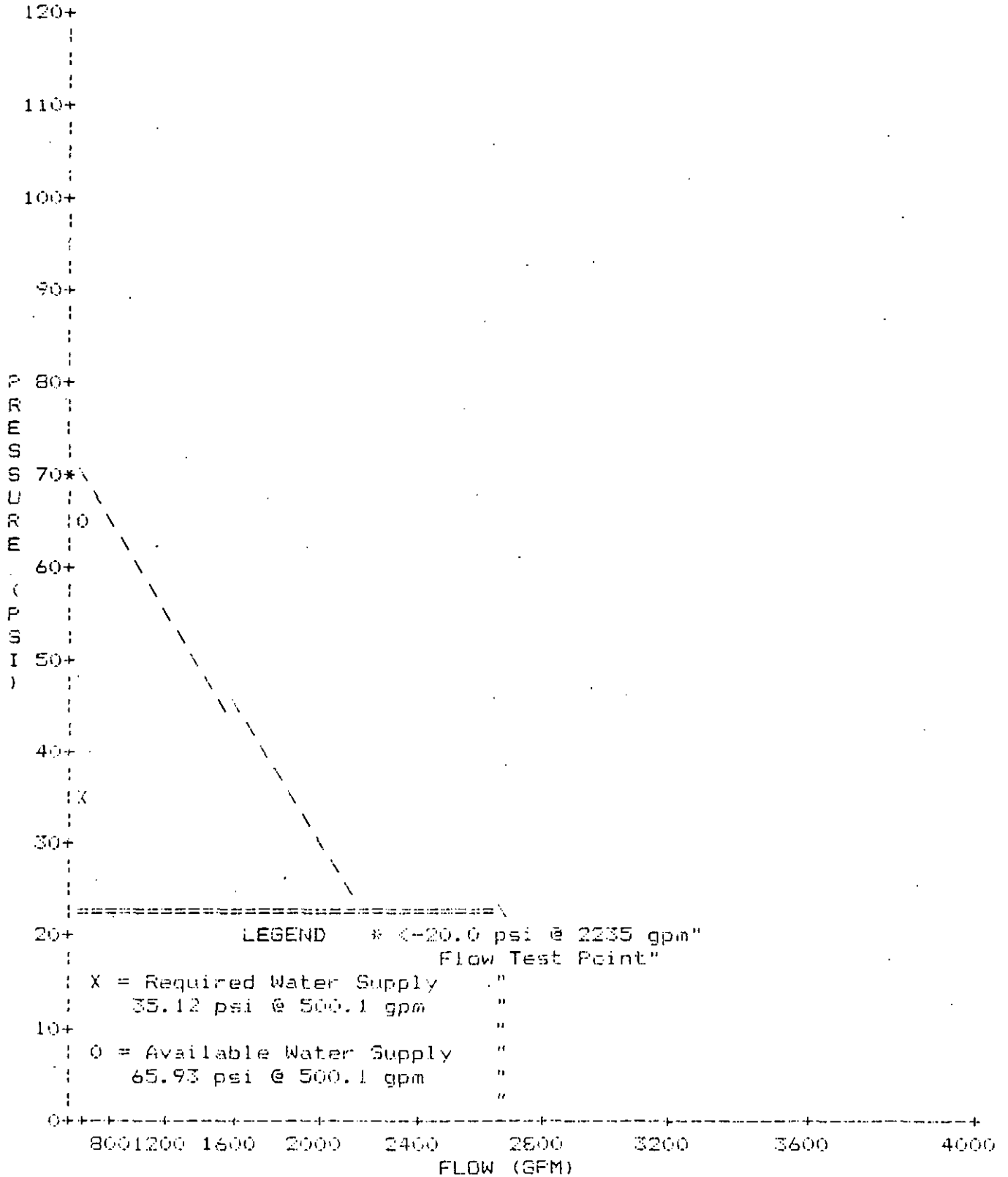
- (1) Calculations were performed by the HASS 5.2.1 computer program under license no. 3146 75A granted by HRS Systems, Inc. 2193 Ranchwood Dr., N.E. Atlanta, GA 30345
- (2) The system has been balanced to provide an average imbalance at each node of 0.007 gpm and a maximum imbalance at any node of 0.231 gpm.
- (3) Velocity pressures are printed for information only, and are not used in balancing the system. Maximum water velocity in any pipe is 5.6 ft/sec.

JOB TITLE: H3275.01 B331 STANDPIPE  
 PIPE FITTINGS TABLE

Nominal Diameter (in)	Equivalent Fitting Lengths in Feet (C=120)							
	E	T	L	C	B	G	A	D
1.00	2.00	5.00	2.00	5.00	6.00	1.00	10.00	10.00
1.25	3.00	6.00	2.00	7.00	6.00	1.00	10.00	10.00
1.50	4.00	8.00	2.00	9.00	6.00	1.00	10.00	10.00
2.00	5.00	10.00	3.00	11.00	6.00	1.00	10.00	10.00
2.50	6.00	12.00	4.00	14.00	7.00	1.00	10.00	10.00
3.00	7.00	15.00	5.00	16.00	10.00	1.00	13.00	10.00
3.50	8.00	17.00	5.00	19.00	11.00	1.00	14.00	10.00
4.00	10.00	20.00	6.00	22.00	12.00	2.00	20.00	10.00
5.00	12.00	25.00	8.00	27.00	9.00	2.00	21.00	15.00
6.00	14.00	30.00	9.00	32.00	10.00	3.00	28.00	19.00
8.00	18.00	35.00	13.00	45.00	12.00	4.00	35.00	27.00
10.00	22.00	50.00	16.00	55.00	19.00	5.00	40.00	29.00
12.00	27.00	60.00	18.00	65.00	21.00	6.00	49.00	35.00
16.00	35.00	70.00	28.00	86.00	22.00	7.00	63.00	45.00
18.00	39.00	85.00	33.00	98.00	39.00	8.00	70.00	50.00

Fitting Code Letters: E=standard ell T=tee L=long turn ell  
 C=check valve B=butterfly valve G=gate valve  
 A=alarm check valve D=dry pipe valve

JOB TITLE: H3275.01 B331 STANDPIPE  
 WATER SUPPLY CURVE



# FIRE SERVICE "PRESSURE-FLOW" REPORT

Address F.H. @ SEICOR, LANKERSHIM BL. & RIVERSIDE DR.

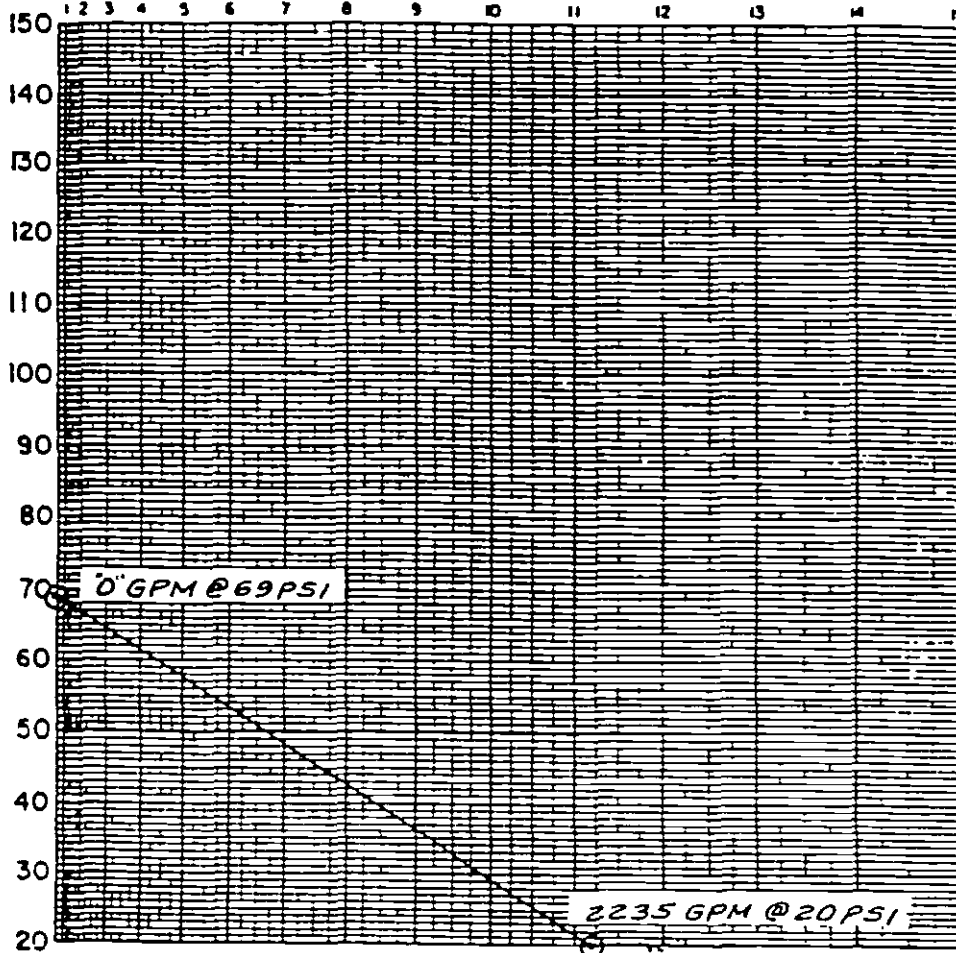
Location of (proposed/existing) \_\_\_\_\_-inch Fire Service and (proposed/existing) \_\_\_\_\_-inch Domestic Service off the \_\_\_\_\_-inch main in \_\_\_\_\_ side approximately \_\_\_\_\_ feet .. of \_\_\_\_\_.

The system maximum pressure is 118 psi based on a street elevation of 587 feet at the service location. This system maximum pressure should be used in determining the class of piping and fittings only.

The flow quantities and pressures delineated by the graph below are determined to be available at the requested service location. Future conditions in the system, however, may cause the flow quantities and pressures to change accordingly.

MULTIPLY SCALE BY 200

SYS. RESIDUAL PRESSURE IN STREET MAIN - PSI



DOMESTIC METER	FLOW (GPM)
1"	56
1 1/2"	96
2"	160
3"	220
4"	400
6"	700*
8"	1500
10"	2500

FIRE SERVICE	FLOW (GPM)
2"	250
4"	600
6"	1400
8"	2500
10"	5000

F. M. SERVICE	FLOW (GPM)
8"	2500
10"	5000

\* 1000 GPM MAY BE USED WHEN COMBINAT. ON STANOPIPE IS INSTALLED

\*\* MAXIMUM PERMISSIBLE FLOW FOR SIZE SHOWN

TOTAL FLOW AT SERVICE LOCATION - GPM

This graph will be submitted to the Department of Building and Safety for plan check purposes. For additional information call

## WATER OPERATING DIVISION

Distribution Engineering - Rm. 1432 GORDON HORST

**S.C.R.T.D. LIBRARY**

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MIRA SIMON

M. J. ALDRIAN

APR 14 1989

Coord. 166-174

APR 1 1989

APR 1 1989

P.M. \_\_\_\_\_

Prepared by

Checked by

Approved by

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